#### **University of Engineering & Technology Peshawar, Pakistan**



#### **CE301: Structure Analysis II**

#### Module 04: Analysis of S.I Pin Jointed Frames (Trusses) Using Flexibility method

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# **Topics to be Covered**

- Introduction
- Prerequisites for using flexibility method for Trusses
- Indeterminacy of trusses
- Revision of Unit Load Method
- Flexibility method procedure for truss analysis
- Analysis of trusses Example 1
- Example 2
- Example 3
- Assignments

#### **Introduction:**

Trusses are analyzed with flexibility method due to

- To solve the problem in matrix notation, which is more systematic
- To compute reactions at all the supports.
- To compute internal resisting axial force in any member of the indeterminate Truss.

The basic method for the analysis of indeterminate truss by force method is similar to the indeterminate beam analysis discussed in the previous lessons. Determine the degree of static indeterminacy of the structure. Identify the number of redundant reactions equal to the degree of indeterminacy. The redundants must be so selected that when the restraint corresponding to the redundants are removed, the resulting truss is statically determinate and stable. In case of E.I. select support reactions as redundant actions in case of I.I selection member forces as redundant actions. E.I + I.I  $\Rightarrow$  select redundants from both support reactions & member forces.

#### **Prerequisites for Analysis with Flexibility method:**

- It is necessary that students must have strong background of the following concepts before starting analysis with flexibility or any other matrix method.
- Enough concept of Matrix Algebra
- Must be able to find the Statical Indeterminacy of trusses
- Concept of methods for finding displacements (Unit load method for displacement)
- Must be able to analyze a determinate truss

#### □ Indeterminacy of Trusses:

The truss is said to be statically indeterminate when the total number of reactions and member axial forces exceed the total number of static equilibrium equations.

S.I = (m+r) - 2j

where m, j and r are number of members, joints and unknown reaction components respectively.

• Types of Indeterminate trusses:

There are three different types of S.I trusses.

- Internally indeterminate trusses only (I.I = m + 3 2j)
- Externally indeterminate trusses only (E.I = r 3)
- Internally & externally indeterminate trusses (T.I = m + r 2j)

• Internally indeterminate trusses:

A truss is said to be internally indeterminate if it has exactly three reaction components and more than (2j-3) members.

I.I = (m + 3) - 2j

• Externally indeterminate trusses:

A planar truss is said to be externally indeterminate if the number of reactions exceeds the number of static equilibrium equations available.

E. I = 
$$r - 3$$

- Internally & externally indeterminate trusses:
  - A truss is both externally and internally indeterminate if it has more than three reaction components and also has more than (2j-3) members.



To analyze this truss we have to chose two redundant actions One member force & one support reaction.

#### **Unit Load Method for Displacement**

- This Method is based on the principle of virtual work.
- Best suitable to find the slope & deflection of primary structure when subjected to external loads and unit loads.
- Displacement due to actual loads at any point is given by

$$\Delta_n = \sum \frac{P_i U n_i L i}{E A_i}$$

Note: When axial forces are predominant then use axial rigidity EA instead of EI

• Displacement at any point due to a unit load at that point or any other point is given by (also called flexibility coefficient)

$$\delta_{nm} = \sum_{i=1}^{m} \frac{U_{ni} Um_i L_i}{EA_i} = f_{nm}$$

• Where P are the values of member forces in primary structure subjected to actual loads & U are the values of member forces in BDS subjected to unit loads.

#### □ Analysis Procedure

The following steps should be followed to analyze a statical indeterminate Truss using flexibility method. 1<sup>st</sup> find the degree of statical indeterminacy S.I (E.I & I.I).

Step # 01: Identify redundants accordingly and obtain BDS
Compute [DRS] values.
Assume # of redundants = 2
No. of redundants = D.S.I

Note: when a truss is both externally & internally indeterminate then choose redundant actions from both member forces & support reactions. DRS is initial support rotation or settlement corresponding to redundant locations

- **Step # 02:** Analyze the BDS (primary structure) under the following loading conditions
- i. Analyze the BDS when acted upon by the actual loads and compute the values of member forces P<sub>i</sub>.
- ii. Analyze the BDS when acted upon by the unit load at redundant location 1 & compute the values of member forces  $U_{1i}$ .
- iii. Analyze the BDS when acted upon by the unit load at redundant location 2 & compute the values of member forces  $U_{2i}$ .
- iv. And so on if redundants are more than 2.

• Step # 03: Develop members forces table. Member forces table will consist of following information's for making the calculation work easy.

Members	Axial rigidity EA	Length	Forces in the released truss due to actual applied loading (P <sub>i</sub> values)	Forces in the released truss due to unit load at redundant 1 $(U_{1i} \text{ values})$	Forces in the released truss due to unit load at redundant 2 $(U_{2i} \text{ values})$
AB	(EA) <sub>AB</sub>	L <sub>AB</sub>	P <sub>AB</sub>	$(U_1)_{AB}$	$(U_2)_{AB}$
BC	(EA) <sub>BC</sub>	L <sub>BC</sub>	P <sub>BC</sub>	$(U_1)_{BC}$	$(U_2)_{BC}$
And so on up to m members	(EA) <sub>m</sub>	$L_m$	P <sub>m</sub>	(U <sub>1</sub> ) <sub>m</sub>	(U <sub>2</sub> ) <sub>m</sub>

• **Step # 04:** Find BDS Displacements due to external loads or Compute the values of DRL

$$DRL_1 = \sum_{i=1}^m \frac{P_i U_{1i} Li}{EA_i}$$

$$DRL_2 = \sum_{i=1}^m \frac{P_i U_{2i} Li}{EA_i}$$

 Step # 05: Find BDS Displacements due to unit loads or Compute the values of flexibility coefficients & flexibility matrix.

$$f_{12} = f_{21} = \sum_{i=1}^{m} \frac{U_{1i} U_{2i} L_i}{EA_i}$$

$$f_{11} = \sum_{i=1}^{m} \frac{U_{1i} U_{1i} L_i}{EA_i} \qquad f_{22} = \sum_{i=1}^{m} \frac{U_{2i} U_{2i} L_i}{EA_i}$$
$$[f] = \begin{bmatrix} f_{11} & f_{12} \\ f_{21} & f_{22} \end{bmatrix} \qquad \text{Note:} f_{12} = f_{21}$$

**Flexibility Method for Trusses Analysis Step # 06:** Apply compatibility and principle of superposition at the location of the removed redundants and solve the matrices to compute the values of redundant actions AR. As we know

 $[DRS] = [DRL] + [f] \cdot [AR]$  $[AR] = [f]^{-1} \cdot [DRS - DRL]$  $\begin{bmatrix}AR_{1}\\AR_{2}\end{bmatrix} = \begin{bmatrix}f_{11} & f_{12}\\f_{21} & f_{22}\end{bmatrix}^{-1} \begin{bmatrix}DRS_{1} - DRL_{1}\\DRS_{2} - DRL_{2}\end{bmatrix}$ 

From this

**Step # 07:** Compute the member end actions

[AM] = [AML] + [AMR][AR]

Note: Sign convention

Tension in member will be taken as +ive ( away from the joints)

Compression in member will be taken as -ive ( towards the joints)

**Problem 01:** Analyze the given truss using flexibility method.



S.I = 2 degrees So two redundant actions should be chosen.

• **Step # 01:** Identify the redundants and obtain BDS also compute [DRS] values.



Horizontal reaction at D and member force in member AD are chosen as redundant actions



#### Basic determinate structure (BDS) or Primary structure or Released structure

- **Step # 02:** Analyze the BDS (primary structure) under the following loading conditions
- i. Analyze the BDS when acted upon by the actual loads and compute the values of member forces  $P_i$ .
- ii. Analyze the BDS when acted upon by the unit load at redundant location 1 & compute the values of member forces  $U_{1i}$ .
- iii. Analyze the BDS when acted upon by the unit load at redundant location 2 & compute the values of member forces  $U_{2i}$ .

#### i. BDS acted upon by the actual loads



Now find all the member forces using method of joints.

Joint B: Joint A: Joint D:  $F_{DC}$  $F_{BD}$  $F_{BC}$ 10k  $F_{AB} = 10k$  $F_{BD} = 0k$  $F_{AB}$ 7.5k  $\sum Fx = 0$ 12.5k  $\sum Fx = 0$  $\Rightarrow F_{DC} = 0 k$  $\sum Fy = 0 \implies F_{AC} = -12.5 k$  $FBC\cos 36.86 = -10$  $\sum Fx = 0 \implies F_{AB} = 10 k$  $F_{BC} = -12.5 \ k$  $\sum Fy = 0$  $\overline{F}_{BD} = -FBC\sin 36.86 - 7.5$  $F_{BD} = 0k$ 



P<sub>i</sub>-Values

ii. BDS acted upon by the Unit load at redundant location 1.



Now find all the member forces using method of joints.

Joint A:

Joint B:

Joint D:

 $F_{DC}$ 





 $\sum Fx = 0$  $FBC \cos 36.86 = -1$  $F_{BC} = -1.25 \ k$  $F_{BD} = -FBC \sin 36.86 - 0.75$  $F_{BD} = 0k$ 

 $F_{BD} = 0k$  $\sum Fx = 0$ 

 $\Rightarrow F_{DC} = 1 k$ 

 $\sum Fx = 0 \implies F_{AB} = 1 k$ 

 $\sum Fy = 0 \implies F_{AC} = 0.75 k$ 

 $\sum Fy = 0$ 



 $\overline{U_{1i}}$  - Values

iii. BDS acted upon by the Unit load at redundant location 2.



Now find all the member forces using method of joints.





 $U_{2i}$  - Values

• **Step # 03:** Develop Member forces table.

Member	AE	Length, L <sub>i</sub> (ft)	P <sub>i</sub> values	U <sub>1i</sub> Values	U <sub>2i</sub> values
AB	Constant	20	10	1	-0.8
BC		25	-12.5	-1.25	1
CD		20	0	1	-0.8
AD		25	0	0	1
AC		15	-12.5	0.75	-0.6
BD		15	0	0	-0.6

• **Step # 04:** Find BDS Displacements due to external loads or Compute the values of DRL matrix.

$$DRL_1 = \sum_{i=1}^m \frac{P_i U_{1i} Li}{EA_i}$$

$$DRL_{1} = \frac{1}{EA} \begin{bmatrix} (20 * 10 * 1) + (25 * -12.5 * -1.25) + 0 \\ + 0 + (15 * -12.5 * 0.75) + 0 \end{bmatrix}$$

$$DRL_1 = \frac{450}{EA}$$

$$DRL_2 = \sum_{i=1}^m \frac{P_i U_{2i} Li}{EA_i}$$

$$DRL_{2} = \frac{1}{EA} \begin{bmatrix} (20 * 10 * -0.8) + (25 * -12.5 * 1) + 0 \\ + 0 + (15 * 0.75 * 0.75) + 0 \end{bmatrix}$$

$$DRL_2 = \frac{-360}{EA}$$

$$DRL_{1} = \frac{450}{EA} \qquad DRL_{2} = \frac{-360}{EA}$$
$$[DRL] = \begin{bmatrix} DRL_{1} \\ DRL_{2} \end{bmatrix} = \frac{1}{EA} \begin{bmatrix} 450 \\ -360 \end{bmatrix}$$

• **Step # 05:** Find BDS Displacements due to unit loads

or Compute the values of flexibility coefficients & flexibility matrix.

$$f_{11} = \sum_{i=1}^{m} \frac{U_{1i} U_{1i} L_i}{EA_i}$$

$$f_{11} = \frac{1}{EA} \begin{bmatrix} (20 * 1 * 1) + (25 * -1.25 * -1.25) + (20 * 1 * 1) \\ + 0 + (15 * -12.5 * -0.6) + 0 \end{bmatrix}$$

$$f_{11} = \frac{87.5}{EA}$$
$$f_{12} = f_{21} = \sum_{i=1}^{m} \frac{U_{1i}U_{2i}L_{i}}{EA_{i}}$$

 $f_{12} = f_{21} = \frac{1}{EA} \begin{bmatrix} (20 * 1 * -0.8) + (25 * -1.25 * 1) + (20 * 1 * -0.8) \\ + 0 + (15 * 0.75 * -0.6) + 0 \end{bmatrix}$ 

$$f_{12} = f_{21} = \frac{-70}{EA}$$

$$f_{22} = \sum_{i=1}^{m} \frac{U_{2i} U_{2i} L_i}{EA_i}$$

 $f_{22} = \frac{1}{EA} \begin{bmatrix} (20 * -0.8 * -0.8) + (25 * 1 * 1) + (20 * -0.8 * -0.8) \\ + (25 * 1 * 1) + (15 * -0.6 * -0.6) + (15 * -0.6 * -0.6) \end{bmatrix}$ 

$$f_{22} = \frac{86.40}{EA}$$

$$F_{11} = \frac{87.5}{EA}$$
  $f_{22} = \frac{86.40}{EA}$   
 $f_{12} = f_{21} = \frac{-70}{EA}$ 

$$[f] = \begin{bmatrix} f_{11} & f_{12} \\ f_{21} & f_{22} \end{bmatrix}$$

$$[f] = \frac{1}{EA} \begin{bmatrix} 87.5 & -70 \\ -70 & 86.40 \end{bmatrix}$$

**Flexibility Method for Trusses Analysis Step # 06:** Apply compatibility and principle of superposition at the location of the removed redundants and solve the matrices to compute the values of redundant actions AR. As we know

 $[DRS] = [DRL] + [f] \cdot [AR]$  $[AR] = [f]^{-1} \cdot [DRS - DRL]$  $\begin{bmatrix}AR_{1}\\AR_{2}\end{bmatrix} = \begin{bmatrix}f_{11} & f_{12}\\f_{21} & f_{22}\end{bmatrix}^{-1} \begin{bmatrix}DRS_{1} - DRL_{1}\\DRS_{2} - DRL_{2}\end{bmatrix}$ 

From this

$$\begin{bmatrix} AR_1 \\ AR_2 \end{bmatrix} = \begin{bmatrix} f_{11} & f_{12} \\ f_{21} & f_{22} \end{bmatrix}^{-1} \begin{bmatrix} DRS_1 - DRL_1 \\ DRS_2 - DRL_2 \end{bmatrix}$$

$$\begin{bmatrix} AR_1 \\ AR_2 \end{bmatrix} = EA \begin{bmatrix} 87.5 & -70 \\ -70 & 86.40 \end{bmatrix}^{-1} \begin{bmatrix} 0 - (450) \\ 0 - (-360) \end{bmatrix} \times \frac{1}{EA}$$

$$\begin{bmatrix} AR_1 \\ AR_2 \end{bmatrix} = \begin{bmatrix} -5.14 \\ 0 \end{bmatrix}$$

-ive sign shows that our assumed redundant action direction is wrong



Final determinate structure

**Step # 07:** Compute the member end actions. As we know that

[AM] = [AML] + [AMR][AR]





b) Compute the AMR values: when unit is applied at redundant location 1.



b) Compute the AMR values: when unit is applied at redundant location 2.



$$[AMR]_{6*2} = \begin{bmatrix} AMR_{11} & AMR_{12} \\ AMR_{21} & AMR_{22} \\ AMR_{31} & AMR_{32} \\ AMR_{41} & AMR_{42} \\ AMR_{51} & AMR_{52} \\ AMR_{61} & AMR_{62} \end{bmatrix} = \begin{bmatrix} 1 & -0.8 \\ -1.25 & 1 \\ 1 & -0.8 \\ 0 & 1 \\ 0.75 & -0.6 \\ 1 & -0.6 \end{bmatrix}$$

Now member end actions will be computed as given below

$$\begin{bmatrix} AM_{1} \\ AM_{2} \\ AM_{3} \\ AM_{4} \\ AM_{5} \\ AM_{6} \end{bmatrix} = \begin{bmatrix} AML_{1} \\ AML_{2} \\ AML_{3} \\ AML_{4} \\ AML_{5} \\ AML_{6} \end{bmatrix} + \begin{bmatrix} AMR_{11} & AMR_{12} \\ AMR_{21} & AMR_{22} \\ AMR_{31} & AMR_{32} \\ AMR_{41} & AMR_{32} \\ AMR_{41} & AMR_{42} \\ AMR_{51} & AMR_{52} \\ AMR_{61} & AMR_{52} \end{bmatrix} \begin{bmatrix} AR_{1} \\ AR_{2} \end{bmatrix}$$





Truss with all member forces are known

To find the support reaction simply apply equilibrium equations and we will get



Completely analyzed structure

**Problem 02:** Analyze the given truss using flexibility method.



E.I =  $5 - 3 = 2^{\circ}$ I.I =  $9 + 3 - 2(6) = 0^{\circ}$ T.I = 2 degrees

S.I = 2 degrees So two redundant actions should be chosen.

• **Step # 01:** Identify the redundants and obtain BDS also compute [DRS] values.



Vertical reaction at B & C are chosen as redundant actions



Basic determinate structure (BDS) or Primary structure or Released structure

- **Step # 02:** Analyze the BDS (primary structure) under the following loading conditions
- i. Analyze the BDS when acted upon by the actual loads and compute the values of member forces  $P_i$ .
- ii. Analyze the BDS when acted upon by the unit load at redundant location 1 & compute the values of member forces  $U_{1i}$ .
- iii. Analyze the BDS when acted upon by the unit load at redundant location 2 & compute the values of member forces  $U_{2i}$ .



Now find all the member forces using method of joints & we will get



Class Activity: Verify the member forces using method of joints

ii. BDS acted upon by the Unit load at redundant location 1.



$$\sum_{\substack{M_A = 0 \\ 1 * 20 = VD * 120 \\ V_D = 0.33 k}} \sum_{\substack{Fy = 0 \\ V_A = 0.67 k}} \sum_{\substack{Fx = 0 \\ H_A = 0k}} K_{A} = 0$$

Now find all the member forces using method of joints & we will get



Class Activity: Verify the member forces using method of joints

iii. BDS acted upon by the Unit load at redundant location 2.



Now find all the member forces using method of joints & we will get



Class Activity: Verify the member forces using method of joints

• **Step # 03:** Develop Member forces table.

Member	AE	Length, L <sub>i</sub> (ft)	P <sub>i</sub> values	U <sub>1i</sub> Values	U <sub>2i</sub> values
AB		40	26.67	0.89	0.44
BC		40	26.67	0.89	0.44
CD		40	26.67	0.44	0.89
DE	Constant	50	-33.33	-0.55	-1.11
EF		40	-26.67	-0.44	-0.89
AF		50	-33.33	-1.11	-0.55
BF		30	0	1	0
CF		50	0	-0.55	0.55
CE		30	0	0.33	0.67

• **Step # 04:** Find BDS Displacements due to external loads or Compute the values of DRL matrix.

$$DRL_1 = \sum_{i=1}^m \frac{P_i U_{1i} Li}{EA_i}$$

 $DRL_{1} = \frac{1}{EA} \begin{bmatrix} (40 * 26.67 * 0.89) + (40 * 26.67 * 0.89) + (40 * 26.67 * 0.44) \\ + (50 * -33.33 * -0.55) + (40 * -26.67 * -0.44) \\ + (50 * -33.33 * -1.11) + 0 + 0 + 0 \end{bmatrix}$ 

$$DRL_1 = \frac{5622.3}{EA}$$

$$DRL_2 = \sum_{i=1}^m \frac{P_i U_{2i} Li}{EA_i}$$

$$DRL_{2} = \frac{1}{EA} \begin{bmatrix} (40 * 26.67 * 0.44) + (40 * 26.67 * 0.44) + (40 * 26.67 * 0.89) \\ + (50 * -33.33 * -1.11) + (40 * -26.67 * -0.89) \\ + (50 * -33.33 * -0.55) + 0 + 0 + 0 \end{bmatrix}$$

$$DRL_{2} = \frac{5622.3}{EA}$$
$$DRL_{1} = \frac{5622.3}{EA} \quad DRL_{2} = \frac{5622.3}{EA}$$
$$[DRL] = \begin{bmatrix} DRL_{1} \\ DRL_{2} \end{bmatrix} = \frac{1}{EA} \begin{bmatrix} 5622.3 \\ 5622.3 \end{bmatrix}$$

• **Step # 05:** Find BDS Displacements due to unit loads

or Compute the values of flexibility coefficients & flexibility matrix.

$$f_{11} = \sum_{i=1}^{m} \frac{U_{1i} U_{1i} L_i}{EA_i}$$

 $f_{11} = \frac{1}{EA} \begin{bmatrix} (40 * 0.89 * 0.89) + (40 * 0.89 * 0.89) + (40 * 0.44 * 0.44) \\ + (50 * -0.55 * -0.55) + (40 * -0.44 * -0.44) \\ + (50 * -1.11 * -1.11) + (50 * -1.11 * -1.11) \\ + (50 * -0.55 * -0.55) + (30 * 0.33 * 0.33) \end{bmatrix}$ 

 $f_{11} = \frac{205.2}{EA}$ 

$$f_{12} = f_{21} = \sum_{i=1}^{m} \frac{U_{1i}U_{2i}L_{i}}{EA_{i}}$$

$$f_{12} = f_{21} = \frac{1}{EA} \begin{bmatrix} (40 * 0.89 * 0.44) + (40 * 0.89 * 0.44) + (40 * 0.44 * 0.89) \\ + (50 * -0.55 * -1.11) + (40 * -0.44 * -0.89) \\ + (50 * -1.11 * -0.55) + 0 + (50 * -0.55 * 0.55) \\ + (30 * 0.33 * 0.67) \end{bmatrix}$$

$$f_{12} = f_{21} = \frac{115.8}{EA}$$

$$f_{22} = \sum_{i=1}^{m} \frac{U_{2i} U_{2i} L_i}{EA_i}$$

$$f_{22} = \frac{1}{EA} \begin{bmatrix} (40 * 0.44 * 0.44) + (40 * 0.44 * 0.44) + (40 * 0.89 * 0.89) \\ + (50 * -1.11 * -1.11) + (40 * -0.89 * -0.89) \\ + (50 * -0.55 * -0.55) + 0 + (50 * 0.55 * 0.55) \\ + (30 * 0.67 * 0.67) \end{bmatrix}$$

$$f_{22} = \frac{185.39}{EA}$$

$$[f] = \begin{bmatrix} f_{11} & f_{12} \\ f_{21} & f_{22} \end{bmatrix} \implies [f] = \frac{1}{EA} \begin{bmatrix} 205.2 & 115.8 \\ 115.8 & 185.39 \end{bmatrix}$$

**Flexibility Method for Trusses Analysis Step # 06:** Apply compatibility and principle of superposition at the location of the removed redundants and solve the matrices to compute the values of redundant actions AR. As we know

 $[DRS] = [DRL] + [f] \cdot [AR]$  $[AR] = [f]^{-1} \cdot [DRS - DRL]$  $\begin{bmatrix}AR_{1}\\AR_{2}\end{bmatrix} = \begin{bmatrix}f_{11} & f_{12}\\f_{21} & f_{22}\end{bmatrix}^{-1} \begin{bmatrix}DRS_{1} - DRL_{1}\\DRS_{2} - DRL_{2}\end{bmatrix}$ 

From this

$$\begin{bmatrix} AR_1 \\ AR_2 \end{bmatrix} = \begin{bmatrix} f_{11} & f_{12} \\ f_{21} & f_{22} \end{bmatrix}^{-1} \begin{bmatrix} DRS_1 - DRL_1 \\ DRS_2 - DRL_2 \end{bmatrix}$$

$$\begin{bmatrix} AR_1 \\ AR_2 \end{bmatrix} = EA \begin{bmatrix} 205.2 & 115.8 \\ 115.8 & 185.39 \end{bmatrix}^{-1} \begin{bmatrix} 0 - (5622.3) \\ 0 - (5622.3) \end{bmatrix} \times \frac{1}{EA}$$

$$\begin{bmatrix} AR_1 \\ AR_2 \end{bmatrix} = \begin{bmatrix} -15.88 \\ -20.41 \end{bmatrix}$$
 -ive sign shows that our assumed redundant action direction is wrong



Final determinate structure

**Step # 07:** Compute the member end actions. As we know that

[AM] = [AML] + [AMR][AR]



a) Compute AML values.



So AML values from the previous slide is

$[AML_1]$		∑ 26 <b>.</b> 67 <sup>-</sup>
$AML_2$		26.67
$AML_3$		26.67
$AML_4$		-33.33
$AML_5$	=	0
$AML_6$		0
$AML_7$		0
$AML_8$		-33.33
$L_{AML_9}$		-26.67 -

b) Compute the AMR values: when unit is applied at redundant location 1.



b) Compute the AMR values: when unit is applied at redundant location 2.


#### So AMR matrix

	$\Gamma AMR_{11}$	$AMR_{12}$		0.89	ך 0.44 כ	
$[AMR]_{9,2} =$	AMR <sub>21</sub>	AMR <sub>22</sub>	=	0.89	0.44	
	AMR <sub>31</sub>	AMR <sub>32</sub>		0.44	0.89	
	AMR <sub>41</sub>	AMR <sub>42</sub>		-0.55	-1.11	
	AMR <sub>51</sub>	$AMR_{52}$		0.33	0.67	
Φ.	AMR <sub>61</sub>	AMR <sub>62</sub>		-0.55	0.55	
	$AMR_{71}$	$AMR_{72}$		1	0	
	AMR <sub>81</sub>	AMR <sub>82</sub>		-1.11	-0.55	
	$L_{AMR_{91}}$	$AMR_{92}$		-0.44	-0.89	

Now member end actions

$[AM_1]$		ך 26.67		0.89	0.44		ך 3.50 ז
$AM_2$		26.67		0.89	0.44		3.50
$AM_3$		26.67		0.44	0.89		1.45
$AM_4$		-33.33		-0.55	-1.11	г 151/л	-1.81
$AM_5$	=	0	+	0.33	0.67	$\begin{vmatrix} -15.14\\ 20.41 \end{vmatrix} =$	-18.41
$AM_6$		0		-0.55	0.55	L-20.413	-2.50
$AM_7$		0		1	0		-15.88
$AM_8$		-33.33		-1.11	-0.55		-4.37
$AM_{9}$		-26.67		-0.44	-0.89		-1.45

To find the support reaction simply apply equilibrium equations and we will get



**Problem 03:** Analyze the given truss using flexibility method.



E.I =  $4 - 3 = 1^{\circ}$ I.I =  $8 + 3 - 2(5) = 1^{\circ}$ T.I = 2 degrees

S.I = 2 degrees So two redundant actions should be chosen.

• **Step # 01:** Identify the redundants and obtain BDS also compute [DRS] values.



Horizontal reaction at C & member force in member AE are chosen as redundant actions



Basic determinate structure (BDS) or Primary structure or Released structure

- **Step # 02:** Analyze the BDS (primary structure) under the following loading conditions
- i. Analyze the BDS when acted upon by the actual loads and compute the values of member forces  $P_i$ .
- ii. Analyze the BDS when acted upon by the unit load at redundant location 1 & compute the values of member forces  $U_{1i}$ .
- iii. Analyze the BDS when acted upon by the unit load at redundant location 2 & compute the values of member forces  $U_{2i}$ .



Now find all the member forces using method of joints & we will get



ii. BDS acted upon by the Unit load at redundant location  $1.(U_{1i} \text{ values})$ 



iii. BDS acted upon by the Unit load at redundant location  $2.(U_{2i} \text{ values})$ 



• **Step # 03:** Develop Member forces table.

Member	AE	Length, L <sub>i</sub> (ft)	P <sub>i</sub> values	U <sub>1i</sub> Values	U <sub>2i</sub> values
AB		20	10	-0.8	1
BC		20	18.34	0	1
CD	Constant	25	-22.92	0	0
DE		20	-18.34	-0.8	0
AD		15	-6.25	-0.6	0
BD		25	10.42	1	0
AE		25	0	1	0
BE		15	-6.25	-0.6	0

• **Step # 04:** Find BDS Displacements due to external loads or Compute the values of DRL matrix.

$$DRL_1 = \sum_{i=1}^m \frac{P_i U_{1i} Li}{EA_i}$$

$$DRL_{1} = \frac{1}{EA} \begin{bmatrix} (20 * 10 * -0.8) + 0 + 0 + (20 * -18.34 * -0.8) \\ + (15 * -6.25 * -0.6) + (25 * 10.42 * 1) \\ + 0 + (15 * -6.25 * -0.6) \end{bmatrix}$$

$$DRL_1 = \frac{506.44}{EA}$$

$$DRL_2 = \sum_{i=1}^m \frac{P_i U_{2i} Li}{EA_i}$$

$$DRL_{2} = \frac{1}{EA} \begin{bmatrix} (20 * 10 * 1) + (20 * -18.34 * 1)^{-1} \\ +0 + 0 + 0 + 0 + 0 + 0 + 0 \end{bmatrix}$$

$$DRL_2 = \frac{566.8}{EA}$$

$$DRL_{1} = \frac{506.44}{EA} \qquad DRL_{2} = \frac{566.8}{EA}$$
$$[DRL] = \begin{bmatrix} DRL_{1} \\ DRL_{2} \end{bmatrix} = \frac{1}{EA} \begin{bmatrix} 506.44 \\ 566.8 \end{bmatrix}$$

• **Step # 05:** Find BDS Displacements due to unit loads or Compute the values of flexibility coefficients

& flexibility matrix.

$$f_{11} = \sum_{i=1}^{m} \frac{U_{1i} U_{1i} L_i}{EA_i}$$

$$f_{11} = \frac{1}{EA} \begin{bmatrix} (20 * -0.8 * -0.8) + 0 + 0 + (20 * -0.8 * -0.8) \\ + (15 * -0.6 * -0.6) + (25 * 1 * 1) \\ + (25 * 1 * 1) + (15 * -0.6 * -0.6) \end{bmatrix}$$

$$f_{11} = \frac{86.4}{EA}$$

$$f_{12} = f_{21} = \sum_{i=1}^{m} \frac{U_{1i}U_{2i}L_i}{EA_i}$$

$$f_{12} = \frac{1}{EA} [(20 * -0.8 * 1) + 0 + 0 + 0 + 0 + 0 + 0 + 0]$$

$$f_{12} = f_{21} = \frac{-16}{EA}$$

$$f_{22} = \sum_{i=1}^{m} \frac{U_{2i} U_{2i} L_i}{EA_i}$$

 $f_{22} = \frac{1}{EA} [(20 * 1 * 1) + (20 * 1 * 1) + 0 + 0 + 0 + 0 + 0 + 0]$   $f_{22} = \frac{40}{EA}$   $[f] = \begin{bmatrix} f_{11} & f_{12} \\ f_{21} & f_{22} \end{bmatrix} \implies [f] = \frac{1}{EA} \begin{bmatrix} 86.4 & -16 \\ -16 & 40 \end{bmatrix}$ 

**Flexibility Method for Trusses Analysis Step # 06:** Apply compatibility and principle of superposition at the location of the removed redundants and solve the matrices to compute the values of redundant actions AR. As we know

 $[DRS] = [DRL] + [f] \cdot [AR]$  $[AR] = [f]^{-1} \cdot [DRS - DRL]$  $\begin{bmatrix}AR_{1}\\AR_{2}\end{bmatrix} = \begin{bmatrix}f_{11} & f_{12}\\f_{21} & f_{22}\end{bmatrix}^{-1} \begin{bmatrix}DRS_{1} - DRL_{1}\\DRS_{2} - DRL_{2}\end{bmatrix}$ 

From this

$$\begin{bmatrix} AR_1 \\ AR_2 \end{bmatrix} = \begin{bmatrix} f_{11} & f_{12} \\ f_{21} & f_{22} \end{bmatrix}^{-1} \begin{bmatrix} DRS_1 - DRL_1 \\ DRS_2 - DRL_2 \end{bmatrix}$$

$$\begin{bmatrix} AR1\\ AR2 \end{bmatrix} = EA \begin{bmatrix} 86.4 & -16\\ -16 & 40 \end{bmatrix}^{-1} \begin{bmatrix} 0 - (506.44)\\ 0 - (566.8) \end{bmatrix} \times \frac{1}{EA}$$

$$\begin{bmatrix} AR_1 \\ AR_2 \end{bmatrix} = \begin{bmatrix} -9.16 \\ -17.84 \end{bmatrix} \stackrel{\text{-iv}}{\text{red}}$$

-ive sign shows that our assumed redundant action direction is wrong



Final determinate structure

**Step # 07:** Compute the member end actions. As we know that

[AM] = [AML] + [AMR][AR]





b) Compute the AMR values: when unit is applied at redundant location 1.



b) Compute the AMR values: when unit is applied at redundant location 2.



So AMR matrix

	$\lceil AMR_{11} \rceil$	$AMR_{12}$		<b>−0.8</b>	ן1	
[ <i>AMR</i> ] <sub>9 * 2</sub> =	AMR <sub>21</sub>	$AMR_{22}$		0	1	
	$AMR_{31}^{}$	$AMR_{32}^{}$		0	0	
	$AMR_{41}$	$AMR_{42}^{2}$		-0.8	0	
	$AMR_{51}$	$AMR_{52}$		-0.6	0	
	$AMR_{61}$	$AMR_{62}^{\circ}$		1	0	
	$AMR_{71}$	$AMR_{72}$		1	0	
	$L_{AMR_{81}}$	AMR <sub>82</sub>		-0.6	0	

Now member end actions

$$\begin{bmatrix} AM_{1} \\ AM_{2} \\ AM_{3} \\ AM_{3} \\ AM_{4} \\ AM_{5} \\ AM_{6} \\ AM_{7} \\ AM_{8} \end{bmatrix} = \begin{bmatrix} 10 \\ 18.34 \\ -22.92 \\ -18.34 \\ -6.25 \\ 10.42 \\ 0 \\ -6.25 \end{bmatrix} + \begin{bmatrix} -0.8 & 1 \\ 0 & 1 \\ 0 & 0 \\ -0.8 & 0 \\ -0.6 & 0 \\ 1 & 0 \\ -0.6 & 0 \end{bmatrix} \begin{bmatrix} -9.16 \\ -17.84 \end{bmatrix} = \begin{bmatrix} -0.512 \\ 0.5 \\ -22.92 \\ -11.01 \\ -0.75 \\ 1.26 \\ -9.16 \\ -0.75 \end{bmatrix}$$

To find the support reaction simply apply equilibrium equations and we will get



Completely analyzed structure

# Flexibility Method for Trusses Analysis Assignment # 03(a)

**Problem 01:** Analyze the given truss using flexibility method ,The cross sectional areas of the members in square centimeters are shown in parenthesis. Assume  $E = 2 * 10^5 \text{ N/mm}^2$ 



#### References

- Structural Analysis by R. C. Hibbeler
- Matrix structural analysis by William Mc Guire
- Matrix analysis of frame structures by William Weaver
- Online Civil Engineering blogs