

# Hydraulic Engineering

M.16  
①

## MODULE # 1-(b)

### Froude Number "Fr"

It is a dimension less numbers used in a variety of ways with open channel flow.

It can be defined as

"The ratio of inertial forces to the gravitational forces".

Mathematically 
$$Fr = \frac{V}{\sqrt{gD_h}} = \frac{V}{\sqrt{gy}}$$

Squaring Both sides.

V = Mean Velocity of flow of water

$$Fr^2 = \frac{V^2}{gD_h}$$

D<sub>h</sub> = Hydraulic depth of the channel = y

$$Fr^2 = \frac{Q^2}{A^2 g D_h}$$

$$Q = AV$$

$$V = \frac{Q}{A}$$

$$Fr^2 = \frac{Q^2}{A^2 g} \times \frac{1}{D_h}$$

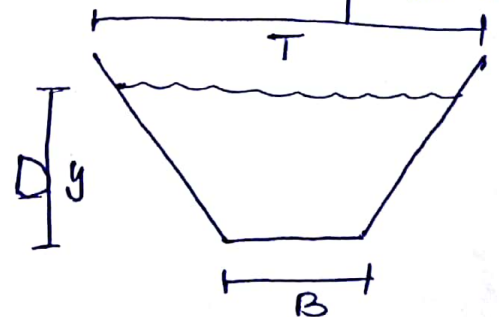
D<sub>h</sub> =  $\frac{\text{Area of flow}}{\text{Water surface width}}$

$$Fr^2 = \frac{Q^2}{A^2 g} \times \frac{T}{A}$$

$$D_h = \frac{A}{T} = \frac{1}{D_h} = \frac{T}{A}$$

$$\sqrt{Fr^2} = \sqrt{\frac{Q^2 T}{A^3 g}}$$

$$Fr = \sqrt{\frac{Q^2 T}{A^3 g}}$$



Condition:-

$Fr < 1$	Sub-critical flow	$y > y_c, E > E_{min}$
$Fr = 1$	Critical flow	$y = y_c, E = E_{min}$
$Fr > 1$	Super critical flow	$y < y_c, E < E_{min}$

Derive  $Fr = \frac{V}{\sqrt{gy}}$

As we know that  $E = y + \frac{q^2}{2gy^2}$

differentiate w-r-t "y"

$$\frac{d}{dy}(E) = \frac{d}{dy}\left(y + \frac{q^2}{2gy^2}\right)$$

$$\frac{dE}{dy} = \frac{d}{dy}(y) + \frac{d}{dy}\left(\frac{q^2}{2gy^2}\right)$$

$$\frac{dE}{dy} = 1 + \frac{q^2}{2g} \cdot \frac{d}{dy}\left(\frac{1}{y^2}\right)$$

$$\frac{dE}{dy} = 1 + \frac{q^2}{2g} \cdot \frac{d}{dy}(y^{-2})$$

$$\frac{dE}{dy} = 1 + \frac{q^2}{2g} (-2y^{-2-1})$$

$$\frac{dE}{dy} = 1 + \frac{q^2}{2g} (-2y^{-3})$$

$$\frac{dE}{dy} = 1 + \frac{q^2}{g} (-y^{-3})$$

$$\frac{dE}{dy} = 1 - \frac{q^2 y^{-3}}{g}$$

$$\boxed{\frac{dE}{dy} = 1 - \frac{q^2}{gy^3}} \rightarrow \text{eq ①}$$

As we know

$$Q = AV \rightarrow \text{①}$$

$$Q = qb \rightarrow \text{②}$$

equating ① and ②

$$AV = qb$$

$$by V = qb$$

$$yV = q$$

$$V = \frac{q}{y} \rightarrow \text{②}$$

$A = by$  (Rectangular section)

put  $V = \frac{q}{y}$  in eq ①

$$\frac{dE}{dy} = 1 - \frac{q^2}{y^2} \cdot \frac{1}{gy}$$

$$\frac{dE}{dy} = 1 - V^2 \cdot \frac{1}{gy} \quad \text{OR} \quad \boxed{\frac{dE}{dy} = 1 - \frac{V^2}{gy}} \rightarrow \text{③}$$

Condition :-

When there is a critical flow

$E = E_{min}$  so  $\frac{dE}{dy} = 0$

After apply Condition on eq. (3)

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$$0 = 1 - \frac{v^2}{gy}$$

$$\frac{v^2}{gy} = 1$$

$$\sqrt{v^2} = \sqrt{gy}$$

$$v = \sqrt{gy}$$

$$\frac{v}{\sqrt{gy}} = 1$$

At Critical Flow

$$Fr = 1$$

$$\boxed{Fr = \frac{v}{\sqrt{gy}}}$$

Hence proved.

## Critical depth $y_c$ for Rectangular Channel

Critical depth  $y_c$  is defined as the depth of flow of liquid at which the specific energy is minimum  $E_{min}$

$$E = y + \frac{q^2}{2gy^2}$$

Differentiating both side wrt  $y$

$$\frac{d(E)}{dy} = \frac{d}{dy}(y) + \frac{d}{dy}\left(\frac{q^2}{2gy^2}\right)$$

At critical depth

$$E = E_{min} \quad \text{so} \quad \frac{dE}{dy} = 0$$

$$0 = \frac{d}{dy}(y) + \frac{d}{dy}\left(\frac{q^2}{2gy^2}\right)$$

$$0 = 1 + \frac{q^2}{2g} \cdot \frac{d}{dy}\left(\frac{1}{y^2}\right)$$

$$0 = 1 + \frac{q^2}{2g} \cdot \frac{d}{dy}(y^{-2})$$

$$0 = 1 + \frac{q^2}{2g} \cdot -2(y^{-2-1})$$

$$0 = 1 - \frac{q^2}{2g} \times 2y^{-3}$$

$$0 = 1 - \frac{q^2}{gy^3}$$

$$\frac{q^2}{gy^3} = 1$$

$$\frac{q^2}{g} = y^3$$

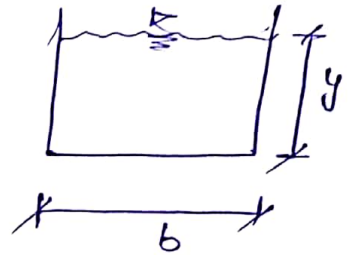
$$y_c = \left(\frac{q^2}{g}\right)^{\frac{1}{3}} \quad \text{proved.}$$

Problem:-

What is Smallest energy that can be associated with  $q = \frac{10 \text{ ft}^2}{\text{sec}}$

Solution:-

let suppose the channel is rectangular



Condition for Smallest Energy

$$y = y_c, \quad E = E_{\min}, \quad Fr = 1$$

As we know that

$$y_c = \left( \frac{q^2}{g} \right)^{1/3}$$

$$y_c = \left( \frac{10^2}{32.17} \right)^{1/3}$$

$$y_c = 1.458 \text{ ft}$$

$$g = 9.8 \text{ m/sec}^2$$

$$g = 9.8 \times 3.28$$

$$\therefore g = 32.17 \frac{\text{ft}}{\text{sec}^2}$$

Also  $Q = AV \rightarrow \textcircled{1}$

$$Q = qb \rightarrow \textcircled{2}$$

equating  $\textcircled{1}$  &  $\textcircled{2}$

$$AV = qb$$

$$\cancel{by} \quad v = \cancel{q/b}$$

$$yv = q$$

$$v = \frac{q}{y_c} = \frac{10}{1.458} \Rightarrow$$

$$v = 6.86 \frac{\text{ft}}{\text{sec}}$$

$$E = y + \frac{v^2}{2g}$$

$$E_{min} = y_c + \frac{v^2}{2g}$$

$$E_{min} = 1.458 + \frac{(6.86)^2}{2(32.17)}$$

$$E_{min} = 1.458 + 0.73$$

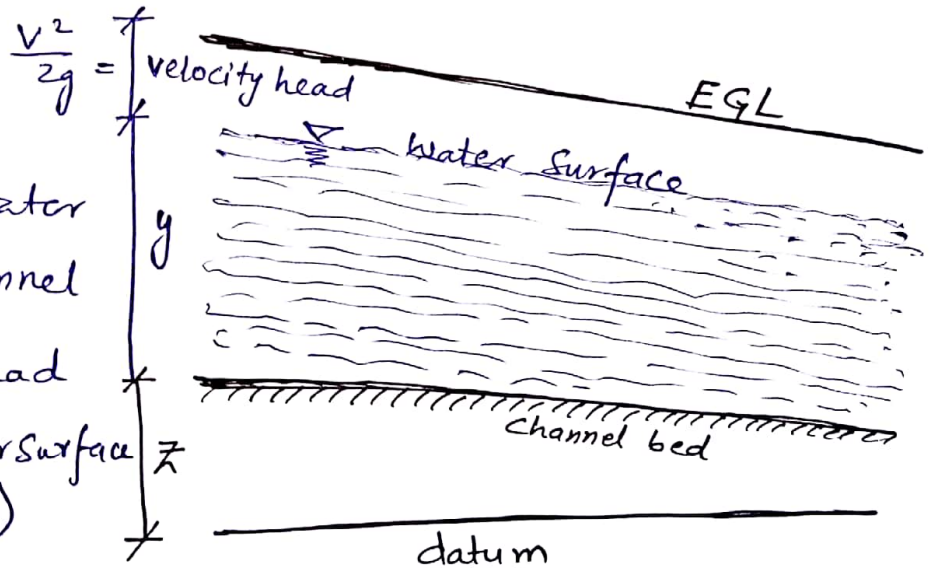
$$E_{min} = 2.217t$$

## Dynamic Equation of Gradually Varied Flow:-

$z$  = height from datum to channel bed.

$y$  = depth of water in open channel

$\frac{v^2}{2g}$  = Velocity head (from water surface to EGL)



EGL = Energy Grade line.

The total head "H" at any section is given by

$$H = z + y + \frac{v^2}{2g} \rightarrow eq \textcircled{1}$$

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Differentiating (1) with respect to horizontal distance on both sides.

$$\frac{d}{dx}(H) = \frac{d}{dx}(z) + \frac{d}{dx}(y) + \frac{d}{dx}\left(\frac{v^2}{2g}\right) \rightarrow \text{eq (2)}$$

(a)  $\frac{dH}{dx}$  = The slope of the energy line =  $-S_f$

(b)  $\frac{dz}{dx}$  = The channel bed slope =  $-S_0$

put (a) and (b) in eq (2)

$$-S_f = -S_0 + \frac{dy}{dx} + \frac{d}{dx}\left(\frac{v^2}{2g}\right)$$

$$S_0 - S_f = \frac{dy}{dx} + \frac{d}{dx}\left(\frac{v^2}{2g}\right) \rightarrow \text{eq (3)}$$

Multiplying and dividing dy with Velocity Term in eq (3).

$$S_0 - S_f = \frac{dy}{dx} + \frac{d}{dx} \times \frac{dy}{dy} \left(\frac{v^2}{2g}\right)$$

$$S_0 - S_f = \frac{dy}{dx} \left[ 1 + \frac{d}{dy} \left(\frac{v^2}{2g}\right) \right]$$

$$\frac{dy}{dx} = \frac{S_0 - S_f}{1 + \frac{d}{dy} \left(\frac{v^2}{2g}\right)} \rightarrow \text{eq (x)}$$

The above eq (x) is known as the dynamic equation of gradually varied flow.



# Surface Profiles and Back Water Curves

## Classification of channel Bed slopes:-

The slope of the channel bed is very important in determining the characteristics of the flow.

let

$S_0$  = The slope of the channel bed

$S_c$  = Critical slope

$y_n$  = The normal depth of water

The slope of the channel bed can be classified as:-

① Critical slope :- The bottom slope of the channel is equal to critical slope

$$S_0 = S_c \text{ or } y_n = y_c$$

② Mild slope :- The bottom slope of the channel is less than the critical slope.

$$S_0 < S_c$$

③ Steep slope :- The bottom slope of the channel is greater than the critical slope

$$S_0 > S_c$$

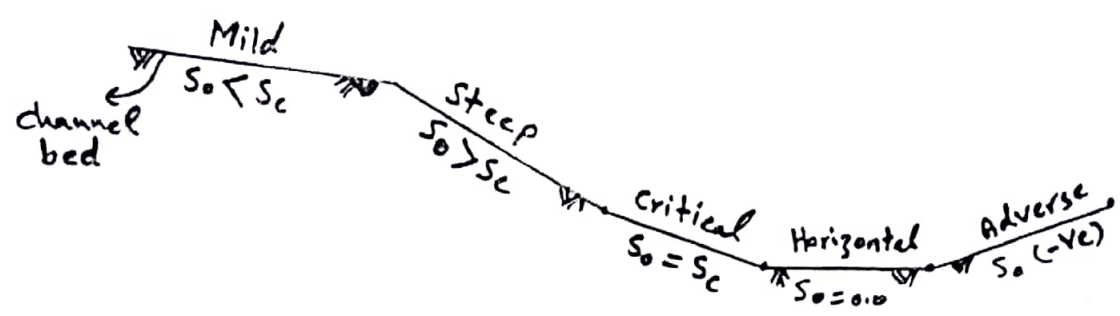
④ Horizontal slope :- the bottom slope of the channel is equal to zero

$$S_0 = 0$$

### ⑤ Adverse slope :-

The slope opposite to the direction of flow .

$$S_0 = \text{negative}$$



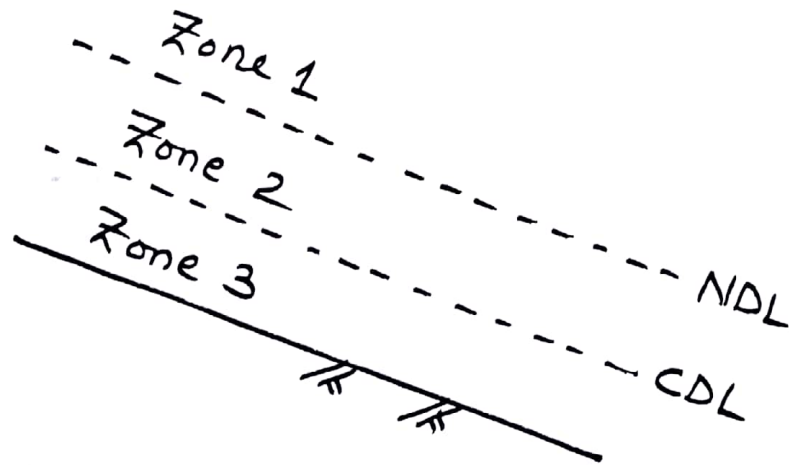
## Classification of flow Profiles (Water Surface Profile)

- \* The Surface Curves of water are called flow profiles (or water surface profiles)
- \* The shape of water surface profiles is mainly determined by the slope of the channel bed  $S_0$ .

For a given discharge, the normal depth  $y_n$  and the critical depth  $y_c$  may be calculated.

The following steps are followed to classify the flow profiles :-

- 1- A line parallel to the channel bottom with a height of  $y_n$  is drawn and is designated as the normal depth line (N.D.L)
- 2- A line parallel to the channel bottom with a height  $y_c$  is drawn and is designated as the critical depth line (C.D.L)
- 3- The vertical space in a longitudinal section is divided into 3 zones using the two lines drawn in step 1 and 2

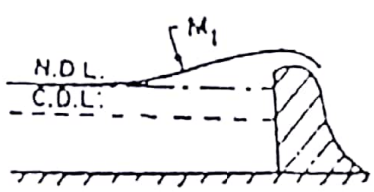


- 4- Depending upon the zone and the slope of the bed channel, the water profiles are classified into 13 types as follows:-

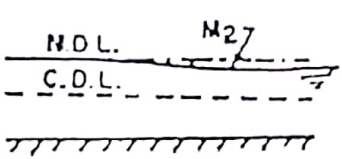
- (a) Mild slope Curves  $M_1, M_2, M_3$
- (b) Steep slope Curves  $S_1, S_2, S_3$
- (c) Critical slope Curves  $C_1, C_2, C_3$
- (d) Horizontal Slope Curves  $H_2, H_3$
- (e) Adverse Slope Curves  $A_2, A_3$

In all these curves the letter indicates the slope type and the subscript indicates the zone

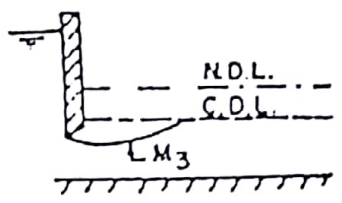
Example :  $S_2$  Curve occurs in the zone 2 of the steep slope.



(a)

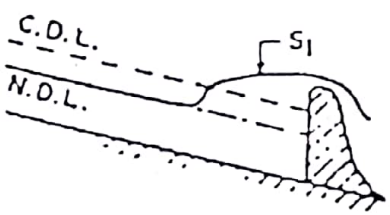


(b)

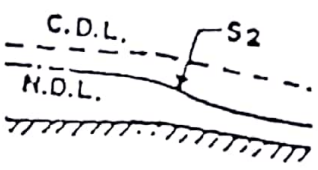


(c)

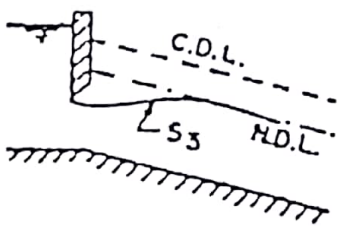
Flow Profiles in Mild slope



(a)

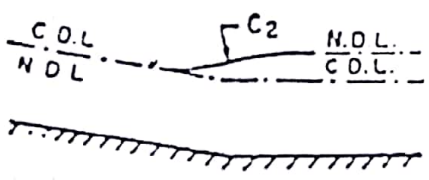


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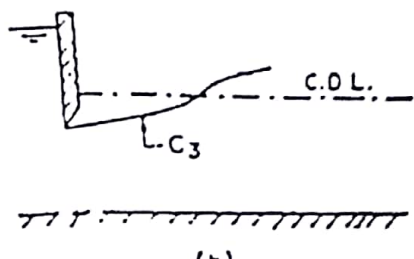


(c)

Flow Profiles in Steep slope

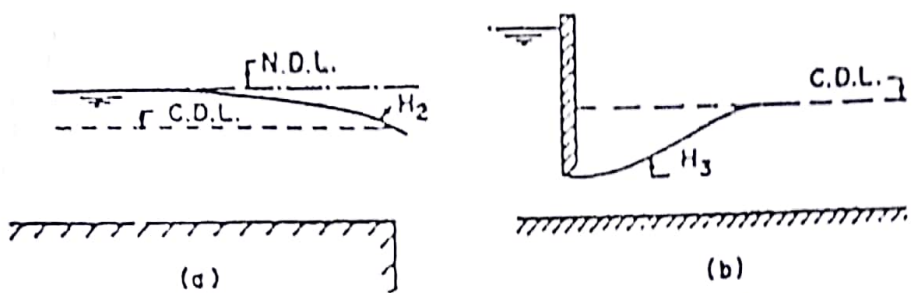


(a)

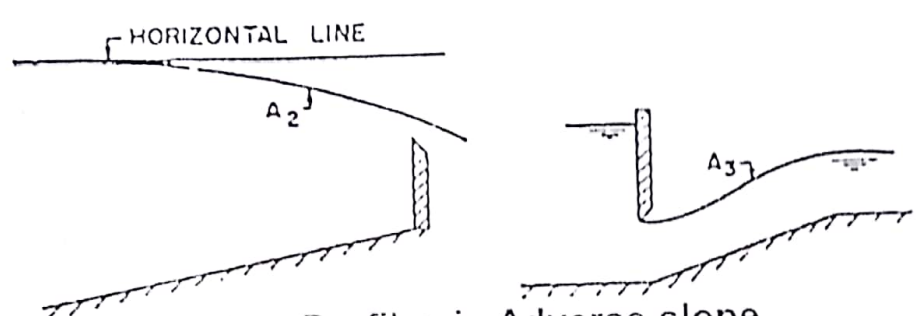


(b)

Flow Profiles in Critical slope



Flow Profiles in Horizontal slope



Flow Profiles in Adverse slope

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