

NAME ≈ Junaid Khan

ID No ≈ 7766

Section ≈ C

instructor ≈ Eng - Sagib Khan

Department ≈ BE civil

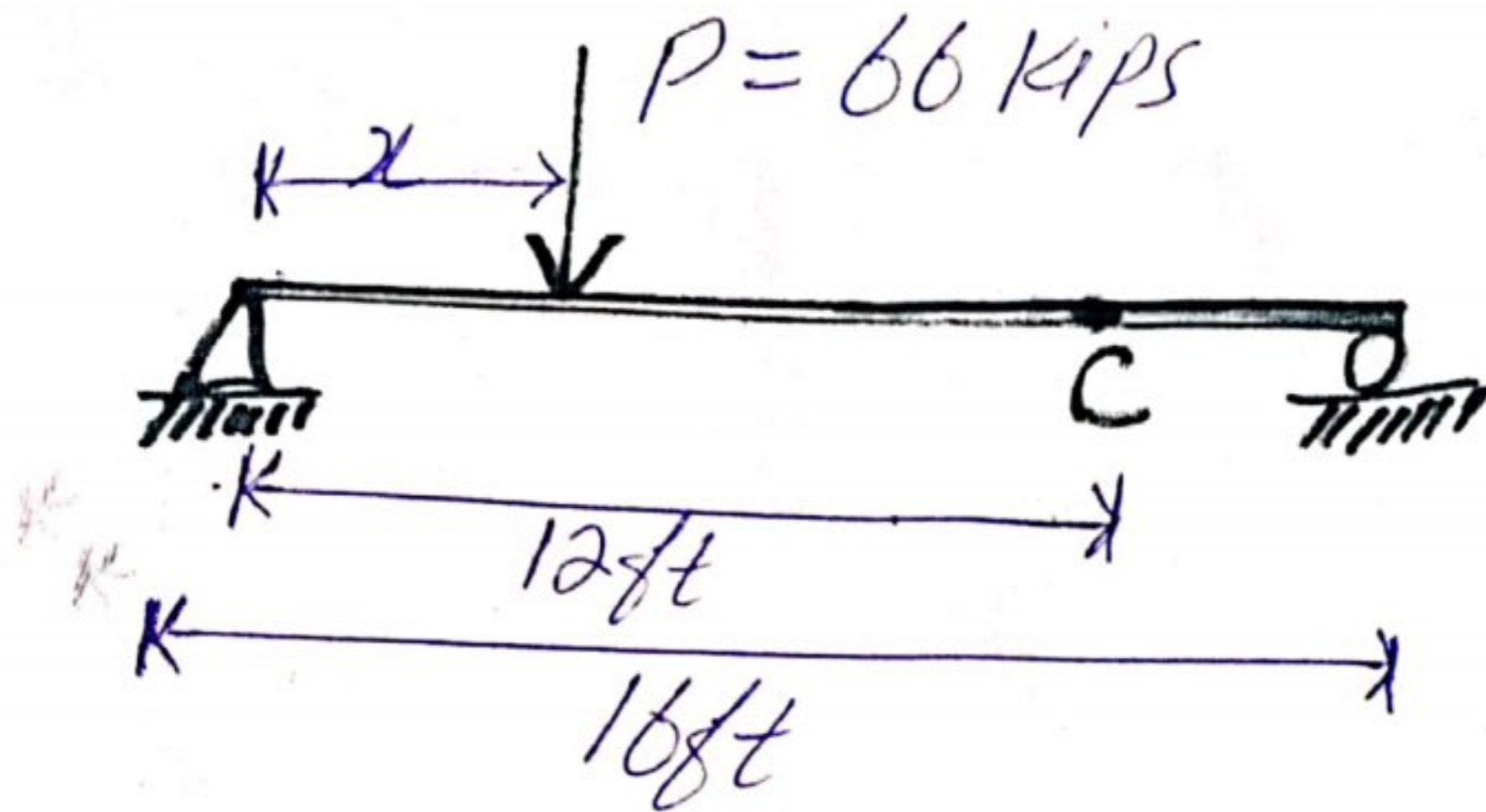
Date ≈ 26/9/2020



"Ans#03:.

(1)

influence line:

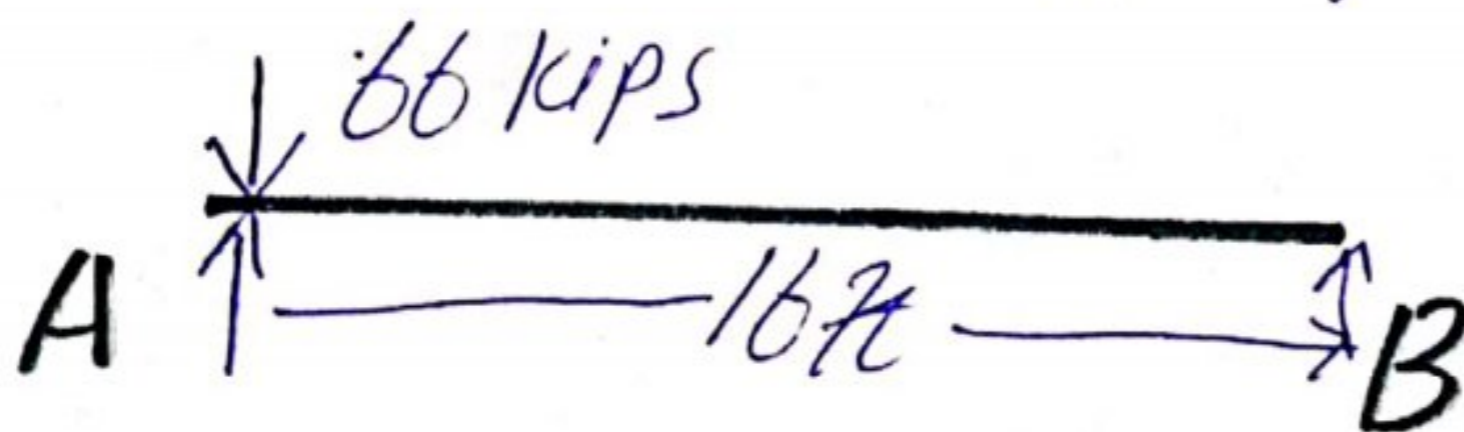


- * Shear influence line at point "C" = ?
- * influence line for reaction "A" = ?

Sol:-

influence line for Reaction "A";

For $x=0$, $R_A = ?$



$$\sum M_B = 0$$

$$(66)(16) - R_A(16) = 0$$

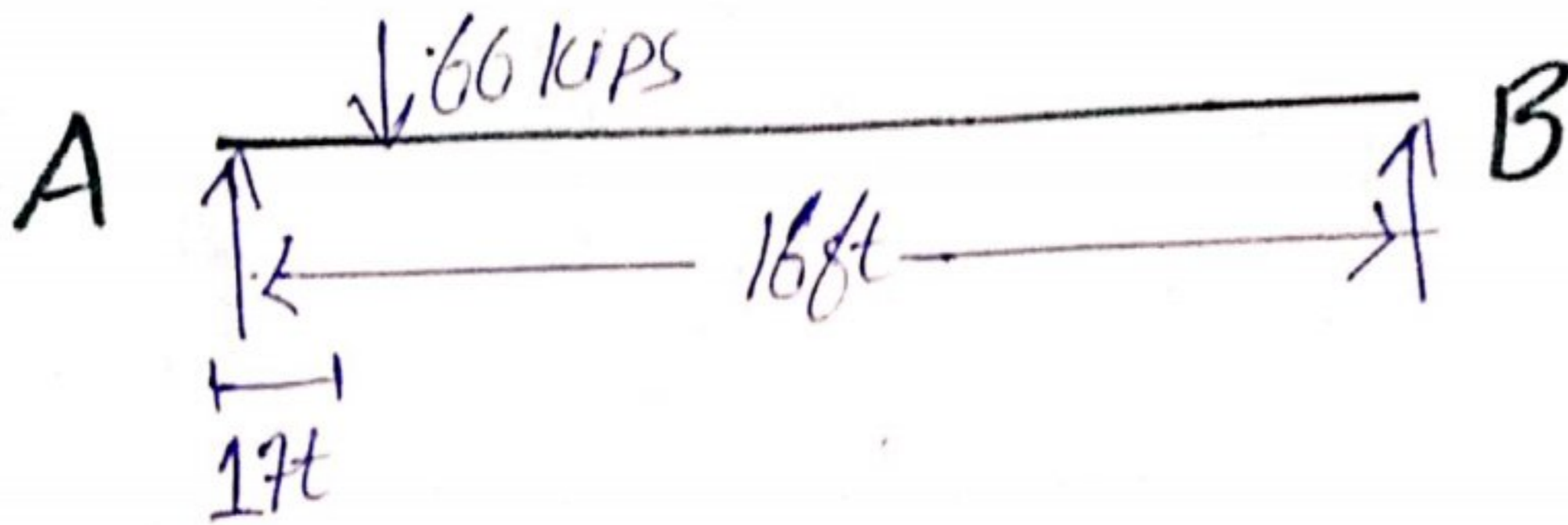
$$16R_A = 1056$$

$$\boxed{R_A = 66 \text{ kips}}$$

For $x = 1\text{ft}$

(2)

$R_A = ?$



$$\sum M_B = 0$$

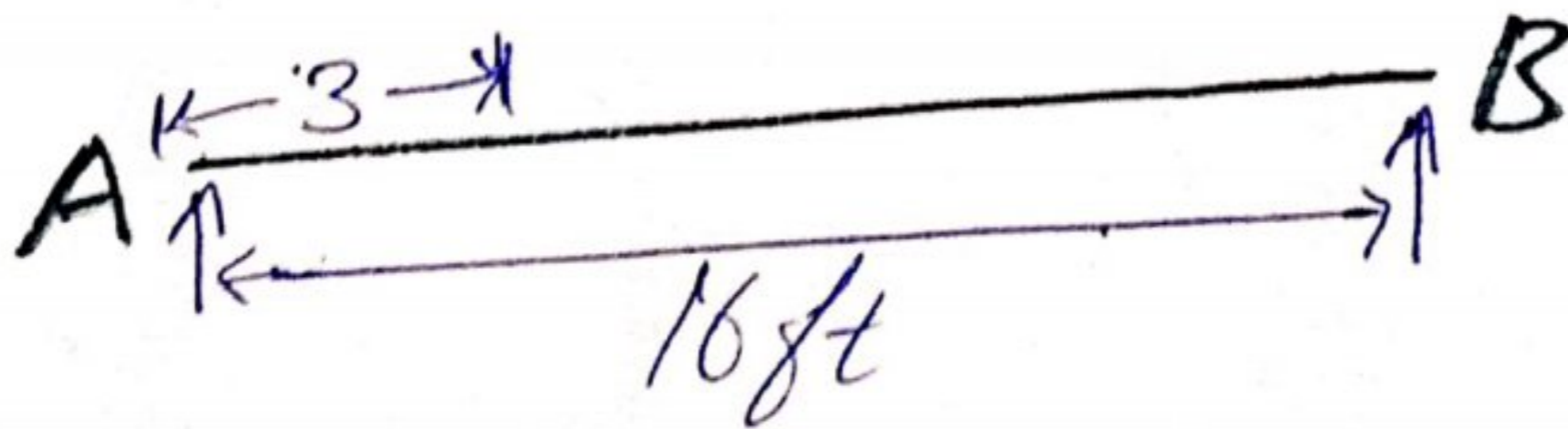
$$(66)(15) - R_A(16) = 0$$

$$990 - 16R_A = 0$$

$$16R_A = 990$$

$$R_A = 61.87 \text{ kips}$$

For $x = 3\text{ft}$:



$$\sum M_B = 0$$

$$(66 \times 13) - R_A(16) = 0$$

$$858 - 16R_A = 0$$

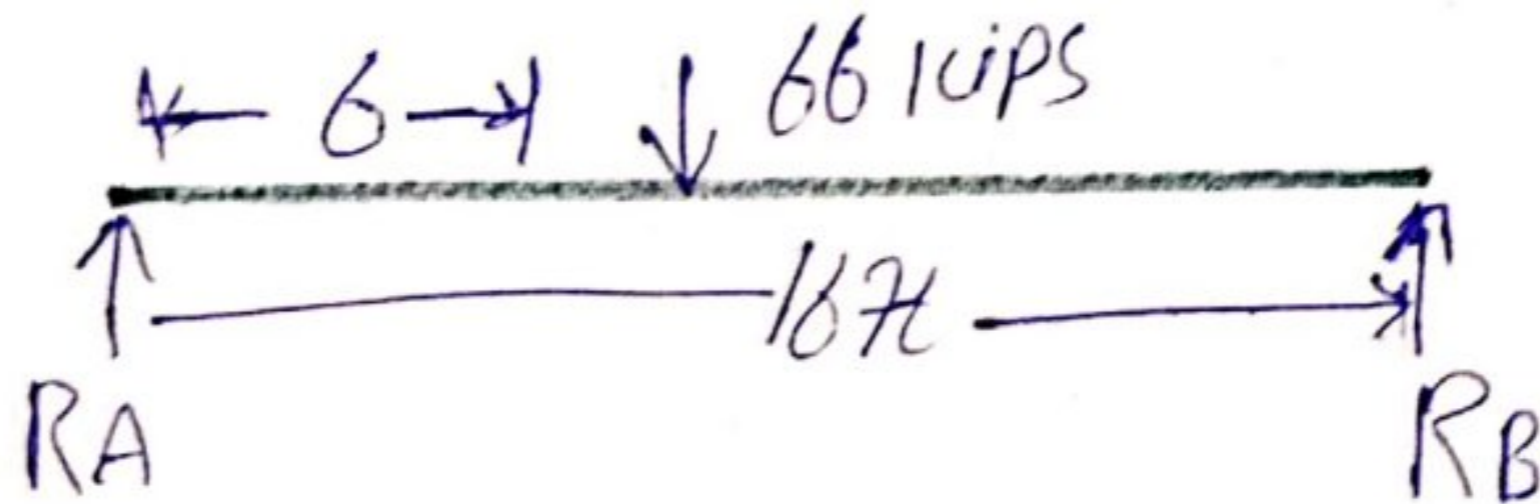
$$R_A = 53.625$$

$$16 RA = 858 \quad (3)$$

$$RA = 53.62 \text{ kips}$$

For $x = 6\text{ft}$:

$$RA = ?$$



$$\sum M_B = 0$$

$$(66 \times 10) - RA(16) = 0$$

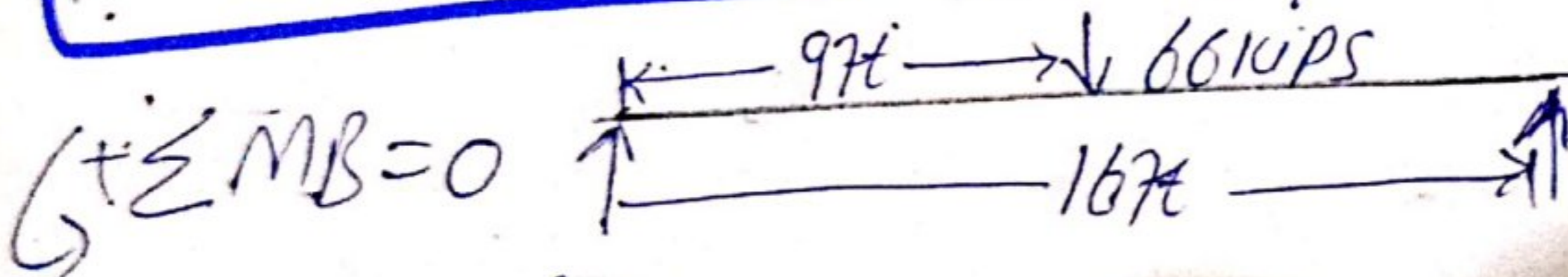
$$660 - 16RA = 0$$

$$16RA = 660$$

$$RA = 41 \text{ kips}$$

For $x = 9\text{ft}$:

$$RA = ?$$



$$\sum M_B = 0$$

$$(66 \times 7) - RA(16) = 0$$

$$462 - 16RA = 0$$

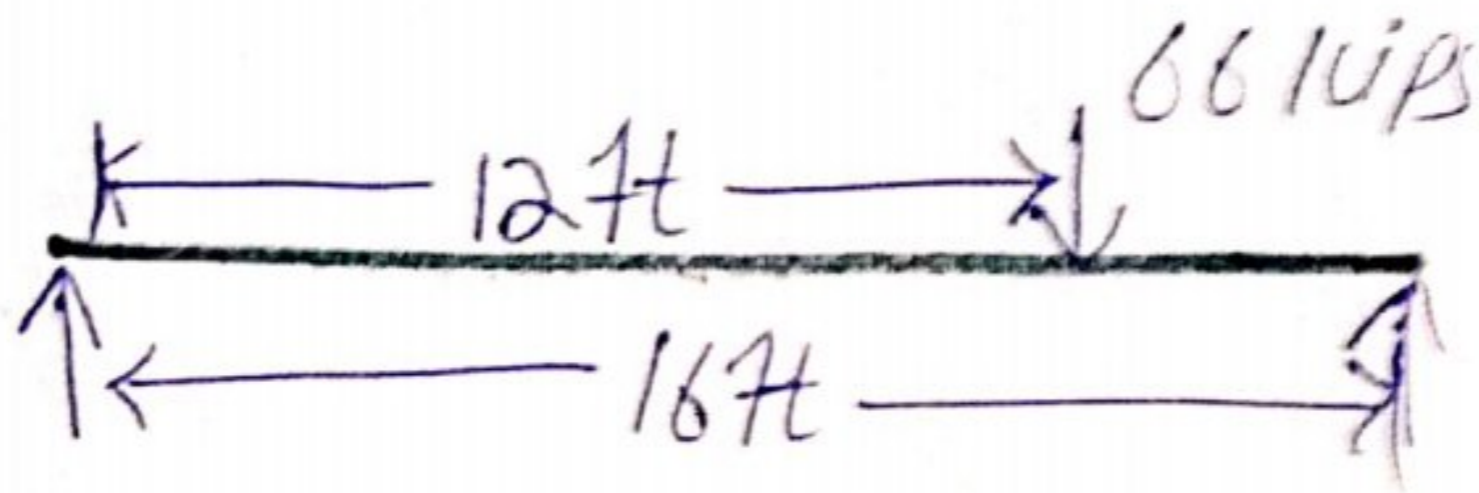
$$16RA = 462$$

$$RA = 28.87$$

For $x = 12 \text{ ft}$

(4)

$RA = ?$



$$\sum M_B = 0$$

$$(66 \times 4) - RA(16) = 0$$

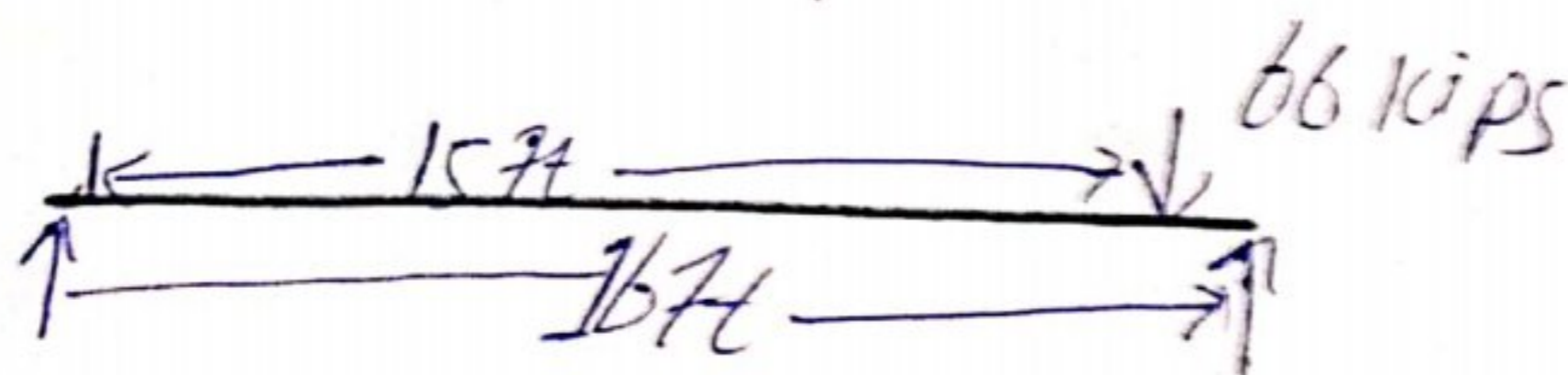
$$264 - 16RA = 0$$

$$16RA = 264$$

$$RA = 16.5 \text{ kips}$$

For $x = 15 \text{ ft}$

$RA = ?$



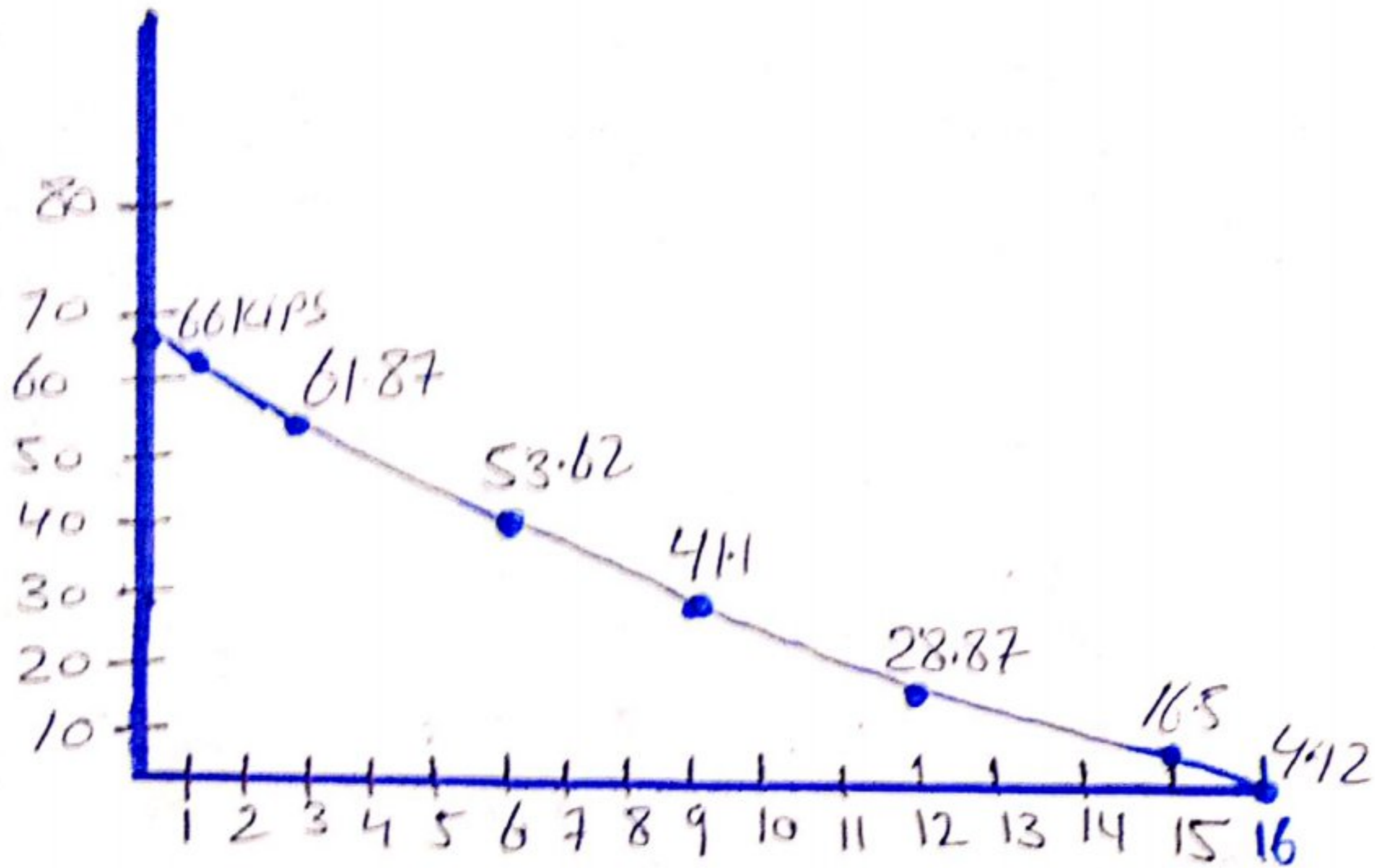
$$(66 \times 1) - RA(16) = 0$$

$$66 - 16RA = 0$$

$$16RA = 66$$

$$RA = 4.12 \text{ kips}$$

(5)

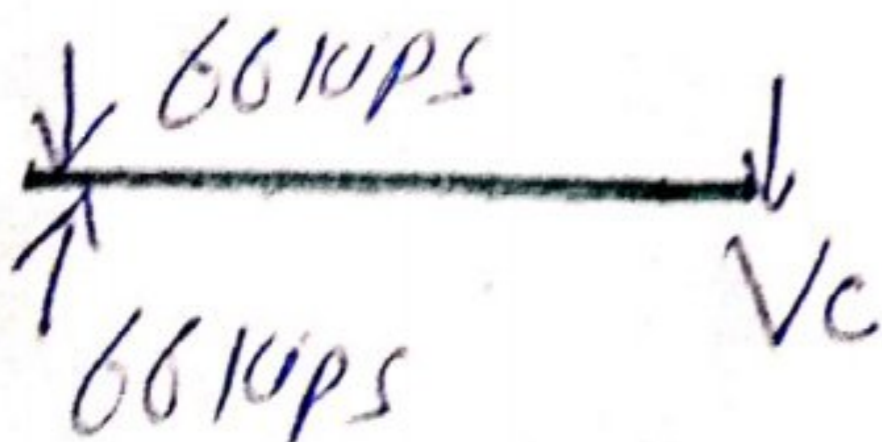


∴ influence line of RA:

Now:

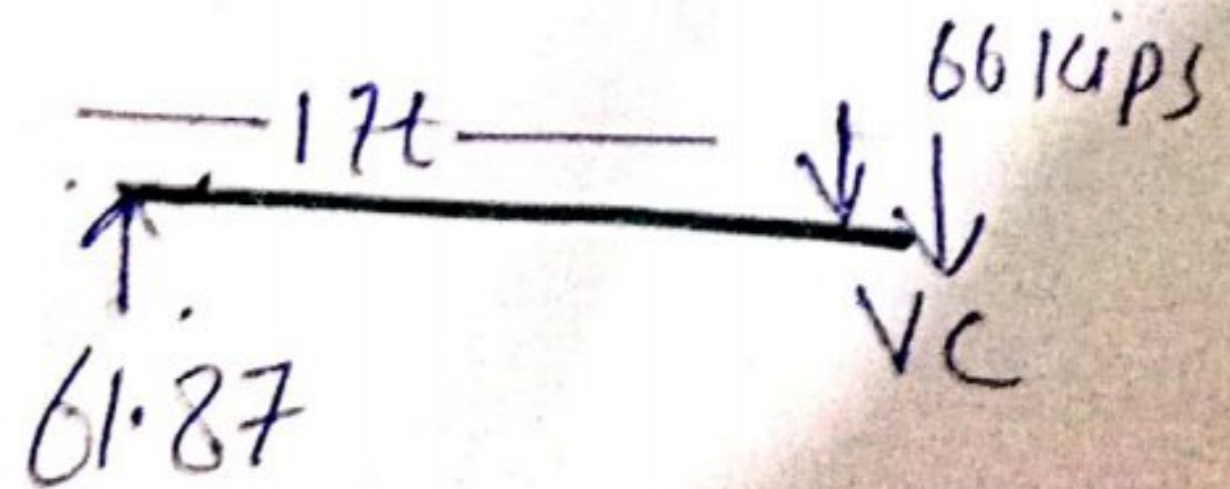
Shear influence line at point "C"

⇒ For $x=0$ $V_C = ?$



$$\uparrow \sum F_y = 0$$
$$RA - 66 - V_C = 0$$

⇒ For $x=17t$ $V_C = ?$



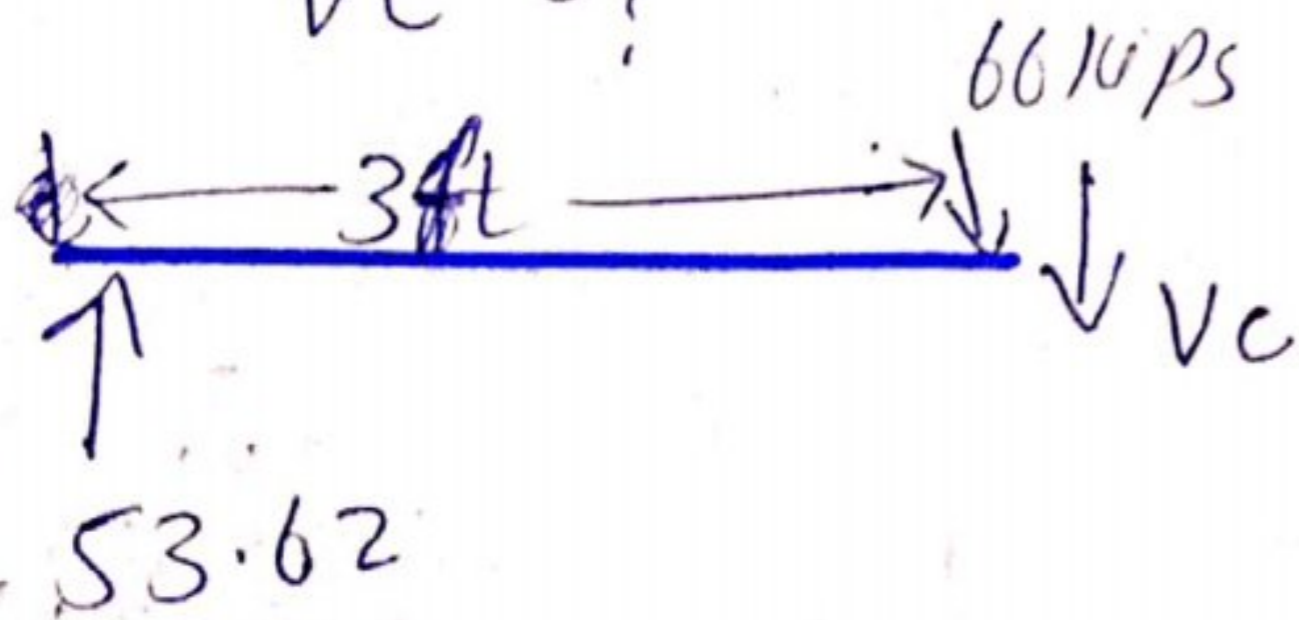
$$\uparrow \sum F_y = 0$$
$$RA - P - V_C = 0$$

$$66 - 66 - V_C = 0$$

$$V_C = 0$$

⇒ For $x = 37t$

$$V_C = ?$$



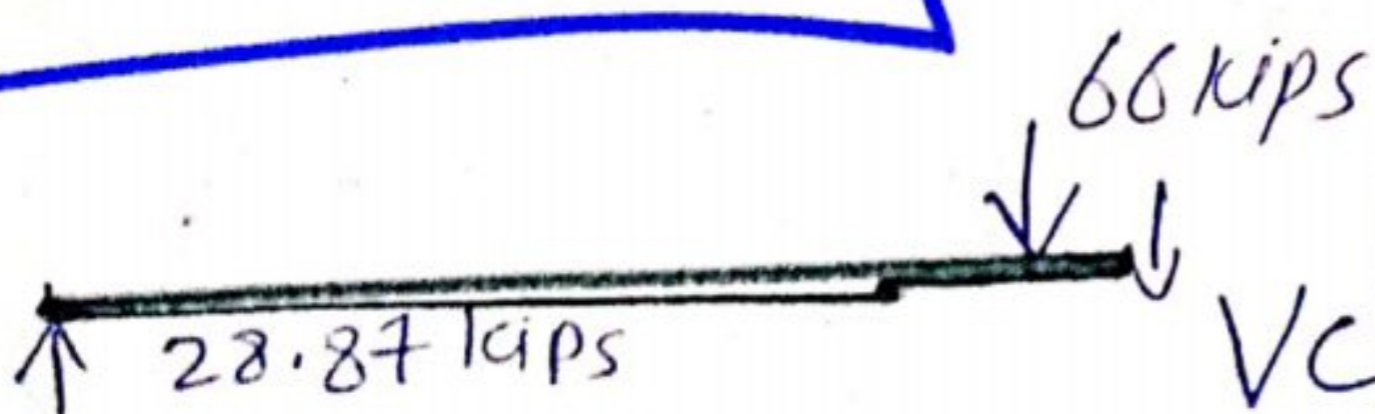
$$\uparrow \sum F_y = 0$$

$$R_A - P - V_C = 0$$

$$53.62 - 66 - V_C = 0$$

$$V_C = -12.38$$

For $x = 97t$



$$\uparrow \sum F_y = 0$$

$$R_A - P - V_C = 0$$

$$28.87 - 66 - V_C = 0$$

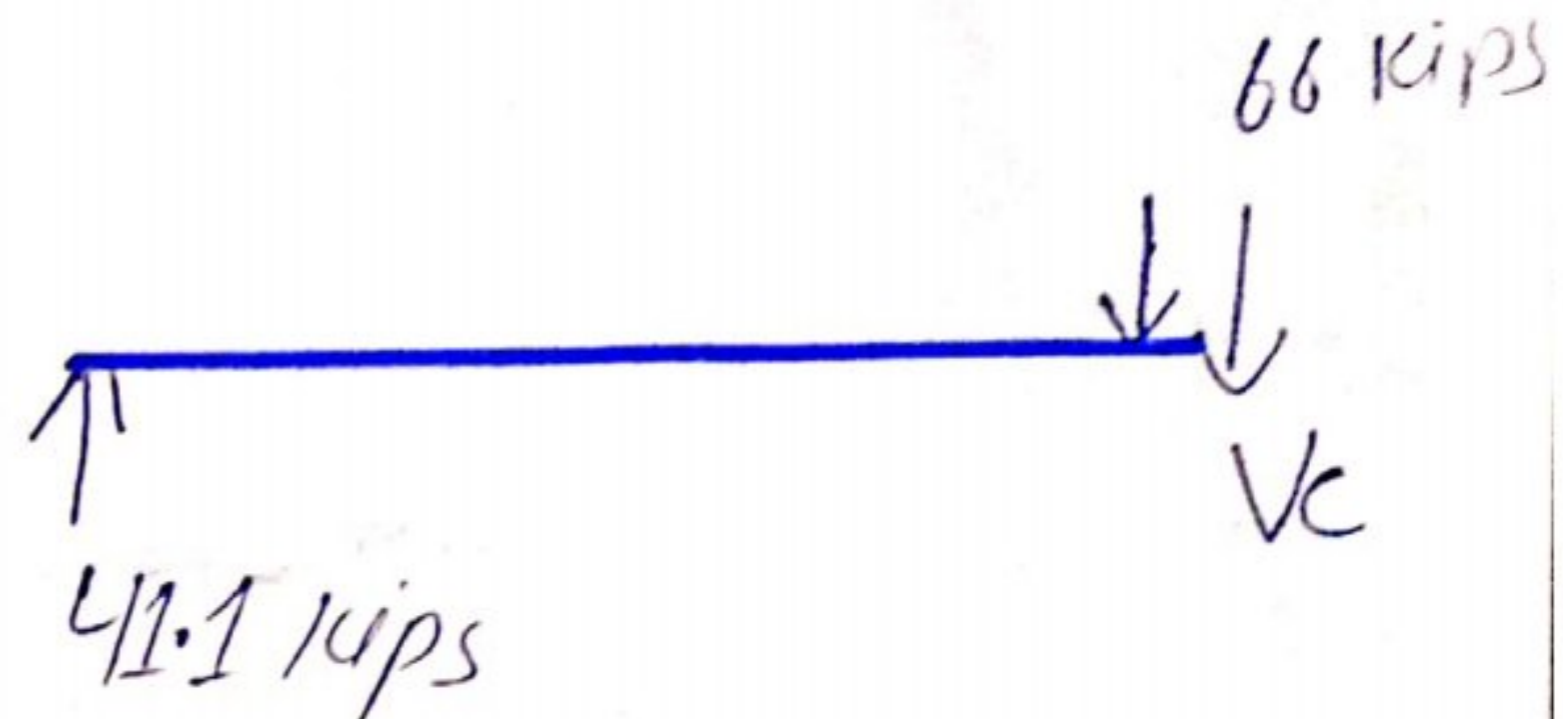
$$V_C = -37.13$$

(6)

$$61.87 - 66 - V_C = 0$$

$$V_C = -4.13$$

⇒ For $x = 67t$



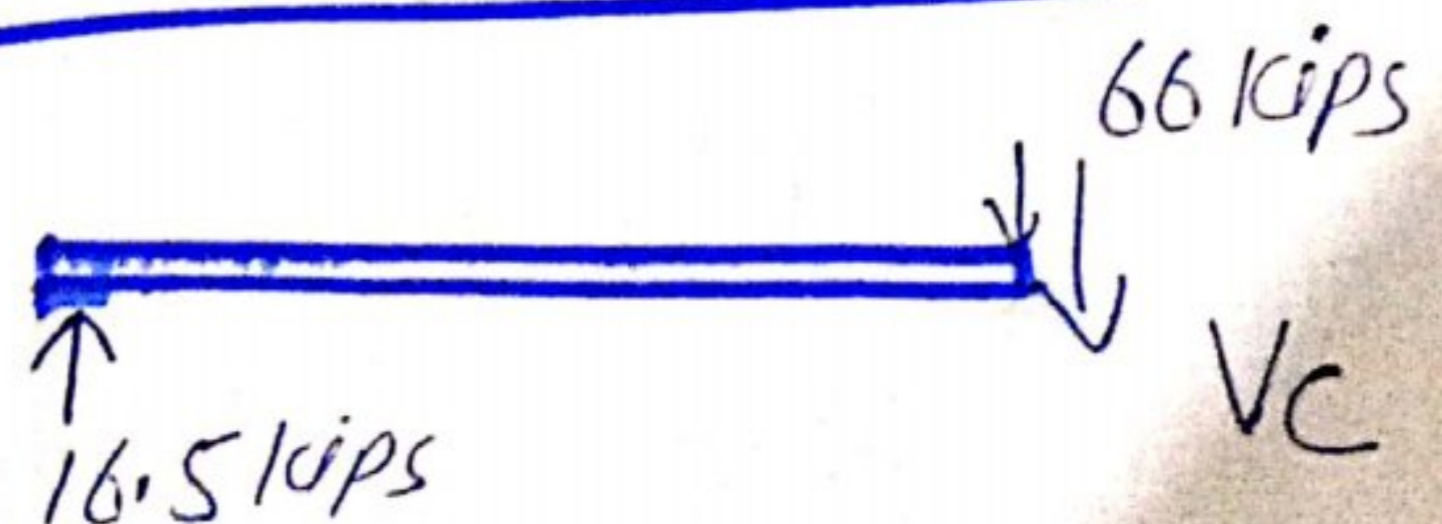
$$\uparrow \sum F_y = 0$$

$$R_A - P - V_C = 0$$

$$41.1 - 66 - V_C = 0$$

$$V_C = -24.9$$

⇒ For $x = 127t$



$$\uparrow \sum F_y = 0$$

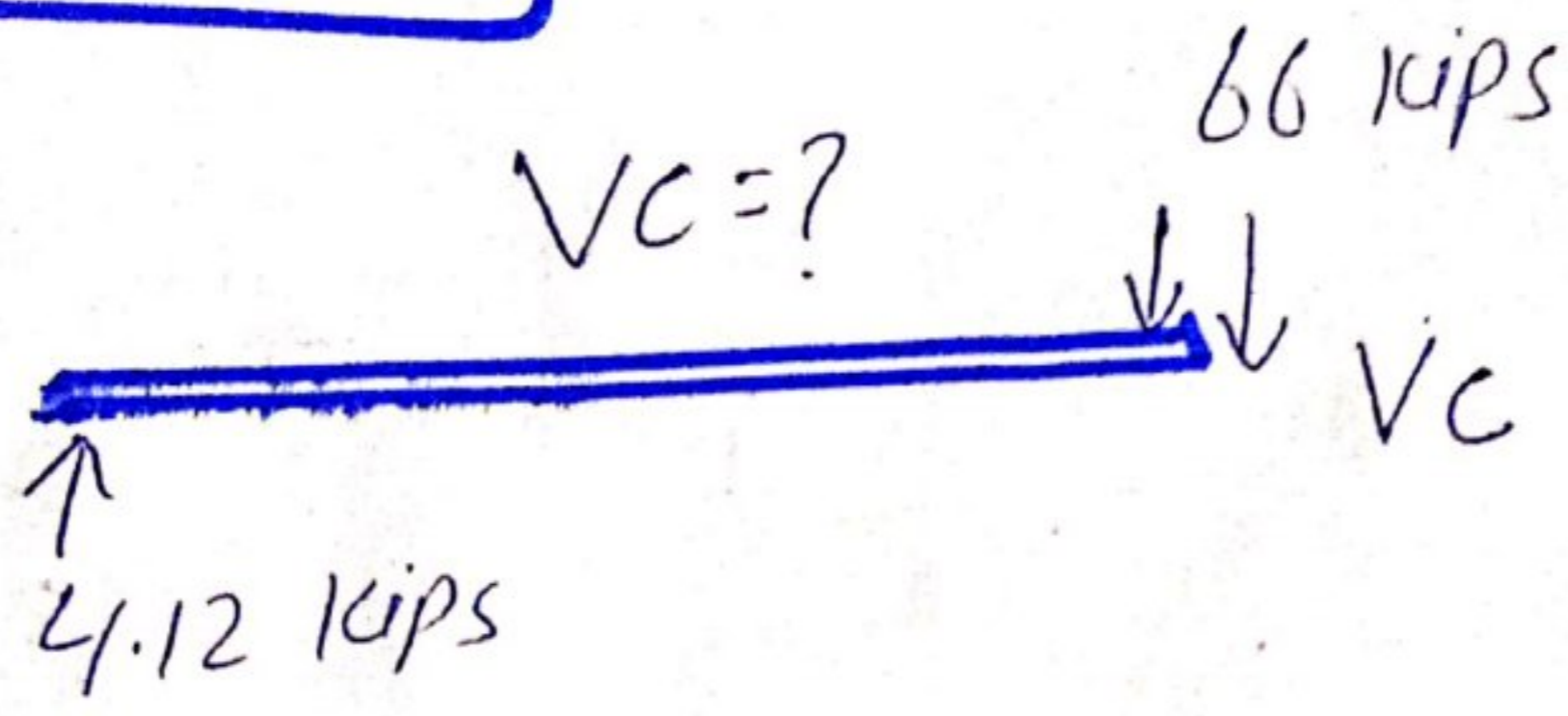
$$R_A - P - V_C = 0$$

$$16.5 - 66 - V_C = 0$$

$$V_C = -49.5$$

For $x = 157t$

(7)

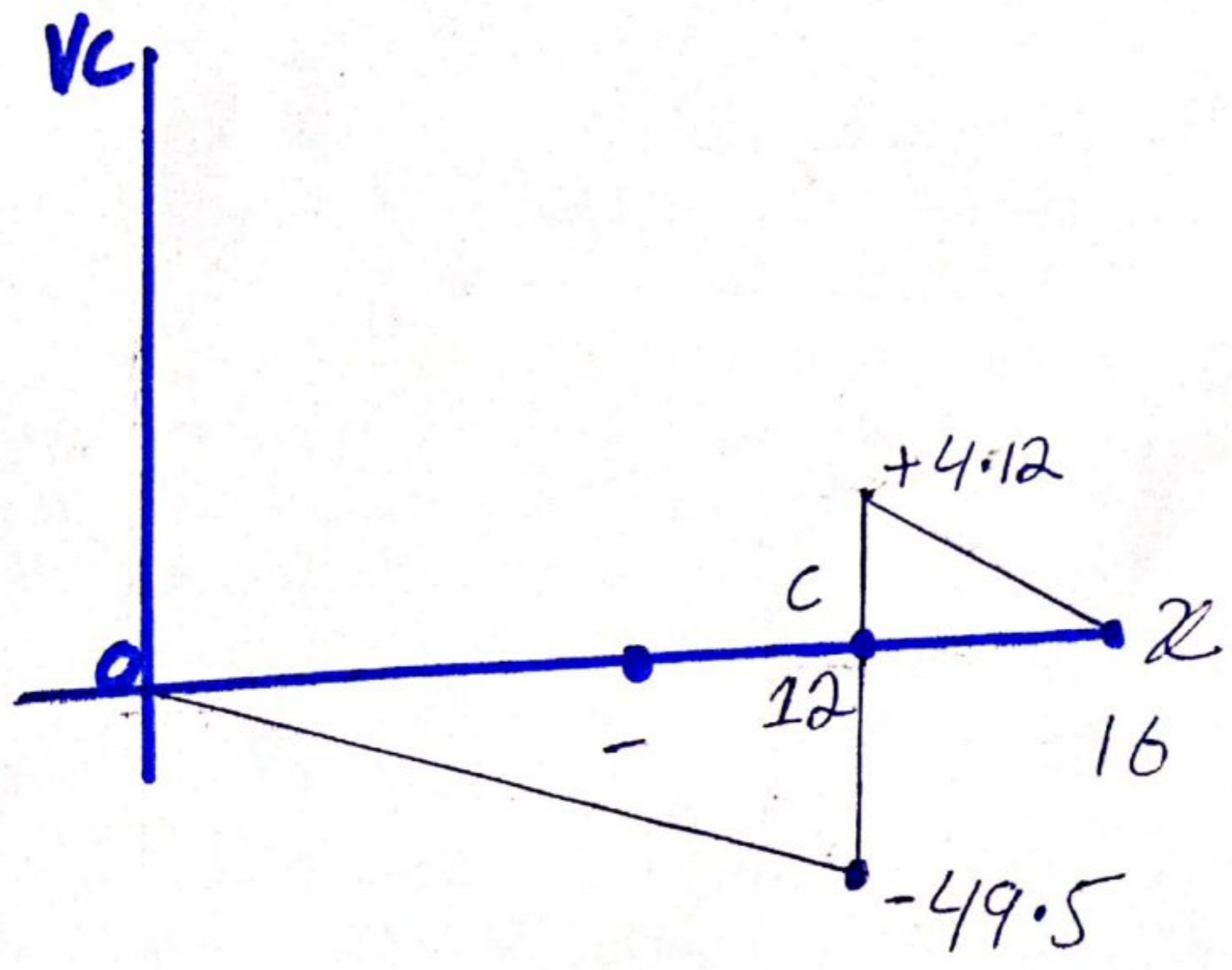


$$\uparrow \sum F_y = 0$$

$$R_A - V_C = 0$$

$$4.12 - V_C = 0$$

$V_C = 4.12$



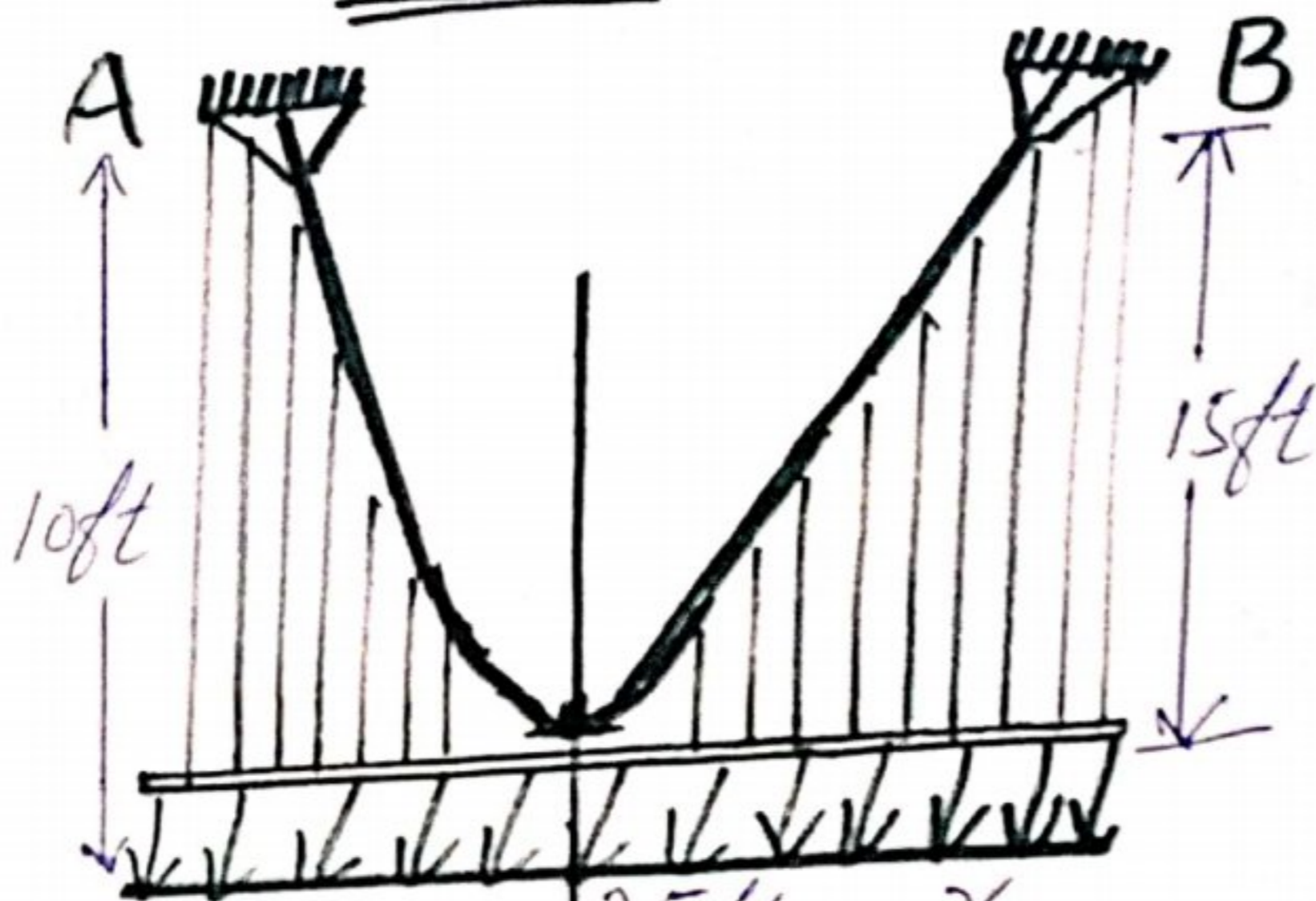
Shear influence line at point "C"



2
ANS#02 =

(U)

Cable:



* Cable supports uniform load = 872 lb/ft

Determine the tension in the cable at;

support A = ?

support B = ?

Sol.:

$$y = \frac{w_0 x^2}{2FH}$$

By putting values;

$$15 = \frac{766 x^2}{2FH} \quad \text{--- (i)}$$

(2)

$$10 = \frac{766}{2FH} (25-x)^2 \quad \text{--- (ii)}$$

By solving both equations:

$$FH = \frac{766}{2(15)} x^2, \quad FH = \frac{766}{2(10)} (25-x)^2$$

Now:

$$FH = FH$$

$$\frac{766}{2(15)} x^2 = \frac{766}{2(10)} (25-x)^2$$

$$25.533x^2 = 38.3 (25-x)^2$$

$$x^2 = \frac{38.3}{25.533} (25-x)^2$$

$$x^2 = 1.5001 (625 - 50x + x^2)$$

$$x^2 = 1.50 (625 - 50x + x^2)$$

$$0.5 - 75x + 937.50 = 0 \quad \text{--- (i)}$$

Now choose root ≤ 25 ft

By solving equation (i)

$$x = 13.76 \text{ ft}$$

Now:

$$25 - 13.76 = 11.25 \text{ ft}$$

As;

$$FH = \frac{766}{2(15)} x^2 = \frac{766}{2(15)} (13.76)^2 = 4834 \text{ lb. (A)}$$

$$FH' = \frac{766}{2(10)} (25-x)^2 = \frac{766}{20} (11.25)^2 = 4847 \text{ lb. (B)}$$

Support # B

$$y = \frac{w_0}{2FH} x^2 = \frac{766}{2(4834)} (13.76)^2 = 15.001$$

Now,

$$\frac{dy}{dx} = \tan \theta_B = 15.001$$

$$\theta_B = \tan^{-1} (15.001)$$

$$= \boxed{\theta_B = 86.18^\circ} \quad (4)$$

we have;

Tension at B;

$$T_B = \frac{F_H}{\cos \theta_B} = \frac{4834}{\cos(86.18)} = 72558.40 \text{ lb}$$
$$= \boxed{72.558 \text{ kips}}$$

Support: A

$$y = \frac{w_0}{2F_H} x^2 = \frac{766}{2(4834)} (25-x)^2 = \frac{766}{2(4834)} (11.25^2)$$

$$\frac{dy}{dx} = \tan \theta_A = 9.902$$

$$\boxed{y = 9.902}$$

$$\theta_A = \tan^{-1}(9.902) = \theta_A = \boxed{84.23}$$

Now:

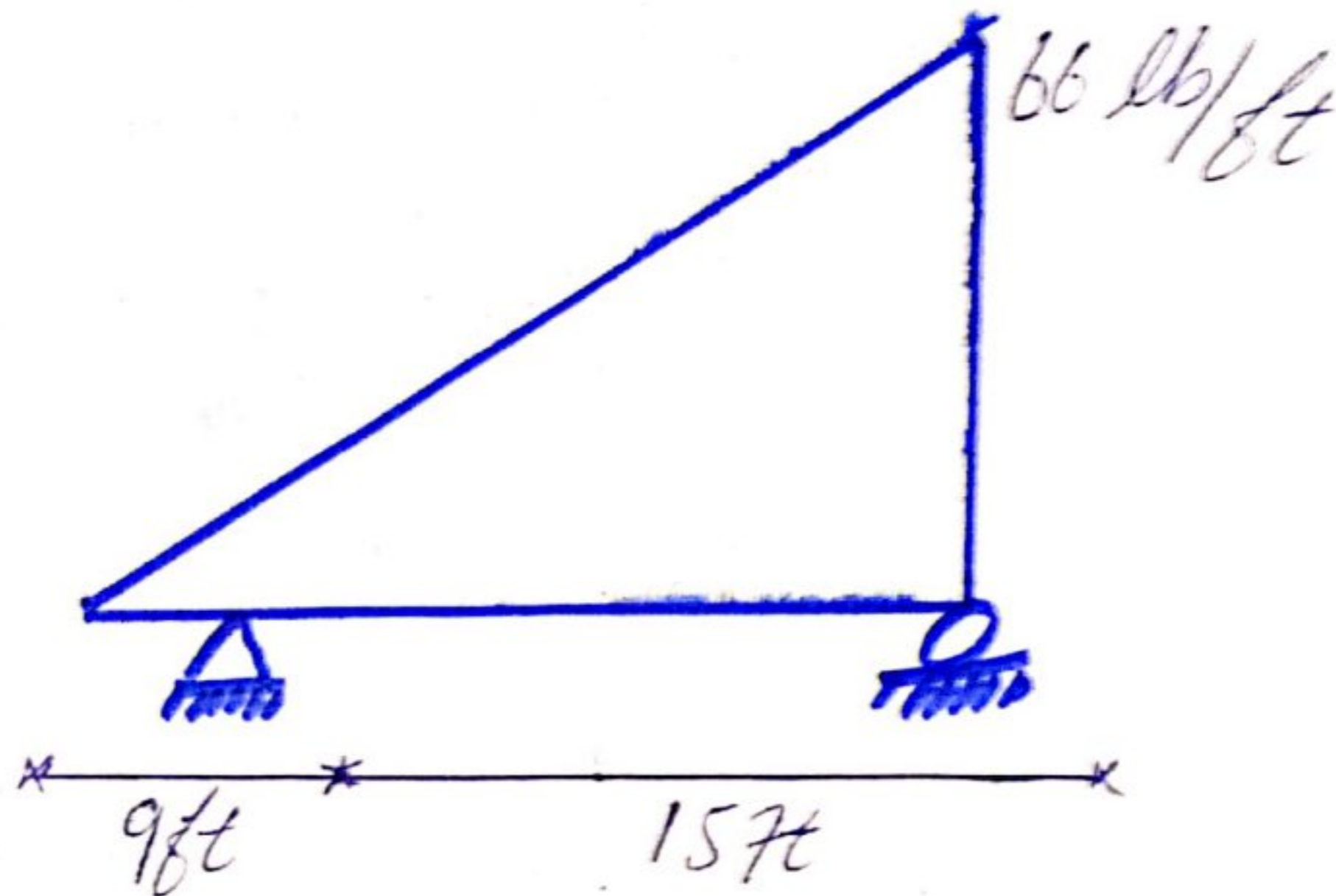
$$T_A = \frac{F_H}{\cos \theta_A} = \frac{4834}{\cos(84.23)} = 48082.5 \text{ lb}$$

$$= \boxed{48.0825} \text{ kips}$$



Ans: (i)

Shear and Bending Moment



Sol:

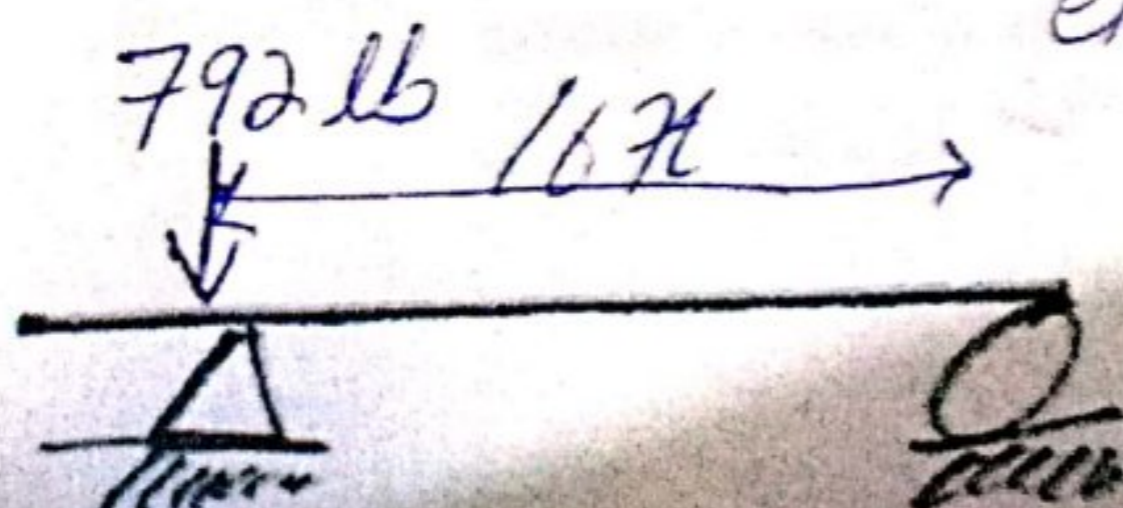
Converting UVL To point load

We have;

$$X = \frac{2B}{3} = \frac{2(24)}{3} = \frac{48}{3}$$

$$X = 16 \text{ ft}$$

From the large end of the load Triangle.



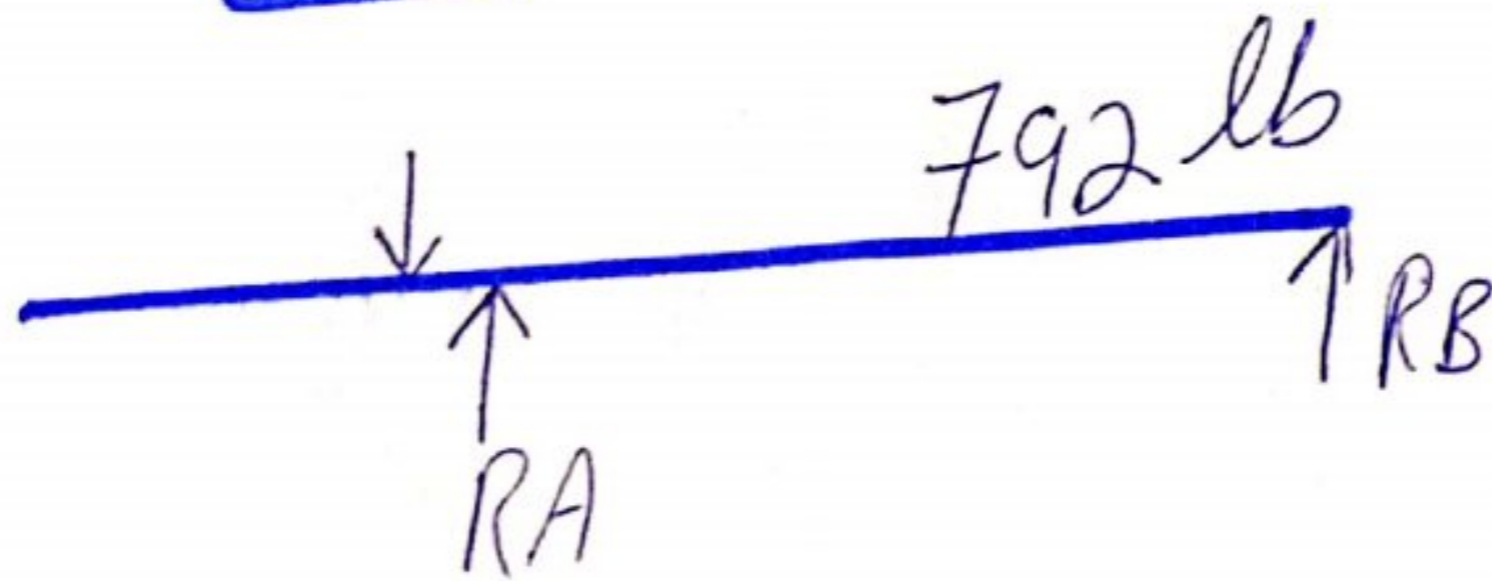
(2)
Magnitude of concentrated point load:-

$$\frac{1}{2}bh$$

$$= \frac{1}{2}(24)(66)$$

$$= 792$$

Support Reaction:



$$\sum M_B = 0$$

$$-15RA + 792(16) = 0$$

$$15RA = 12657$$

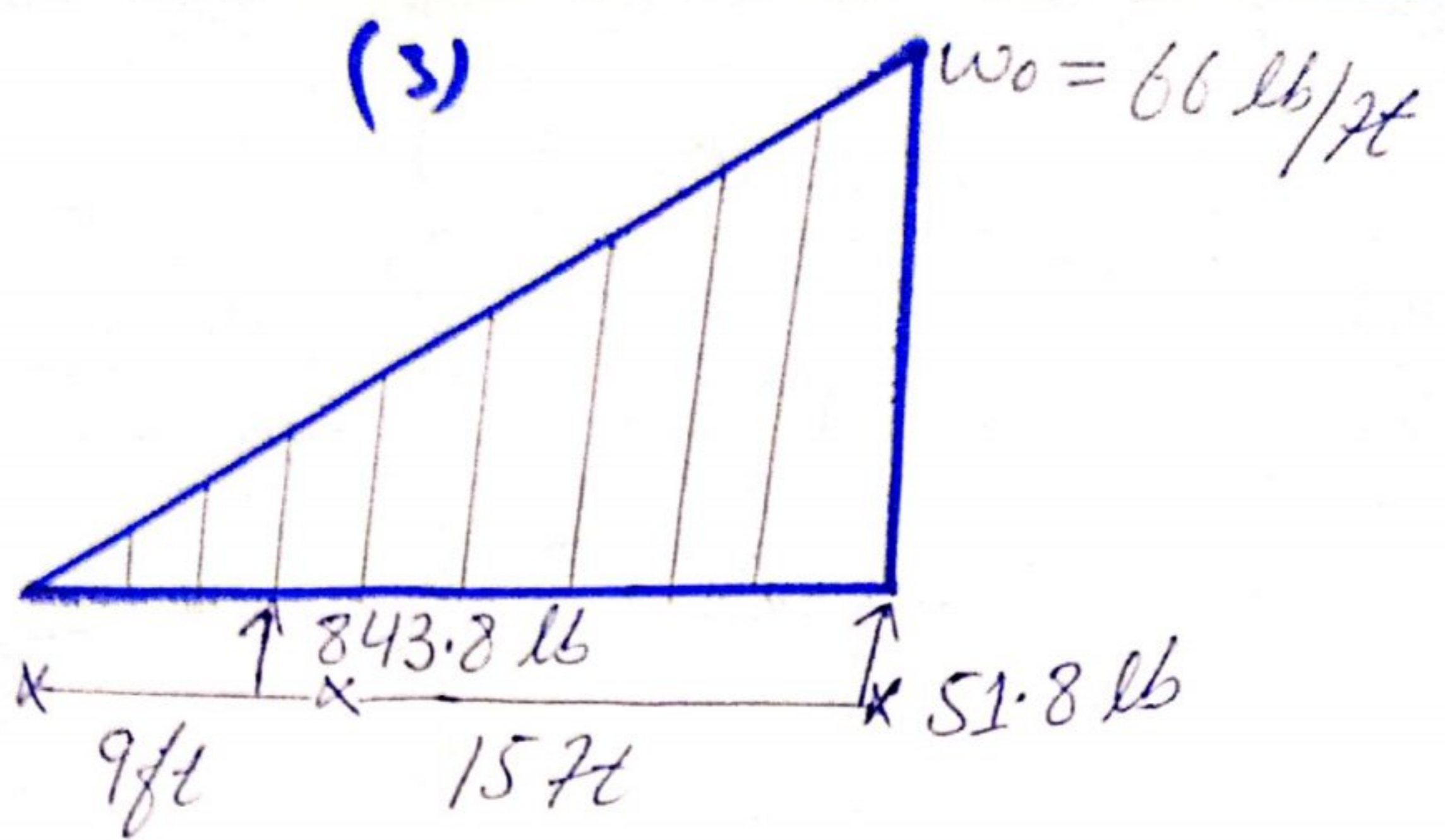
$$RA = 843.8 \text{ lb}$$

$$\sum F_Y = 0$$

$$-792 + RA + RB = 0$$

$$-792 + 843.8 + RB = 0$$

$$RB = 51.8 \text{ lb}$$



Now:

$$\frac{w_0 L}{4} - \frac{1}{2} \left(\frac{w_0 x^2}{L} \right) (x) = 0$$

$$432 - \frac{1}{2} \left(\frac{66 x^2}{L} \right) = 0$$

$$1.5 x^2 - 432 = 0$$

$$x^2 = 432 / 1.5$$

$$\sqrt{x^2} = \sqrt{288}$$

$$x = 16.970$$

Now:

$$\sum M = 0$$

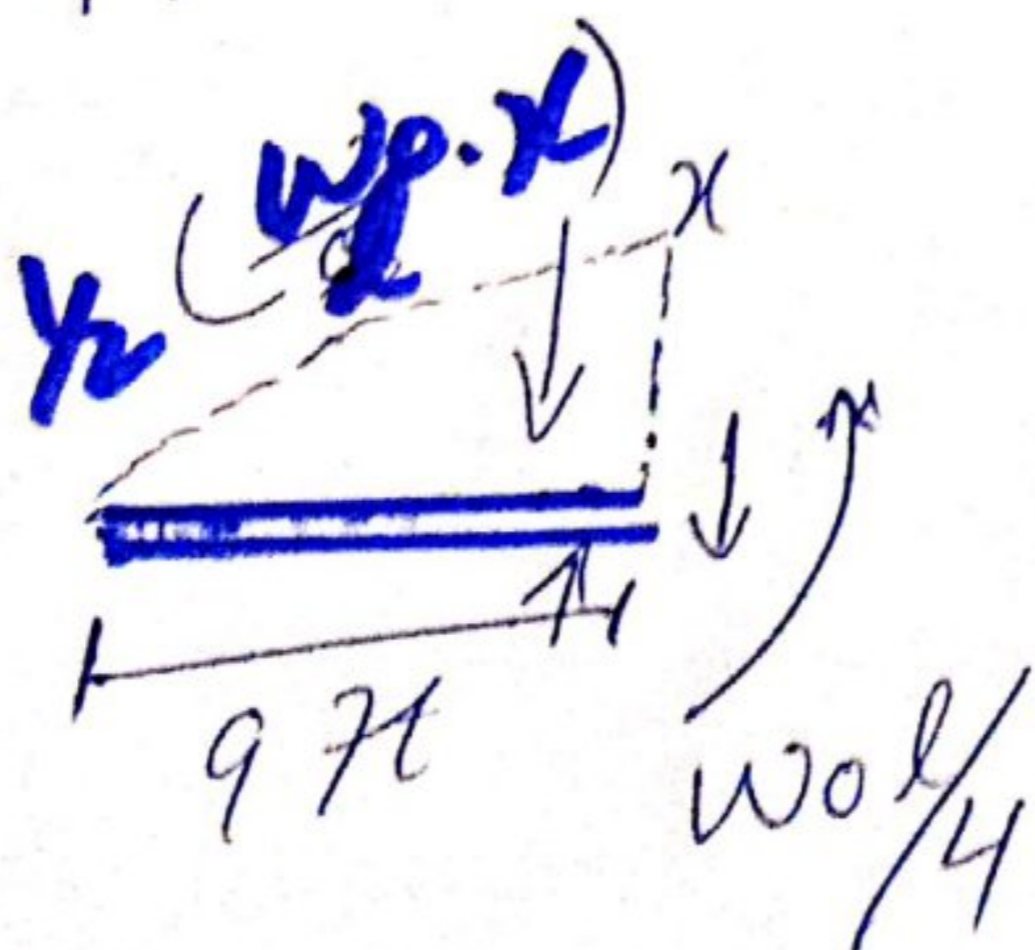
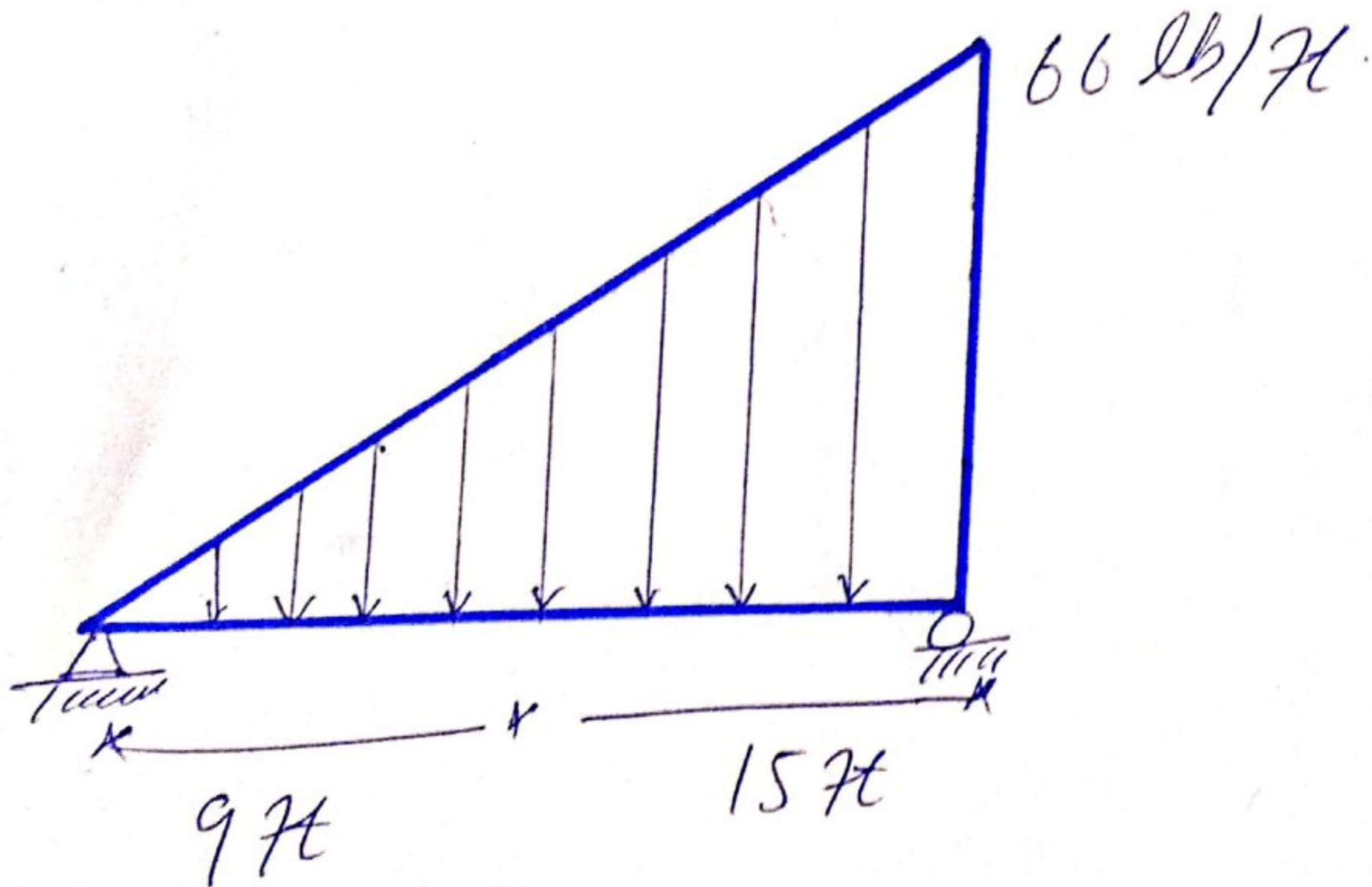
$$M + \frac{1}{2} \left(\frac{w_0 x}{L} \right) \left(\frac{x}{3} \right) - \left(\frac{w_0 L}{4} \right) \left(x - \frac{1}{3} \right) = 0$$

(4)

$$M = -\frac{1}{2} (66)(16.97) \times \left(\frac{16.97}{3}\right) + \frac{66(24)}{4} \left(16.97 - \frac{1}{3}\right) = 0$$

$$M = -7698.30 + 4025.48$$

$$M = -3672.82 \text{ lb/ft.}$$



$$\frac{1}{2} \left(\frac{66(16.97)}{24} \right) (16.97) = 431.97 \text{ lb}$$

$$\frac{w_0}{l} x = \frac{66}{24} (16.97) = 50.91 \text{ lb/ft}$$

Now Shear force and Bending Moment Diagram:-

