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SECTION

A

PAPER

MOS II

DATE

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ANSWER TO QUESTION 1

PART A.

Given that

Height of section $h = 50 \text{ mm}$

Thickness $\rightarrow b = 20 + b$

$$b = 26 \text{ mm}$$

$$T_f = 2 \text{ mm}$$



REQUIRED

Shear center?

As we know that

for unsymmetrical member, the Shear Center is some distance away from the Geometrical Center.

\Rightarrow This distance is called eccentricity which is given as

$$e = \frac{T_f h^2 b^2}{4I}$$

Here I = moment of inertia e
is given as

$$I = 2 \left(\frac{bh^3}{12} + Ay^2 \right) + \left(\frac{bh^3}{12} + Ay^2 \right)$$

$$\Rightarrow I = 2 \left(\frac{20(2)^3}{12} + (20 \times 2)(25)^2 \right) + \left(\frac{2(50)^3}{12} + 0 \right)$$

$$I = 50034.66 + 20833$$

$$I = 70867.99 \text{ mm}^4$$

Now equation (1) \Rightarrow

$$e = \frac{I_p h^2 b^2}{4I}$$

$$e = \frac{2 \times (50)^2 \times (25)^2}{4(70867.99)}$$

$$e = 11.0234 \text{ mm}$$

So, Shear center is 11.0234 mm away from geometrical center.

ANSWER TO QUESTION NO 1 :-

(P3)

PART B =

GIVEN DATA

Height = 26 ft

Tangential stress = 6000 psi

Specific weight of water = 62.4 lb/ft³

REQUIRED DATA:

Thickness of wall of water tank = $t = ?$

SOLUTION:

AS we know

pressure is equal to

$$P = \gamma h$$

$$6t = \frac{PD}{2t} = \frac{\gamma h \times D}{2t}$$

$$t = \frac{\gamma h D}{26t}$$

Putting the values, we get.

$$t = \frac{62.4 \times 26 \times D}{(12)^3} \\ \frac{\quad}{2(6000)}$$

$$t = \frac{62.4 (26 \times 12 \times 22 \times 12)}{(12)^3} \\ \frac{\quad}{2(6000)}$$

$$t = 0.24 \text{ inch}$$

Ans.

ANSWER TO QUESTION NO 2:

(P5)

PART A

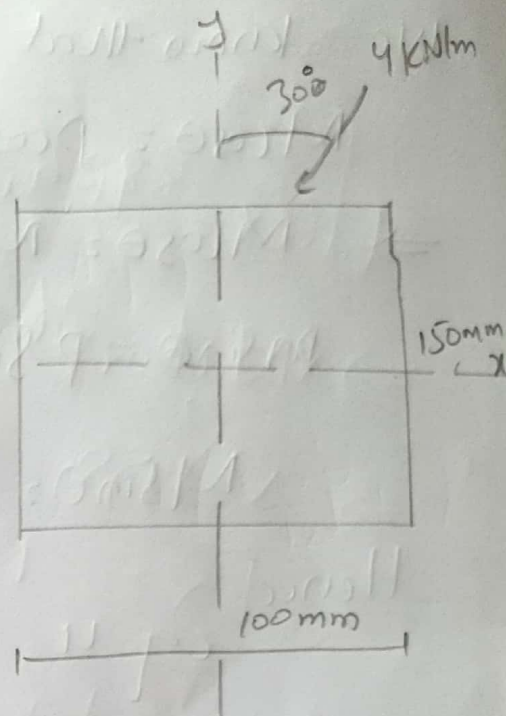
GIVEN DATA

$$b = 100 \text{ mm}$$

$$h = 150 \text{ mm}$$

$$\text{load} = P = 4 \text{ kN/m}$$

$$\text{Length of Beam} = 3 \text{ m}$$



REQUIRED

Bending Stress = ?

$$N.A = ?$$

Now,

$$\sigma = \frac{M_z y}{I_z} + \frac{M_y z}{I_y}$$

$$\therefore M_z = M \cos \theta$$

$$M_y = M \sin \theta$$

$$\sigma = \frac{M \cos \theta}{I_z} + \frac{M \sin \theta}{I_y} \rightarrow \textcircled{a}$$

Now, moment about x-axis

$$M_z = -11.8563 \times 3$$

$$M_z = -35.57 \text{ N.m}$$

$$M_y = 6.851 \times 3$$

$$\Rightarrow M_y = 5.55 \text{ N.m}$$

We know that

$$M \cos \theta = P \cos \theta = M_z$$

$$\Rightarrow M \cos \theta = M_z$$

$$M \sin \theta = P \sin \theta = M_y$$

$$M \sin \theta = M_y$$

Hence

eq 4

$$\Rightarrow \int_z \frac{M \cos \theta}{I_z} + \frac{M \sin \theta}{I_y}$$

Now moment of Inertia

$$I_z = \frac{bh^3}{12} = \frac{0.1(0.15)^3}{12}$$

$$I_z = 2.8125 \times 10^{-5} \text{ in}^4$$

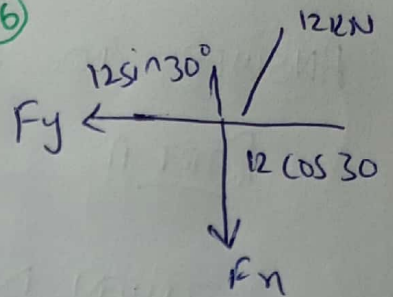
$$I_y = \frac{hb^3}{12} = \frac{0.15(0.1)^3}{12}$$

$$I_y = 1.25 \times 10^{-5}$$

eq 2

$$\int_z \frac{1.851}{2.812 \times 10^{-5}} + \frac{-11.8563}{11.85 \times 10^{-6}}$$

P6



$$I_z = 882678 \text{ Nm}^2$$

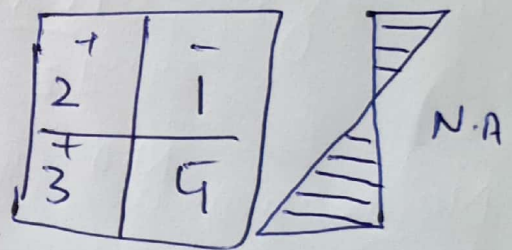
P7

⇒ Natural axis (N.A):
Sign convention are

2	1
4	3

⇒ if we take compression as negative & tension as positive, then beam is simply supported.

In this case: Natural axis passes 2 & 4



Quadrant.

→ In unsymmetrical loading cases the neutral axis lies on angle of ' α ' which is given by

$$\tan \alpha = \frac{I_z \tan \theta}{I_y}$$

$$\tan \alpha = \frac{2.8125 \times 10^{-5}}{1.25 \times 10^{-5}} (\tan 30^\circ)$$

→

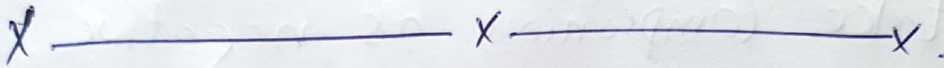
(F7) $\tan \alpha = 14.4124$

(P8) $-14.4124 = \tan \alpha$

$$\alpha = \tan^{-1}(-14.4124)$$

$$\Rightarrow \alpha \approx 1.5^\circ$$

$$\boxed{\alpha = 1^\circ 30' 5''}$$



ANSWER TO QUESTION NO 2:

(9)

PART B

GIVEN DATA:

$$L = 16 \text{ ft}$$

$$I_x = 112.6 \text{ in}^4$$

$$I_y = 18.7 \text{ in}^4$$

$$\sigma_c = 12000 \text{ psi}$$

$$\sigma_t = 5000 \text{ psi}$$

SOLUTION:

By seeing to the figure, we can judge that maximum compression would occur on A & maximum tension at C.

At B there will be taken in

well as compression, which will

reduce the effects of each other

So, we will calculate stresses at A & C.

So,

$$\sigma_A = \frac{Mx}{I_x} + \frac{My}{I_y} \quad \text{Compression}$$

$$\sigma_C = \frac{Mx}{I_x} - \frac{My}{I_y} \quad \text{Tension}$$

Now $M_x \& M_y$

So

$$M_x = \frac{P \cos 60^\circ \times 16 \times 12}{4}$$

$$M_x = 48P \cos 60^\circ$$

$$M_y = \frac{48 \cdot P \sin 60^\circ (16 \times 12)}{4}$$

$$\boxed{M_y = 48P \sin 60^\circ}$$

Now

$$\delta_A = \frac{M_x y}{I_x} + \frac{M_y x}{I_y}$$

$$12000 = \frac{48P \cos 60^\circ \times 3.07}{112.6} + \frac{48P \sin 60^\circ \times 3}{18.7}$$

By Solving the equation.

$$\boxed{P = 1638.6 \text{ lb}}$$

$$\text{Now } \delta_C = \frac{M_x y}{I_x} + \frac{M_y x}{I_y}$$

$$8000 = \frac{48P \cos 60^\circ (5.93)}{112.6} + \frac{48P \sin 60^\circ \times 0.5}{18.7}$$

Solving the equation

$$P = 2104.9 \text{ lb}$$

So the maximum load of P applied should be 1638.6 lb.

$$P = 1638.6 \text{ lb}$$

$$\frac{P}{EI} + \frac{P}{EI} = \delta$$

$$\frac{18921.00 \times 3}{18.3} + \frac{18921.00 \times 3}{18.3} = 0.006$$

$$P = 1638.6 \text{ lb}$$

$$\frac{P}{EI} + \frac{P}{EI} = \delta$$

ANSWER TO QUESTION NO 3:

(P12)

GIVEN DATA:

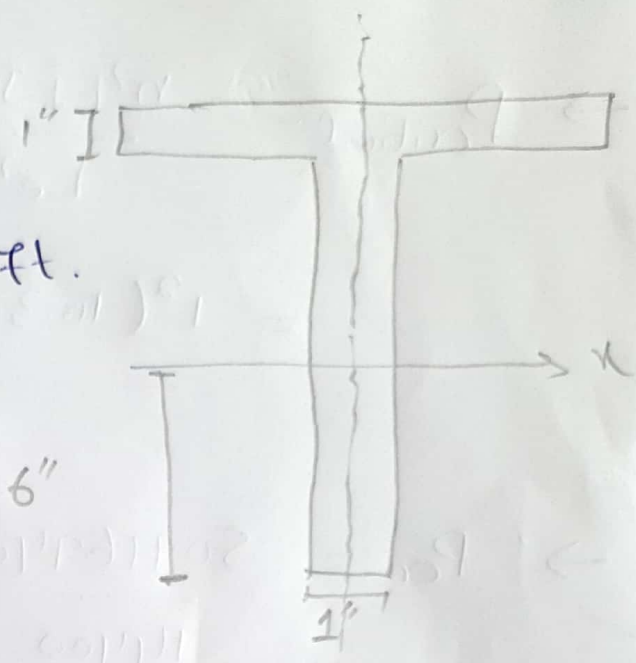
Length of column $L = 10 \text{ ft.}$

$$E = 10.3 \times 10^6$$

$$\text{breadth } b = 0.75$$

$$\text{height} = h = 2$$

Factor of Safety = 2



REQUIRED

→ Safe load = ?

When

→ Both end hinged

→ Both end fixed.

So,

⇒ For hinged column

effective length $L_e = L$

$$I = I_x = \frac{bh^3}{12}$$

$$= \frac{0.75(2)^3}{12}$$



$$I_x = 0.5 \text{ in}^4$$

(P13)

$$\rightarrow P_{\text{critical}} = \frac{n^2 E I \pi^2}{L_e^2}$$

$$= \frac{1^2 (10.3 \times 10^6) (.5) \pi^2}{(10 \times 12)^2}$$

$$\rightarrow P_{\text{cr}} = \frac{50776940}{14400}$$

$$= 3526.176 \text{ lb}$$

$$\text{Safe load} = P_{\text{safe}} = \frac{P_{\text{cr}}}{\text{factor of Safety}}$$

$$= \frac{3526176}{2}$$

$$\Rightarrow P_{\text{safe}} = 1763.088 \text{ lb}$$

→ When Both ends fixed in this

$$\text{Case } L_e = \frac{L}{2}$$

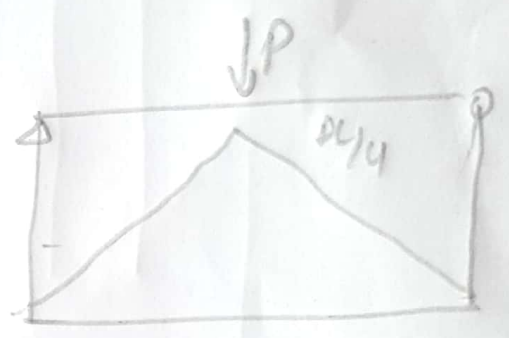
$$L_e = 5 \text{ ft}$$

$$I = I_y = 2 \times \frac{(0.75)^3}{12} = 0.07 \text{ in}^4$$

$$\Rightarrow P_{cr} = \frac{n^2 E I \pi^2}{L e^2}$$
$$= \frac{(1)^2 (10.3 \times 10^6) (0.07) (\pi)^2}{(5 \times 12)^2}$$

$$P_{cr} = \frac{7108771.6}{60^2}$$

$$P_{cr} = \frac{1974.658}{2}$$



So, Safe load

$$P_{\text{safe load}} = \frac{1974.658}{2}$$

$$P_{\text{safe}} = 987.3293 \text{ lb}$$

