



DEPARTMENT OF CIVIL ENGINEERING

SUBJECT: ADVANCE FLUID MECHANICS

NAME: ABDUL BASIT

ID: 7776

Q.NO (01) A

ANSWER

Question NO # 01 (A part) 1

⇒ Drag :- A body which is wholly immersed in a homogenous fluid may be subjected to two kind of forces arising from relative motion b/w body and fluid these forces are termed as drag and lift. if the forces parallel to the motion then it is termed as drag force.

There are two components.

* Pressure Drag (F_p) :- it is equal to integration of components in direction of motion of all pressure forces exerted on surface of body.

$$F_p = C_p \int \frac{V^2}{2} A$$

where C_p depends on shape.

* Friction Drag (F_f) :- it is equal to integration of components of shear stress along surface of body in direction of motion.

$$F_f = C_f \int \frac{V^2}{2} BL$$

The diagram shows a rectangular block of length L and width B in a fluid flow moving with velocity V from left to right. The front face of the block is perpendicular to the flow. A horizontal arrow labeled V indicates the flow direction. The top surface of the block is labeled B and the length is labeled L . A horizontal arrow labeled "shear stress on 3rd area" points to the top surface of the block.

⇒ Friction Drag of Boundary Layer. ②

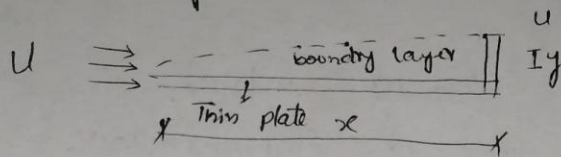
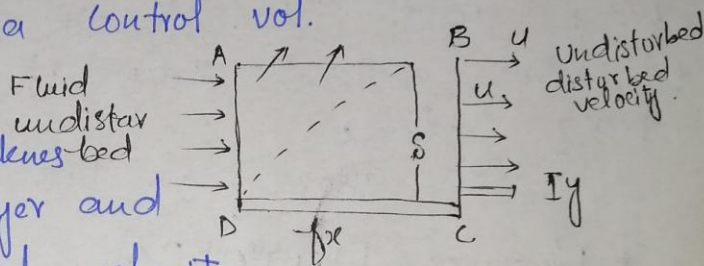


Fig shows growth of boundary layer along one side of smooth plate inside the fluid.

Now consider a control vol.

where δ is thickness of boundary layer and U is undisturbed velocity.



Thus $-F_x = \text{drag} = (\text{rate in momentum in } x\text{-dir.}$

leaving through BC rate of momentum through AB) - rate of momentum entering through DA)

$$\Delta p = P_{out} - P_{in}.$$

Thus according to momentum.

$$\Sigma F = \frac{d}{dt}(P) = \frac{d(mv)}{dt}$$

where $\frac{dm}{dt} = \int Q$ Thus

$$F = \int Qv \Rightarrow F = \int A \cdot v \cdot v \Rightarrow F = \int A v^2$$

$$DA \rightarrow \int U (UB\delta)$$

$$B_c \rightarrow \int_B \int_0^{\delta} u^2 \cdot dy$$

$$A_B \rightarrow \int_V (u_B \delta - B \int_0^{\delta} u \cdot dy)$$

Putting value.

$$F_x = \int_B \int_0^{\delta} u (u - u) dy$$

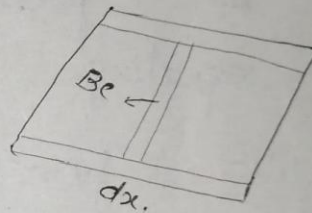
$$F_x = \int_B u^2 \int \alpha$$

$\therefore \alpha$ is Function of boundary layer.

Now to find local wall shear stress.

$$\tau_0 = \frac{df_x}{B \cdot dx - \text{area}}$$

$$F_x = \int_B u^2 \int \alpha$$



$$\tau_0 = \int u^2 \propto \frac{ds}{dx} \text{ is equation of shear stress.}$$

\Rightarrow Laminar boundary layer:-

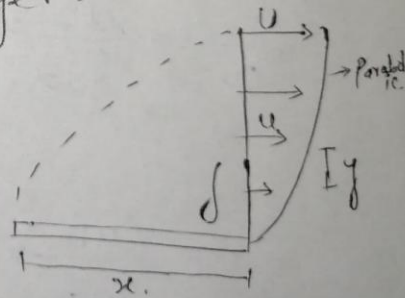
$$\frac{u}{U} = F\left(\frac{y}{\delta}\right)$$

Assume.

$$\eta = y/\delta \text{ or } y = \eta \delta.$$

Thus

$$\frac{u}{U} = f(\eta) \text{ or } u = U f(\eta)$$



⇒ In case of laminar flow.

$$\tau_0 = \mu \left(\frac{du}{dy} \right) = \frac{\mu}{\delta} \left(\frac{dy}{dx} \right)$$
$$= \frac{\mu u}{\delta} \left[\frac{df(u)}{dx} \right]$$

Solving the equation.

$$\tau_0 = \frac{\mu u B}{\delta} \quad \text{--- ①}$$

As general equation is $\tau_0 = \int u^2 \alpha \frac{ds}{dx}$.
equating both equation.

$$\frac{\mu u B}{\delta} = \int u^2 \alpha \frac{ds}{dx} \quad \text{or} \quad \int ds = \frac{\mu B}{\delta u \alpha} dx.$$

integrating the equation.

$$\frac{s^2}{2} = \frac{\mu B}{\delta u \alpha} x + C$$

Now at $x=0$, $s=0$ thus

$$\frac{s^2}{2} = \frac{\mu B}{\delta u \alpha} x \quad \text{or} \quad s = \sqrt{\frac{2B}{\alpha}} \cdot \sqrt{\frac{\mu x}{\delta u}}$$

King and King by "x"

$$\delta = \sqrt{\frac{2\beta}{\alpha}} \cdot \sqrt{\frac{\mu x}{\rho U}} \cdot \sqrt{\frac{x}{\sqrt{x} \cdot \sqrt{x}}}$$

$$\delta = \frac{4.91}{\sqrt{R_x}} \cdot x \quad \text{or} \quad \frac{\delta}{x} = \frac{4.91}{\sqrt{R_x}}$$

$$\begin{aligned} \alpha &= 0.135 \\ \beta &= 1.63 \\ R_x &= \frac{\rho U x}{\mu} \end{aligned}$$

Now

$$\tau_0 = \frac{\mu U \beta}{\delta}$$

thus putting the values.

$$\tau_0 = 0.332 \frac{\mu U}{x} \sqrt{R_x}$$

where R_x is the local Reynold number

Now

$$F_D = B \int_0^L \frac{\tau_0 dx}{\text{stress}}$$

Putting the values.

$$F_D = 0.664 B \sqrt{\rho U L^3}$$

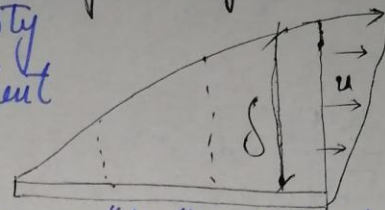
As general equation is

$$F_D = C_D \rho \frac{V^2}{2} B L \quad \rightarrow \quad \text{equations both equation.}$$

$$C_D = 1.328 \frac{\sqrt{U}}{\rho L V} = \frac{1.328}{\sqrt{R}}$$

⇒ Turbulent boundary layer.

Fig show that velocity distribution in turbulent boundary layer.



Resistance is less
So curve become slight.

laminar "transition" turbulent.

Shows a much steeper gradient near wall and flatter through out remaining layer the shear stress is greater in turbulent than in laminar layer.

As we have

$$\tau_0 = f \int \frac{v^2}{8} \quad \because \text{where } v \text{ denotes average velocity of pipe.}$$

Now we have obtained an approximate relation b/w v and U by using pipe factor equation of

$$\frac{v}{U_{max}} = \frac{1}{1 + 1.33 \sqrt{f}}$$

0.020 From what which is middle critical value

$$\text{So } \boxed{U = 1.235 v}$$

NO we have

$$\tau_0 = f \int \frac{v^2}{8}$$

As we have

$$F = \frac{0.316}{Re^{0.25}}$$

Thus

$$\tau_0 = \frac{0.316}{\left(\frac{Du}{\nu}\right)^{1/4}} \frac{\int v^2}{8}$$

where

$$v = \frac{u}{1.235} \quad \text{Thus}$$

$$\tau_0 = \frac{0.316}{\left(\frac{D}{\nu} \left(\frac{u}{1.235}\right)\right)^{1/4}} \cdot \int \frac{1}{8} \left(\frac{v}{1.235}\right)^2$$

$\approx D = 25$

Thus

$$\tau_0 = \frac{0.023 \int v^2}{\left(\frac{8v}{u}\right)^{1/4}}$$

\Rightarrow As we have

$$\tau_0 = \int v^2 \propto \frac{ds}{dx}$$

Equating both and integrating for boundary condition of $x=0, \delta=0$

Thus

$$\delta = \left(\frac{0.0287}{\alpha}\right)^{4/5} \left(\frac{\nu}{u x}\right)^{4/5} x$$

For $\alpha = 0.0972$

$$\Rightarrow \boxed{\frac{f}{Re} = \frac{0.377}{(Re)^{1/5}}$$

Putting values in equation.

$$\tau_0 = 0.0587 \frac{\rho v^2}{2} \left(\frac{v}{Ux} \right)^{1/5}$$

$$\text{Now } F_f = B \int_0^L \tau_0 dx$$

$$F_f = 0.0735 \int_0^L \frac{\rho v^2}{2} \left(\frac{v}{Ux} \right) B dx$$

As

$$F_f = C_f \frac{\rho v^2}{2} BL$$

Equating Both

$\therefore R$ less than 10^7 for
 $300,000 < R < 10^7$

$$\boxed{C_f = \frac{0.0735}{R^{1/5}}$$

For $R > 10^7$

$$\boxed{C_f = \frac{0.455}{(\log R)^{2.58}}$$

Ans

QNO 01 (B part).

As specific energy.

$$E = y + \frac{v^2}{2g}$$

The Flow Q Per unit width b can be expressed as

$$q_v = Q/b$$

Now average velocity will be

$$v = Q/A = \frac{q_v b}{by} = q_v/y$$

Thus

$$E = y + \frac{v^2}{2g} \Rightarrow y + \frac{1}{2g} \left(\frac{q_v^2}{y^2} \right)$$

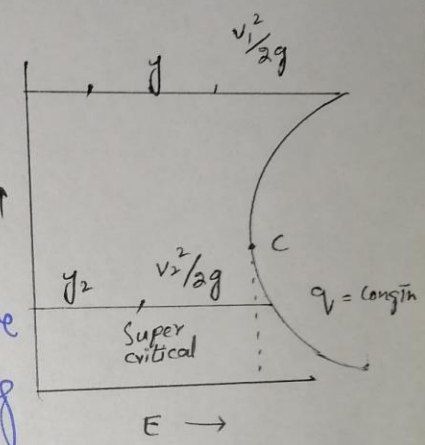
$$(E - y) = \frac{1}{2g} \left(\frac{q_v^2}{y^2} \right) \text{ or}$$

$$(E - y) y^2 = \frac{q_v^2}{2g}$$

②

Thus E vs y will be parabolic
For particular q , there will be
two kind of possible values
of y , for a given E .

The Equation is cubic
with three roots, with
Third root being negative
Point c , represents dividing
Point b/w two regime
of flow thus for given q , and
value of E is minimum and
Flow at that point is critical
Flow. Depth of flow at that
Point is critical depth y_c and
velocity at that point is
critical velocity V_c



Thus

$$E = y + \frac{1}{2g} \left(\frac{q^2}{y^2} \right)$$

For minimum specific energy $\frac{dE}{dy} = 0$

Thus

$$\frac{dE}{dy} = 1 - \frac{2}{2g} \left(\frac{q^2}{y^3} \right) = 0$$

$$\Rightarrow \frac{q^2}{gy^3} = 1 \Rightarrow q^2 = gy^3$$

$$\Rightarrow \frac{q^2}{g} = y^3 \Rightarrow \left(\frac{q^2}{g} \right)^{1/3} = y_{cr}$$

Now

$$\Rightarrow q^2 = gy^3$$

and

$$\Rightarrow q = Vy \Rightarrow V^2 y^2 = gy^3$$

$$\text{or } V^2 = gy_{cr}$$

or

$$V_{cr} = \sqrt{gy_{cr}}$$

Question NO # 02

⇒ Given Data
water flow at rate of $Q = 3.5 \text{ m}^3/\text{s}$
Bed slope, $S_0 = 0.0008$
 $n = 0.0219$
width of bed is $B = 7.776$

⇒ Required

Depth of Rectangular channel = ?

critical depth $y_c = ?$

critical velocity $= v_c = ?$

Flow is - critical or sub-critical = ?

Solution

$$Q = \left(\frac{1}{n} R_n^{2/3} S_0^{1/2} \right) A \quad \text{--- (1)}$$

$$\text{Area} = 7.776 \times d$$

$$\text{Parameter} = d + 7.776 + d$$

$$\text{hydraulic Radius} = R_n = \frac{\text{Area}}{\text{Parameter}}$$

As we know that

$$Q = \left(\frac{1}{n} R_n^{2/3} S_0^{1/2} \right) A$$

Putting the values.

$$3.5 = \left(\frac{1}{0.0219} \times \left(\frac{7.776 \times d}{2d + 7.776} \right)^{2/3} \times (0.0008)^{1/2} \right) \times 7.776 \times d$$

$$\text{As } q = Q/b$$

$$q = \frac{3.5}{7.776} = 0.450$$

For critical depth

$$y_{cr} = \left(\frac{q^2}{g} \right)^{1/3}$$

$$y_{cr} = \left(\frac{(0.450)^2}{9.81} \right)^{1/3}$$

$$\boxed{y_{cr} = 0.274 \text{ m}}$$

⇒ Critical velocity, V_{cr}

using equation $V_{cr} = \sqrt{g y_{cr}}$

$$V_{cr} = \sqrt{(9.81)(0.274)}$$

$$V_{cr} = 1.639 \text{ m/s}$$

$$V = Q/A$$

$$V = \frac{3.5}{7.776 \times 0.831}$$

$$V = 0.541 \text{ m/s}$$

$$y = 0.840 \text{ m} , \quad y_{cr} = 0.274 \text{ m}$$

$$V = 0.541 \text{ m/s} , \quad V_{cr} = 1.639 \text{ m/s}$$

As $y > y_{cr}$ and $V < V_{cr}$

So Flow is Sub-critical

Question No 03

Given data: width of smooth plate
= $B = 200 \text{ mm}$

$$B = 0.2 \text{ m}$$

length of plate $L = 800 \text{ mm}$

$$L = 0.8 \text{ m}$$

Specific gravity of oil $\rho = 0.89$

undisturbed velocity $V = 5 \text{ m/s}$

kinematic viscosity $\nu = 0.93 \times 10^{-4} \text{ m}^2/\text{s}$

⇒ Required data:

Friction drag $F_f = ?$

⇒ Solution

As we know that.

The Friction drag is given by the equation.

$$F_f = C_f \rho \frac{V^2}{2} BL \quad \text{--- (1)}$$

where c_f depends on viscosity. (2)

Reynold's number is given by

$$R = \frac{\rho v L}{\mu} = \frac{v L}{\nu} \quad \therefore \nu = \frac{\mu}{\rho}$$

Putting values we get.

$$R = \frac{5 \text{ m/s} \times 0.8 \text{ m}}{0.93 \times 10^{-4} \text{ m}^2/\text{s}}$$

$$R = 43,010.75$$

which is less than 500,000.

So,

$$c_f = \frac{1.328}{\sqrt{R}} = \frac{1.328}{\sqrt{43011}} = 0.00640.$$

Now

$$\text{As Specific gravity} = \frac{\rho_{\text{soil}}}{\rho_{\text{water}}}$$

$$\rho_{\text{soil}} = \text{Specific gravity} \times \rho_{\text{water}}$$

$$\rho_{\text{soil}} = 0.89 \times 1000$$

$$\boxed{\rho_{\text{soil}} = 890}$$

Putting values in equation ① we get

$$F_f = 0.00640 \times 0.89 \times 1000 \times \frac{(5)^2}{2} \times 0.2 \times 0.8$$

$$F_f = 11.392 \text{ N}$$

So,

The friction drag on one side
of smooth plate is 11.392 N.

THE END