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Sec: B

Q1

Hydraulic structure.

Ans

Culvert

Causeway

Culvert is of a tunnel shape carrying a stream of water under a road or railway.

It works as a bridge to pass an it.

It is normally uses for natural flow of water for controlling it.

A cause way is of course a raised road. it is built on an embankment.

It is supported mostly by earth or stone.

And it is not a bridge because it supports a roadway between piers.



Q.1.
Ans. b

Cross Drainage Work:

is a structure carrying the discharge from a natural stream across a canal intercepting the stream.

Necessarily:

It is required to dispose of the drainage water so that the canal supply water remains uninterupted.

Types:

Some types of cross drainage are following.

- (i) **Adequater:**
It carries an irrigation canal over a drain.
- (ii) **Super Passage:**
It carries a drain over an irrigation canal.
- (iii) **Level Crossing:**
This structure makes it possible to dispose off drain water safely at same level as that of a canal.

(iv) Inlet and Outlet:

When possible drain water is taken in the canal to be discharged afterwards into a drain at suitable location.

Q 2

Ans
2

Wair:

Wairs are commonly used to control the flow rates of rivers during periods of high discharge.

Sluice gates are used to increase or decrease the volume of water going out.

Bassage:

It is used to convert tidal energy into electricity by forcing water through turbines by activating a generator.

Reynolds Number:

The Product of density times length divided by viscosity coefficient.

This is proportional to the ratio of inertial forces and viscous forces in a fluid flow.

Laminar:

The flow in a pipe is laminar if the Reynolds Number is less than 2100.

Turbulent:

If the Reynolds Number is greater than 4000 then it is turbulent.

Neither Laminar nor turbulent flow:

When the Reynolds number is between 2000 and 2800, the flow is neither laminar nor turbulent.

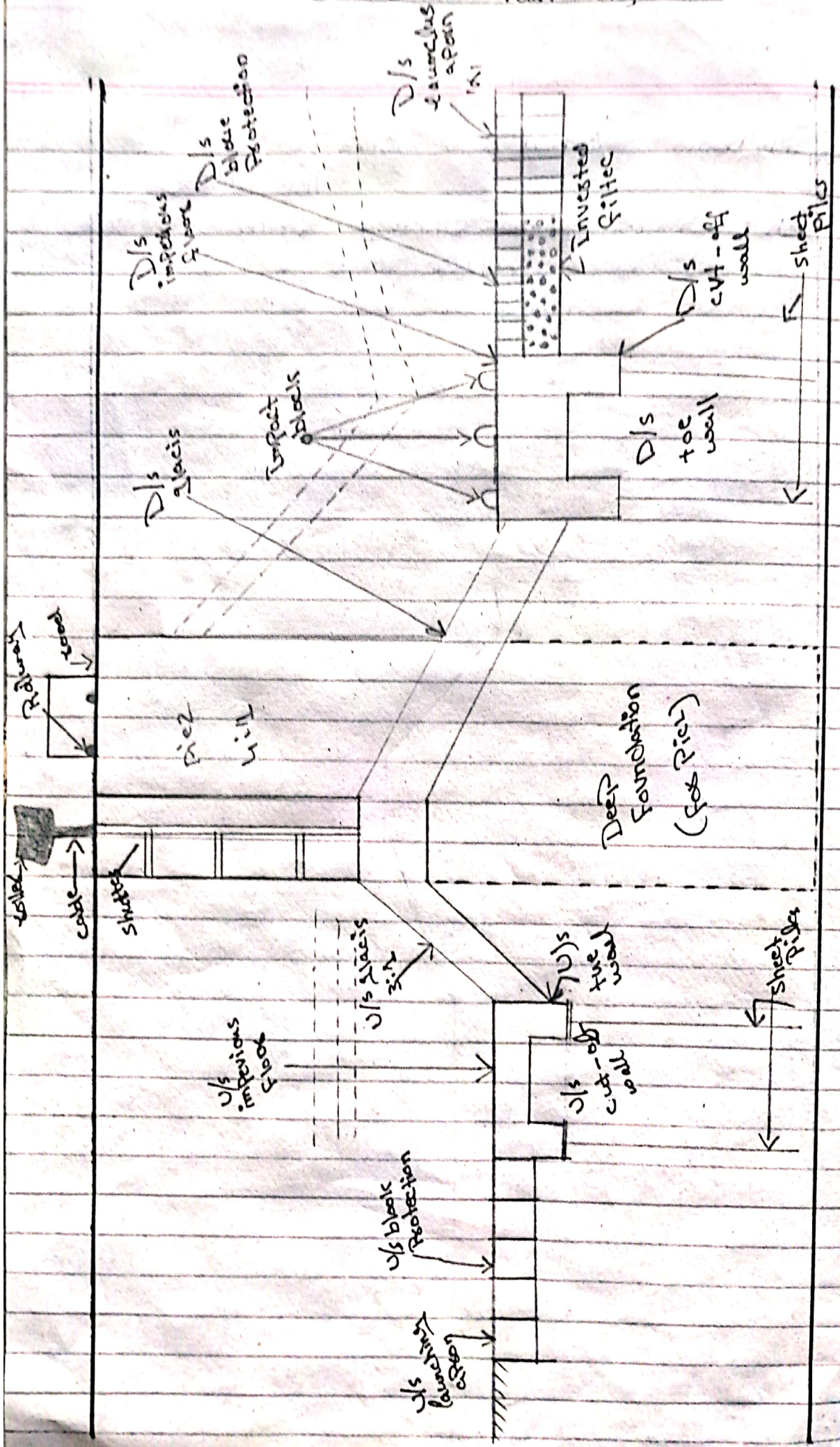
Lower Critical velocity:

The velocity at which flow changes from laminar to transition is called lower critical velocity.

Higher Critical velocity:

The velocity at which flow change from transition to turbulent is called Higher Critical velocity.

Question No 3 Part (i)



Question No 3 Part (ii)

Several formulae based on experimental results have been proposed to predict the "maximum" or "equilibrium" scour depth (y_s) below general bed level around bridge piers. In general than assume the relationship.

$$y_s/b' = \phi(y_0/b', F_s, d/b')$$

where b' is the pier width, y_0 is the upstream flow depth, d is the sediment size and F_s is the flow Froude number.

Causseis (1969) experimental results underestimate the scour depths. Compared to many Indian experiments (Ingilis, 1949) which suggest the formula (approach flow is normal to the bridge pier)

$$y_0/b' = 4.2 (y_0/b')^{0.78} F_s^{0.52}$$

The Indian field data also suggest the scour depth should be taken as twice the regime scour depth.

In case of live beds the formula

$$y_0/y_a = (B/b')^{5/7} - 1$$

Predict the maximum equilibrium scour depth.

In relatively deep flow a first order estimate of scour may be obtained by

$$y_s = 2.3 k_s b'$$

where k_s = angularity

co-efficient which is a function of the Pier alignment.

(9)

Question No 9
Answer No 9

Given Data

$$L.L = 1.5 \text{ kip/ft}^2$$

$$D.L = 300 \text{ lb/ft}^2$$

$$\theta = 30^\circ$$

$$\text{Unit weight of soil} = 100 \text{ lb/ft}^3$$

$$\text{Dimension} = 15' \times 15'$$

$$F_y = 60 \text{ ksi steel}$$

$$\text{concrete} = 1:2:4 = \text{M15}$$

$$D = 0.99 \text{ m thickness.}$$

Sol: (i) Load

Total load on top = self weight +
l.l + D.l

$$\text{Self weight} = 3 \times 5 = 4 \text{ kN/m}^2$$

$$45 \text{ kN/m}^2 = 0.939 \text{ kip/ft}^2$$

$$w = 1.5 + 0.939 + 0.3$$

$$w = 2.739 \text{ kip/ft}$$

(ii) Co. efficient of earth pressure.

$$K_a = \frac{1 - \sin \theta}{1 + \sin \theta}$$

$$= \frac{1 - \sin(30)}{1 + \sin(30)}$$

$$= \frac{1 - \sin(30)}{1 + \sin(30)}$$

$$K_a = 0.33$$

3) Lateral Pressure due to (Dead Load + Live load)

$$\text{Total unfical load} \times K_a$$

$$= (C.C + D.C) \times K_a$$

$$= (1.5 + 0.3) \times 0.33$$

$$= 0.594 \text{ kip/ft}^2$$

$$28.4 \text{ kN/m}^2$$

4) Lateral Pressure due to soil

$$= K_a \times \gamma_{\text{soil}} \times h$$

$$= 0.33 \times 0.1 \times 18$$

$$= 0.594 \text{ kip/ft}^2$$

or

$$= 28.4 \text{ kN/m}^2$$

5) Lateral Pressure at top due to P.C + D.C

$$= 0.594 \text{ kip/ft}^2$$

$$= 28.4 \text{ kN/m}^2$$

6) Lateral Pressure at bottom.

Lateral Pressure due to (C.C + D.L) +
Lateral Pressure due to soil

(11)

$$= 0.594 + 0.594$$

$$= 1.188 \text{ kip/ft}^2$$

$$= 58.88 \text{ kN/m}^2$$