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Section A

Semester 6th

Paper Hydraulic Engr.

Q1

Sol:-

The pressure drop (ΔP) is expected to depend upon gate opening h , and overall depth d , the velocity (V), density (ρ) and viscosity μ .

Relevant variables are $\Delta P, h, d, V, \rho, \mu$

Dimensions are given

| | |
|------------|-----------------|
| ΔP | $ML^{-1}T^{-2}$ |
| h | L |
| d | L |
| V | LT^{-1} |
| ρ | ML^{-3} |
| μ | $ML^{-1}T^{-1}$ |

We have

Number of variables $n=6$

Number of independent dimension $m=3$ (M, L and T)

Number of non-dimensional groups $n-m=3$.

Choose $m (=3)$ scaling variables

geometric (d), kinematic/time dependent (V), dynamic/mass dependent.

Form dimensionless group by non-dimensionalising remaining variables $\Delta P, h, \mu$.

$$\pi_1 = \Delta p d^a V^b \rho^c$$

$$M^0 L^0 T^0 = (ML^{-1}T^{-2})(L)^a (LT^{-1})^b (ML^{-3})^c$$

$$= M^{1+c} L^{-1+a+b-3c} T^{-2-b}$$

$$M \Rightarrow 0 = 1+c \quad c = -1$$

$$T \Rightarrow 0 = -2-b \quad b = -2$$

$$L \Rightarrow 0 = -1+a+b-3c \quad a = 1+3c-b = 0$$

$$\pi_1 = \Delta p V^{-2} \rho^{-1} = \frac{\Delta p}{\rho V^2}$$

$$\pi_2 = h/d \quad (\text{by inspection, since } h \text{ is a length})$$

$$\pi_3 = u d^a v^b \rho^c$$

$$\begin{aligned} M^0 L^0 T^0 &= (ML^{-1}T^{-1})(L)^a (LT^{-1})^b (ML^{-3})^c \\ &= M^{1+c} L^{-1+a+b-3c} T^{-1-b} \end{aligned}$$

$$M \Rightarrow 0 = 1+c \Rightarrow c = -1$$

$$T \Rightarrow 0 = -1-b+0 \Rightarrow b = -1$$

$$L \Rightarrow 0 = -1+a+b-3c \Rightarrow a = 1+3c-b = -1$$

$$\pi_3 = u d^{-1} v^{-1} \rho^{-1} = \frac{u}{\rho v d}$$

Recognition of Reynold's number suggest that we replace π_3 by

$$\pi_3' = (\pi_3)^{-1} = \frac{\rho v d}{u}$$

By dimensional analysis.

$$\pi_1 = f(\pi_2, \pi_3')$$

$$\frac{\Delta p}{\rho v^2} = f\left(\frac{h}{d}, \frac{\rho v d}{u}\right)$$

a) Dynamic similarity requires that all non-dimensional group be same in model and prototype.

$$\pi_1 = \left(\frac{\Delta p}{\rho v^2}\right) = \left(\frac{\Delta p}{\rho v^2}\right)$$

$$\pi_2 = (h/d) = (h/d) \quad (\text{automatic if similar shape "geometric similarity"})$$

$$\pi_3' = \left(\frac{\rho v d}{u}\right) = \left(\frac{\rho v d}{u}\right)$$

We have a velocity ratio

$$\frac{V_p}{V_m} = \frac{(u/p)_p \frac{d_m}{d_p}}{(u/p)_m \frac{d_p}{d_m}} = \frac{0.002/800}{1.0 \times 10^{-6}} \times \frac{1}{5} = 0.5$$

Hence

$$V_m = \frac{V_p}{0.5} = \frac{3.0}{0.5} = 6.0 \text{ m s}^{-1}$$

b) Ratio of quantities of flow

$$\frac{Q_p}{Q_m} = \frac{(\text{Velocity} \times \text{area})_p}{(\text{Velocity} \times \text{area})_m}$$

$$= \frac{V_p}{V_m} \left(\frac{d_p}{d_m} \right)^2$$

$$= 0.5 \times 5^2$$

$$= 12.5$$

c) Pressure drop

$$\pi_1 = \left(\frac{\Delta P}{\rho V^2} \right)_p = \left(\frac{\Delta P}{\rho V^2} \right)_m$$

$$\frac{(\Delta P)_p}{(\Delta P)_m} = \frac{\rho_p (V_p)^2}{\rho_m (V_m)^2}$$

$$= \frac{800}{1000} \times 0.5^2$$

$$= 0.2$$

Now

$$\Delta P_p = 0.2 \times \Delta P_m$$

$$= 0.2 \times 60$$

$$= 12.0 \text{ kPa.}$$

Q2

Sol:-

Maximum depth of water in Reservoir = $H_w = 78\text{m}$ Specific Gravity of dam material = $G_1 = 2.6$ Allowable compressive stress for Dam Masonary = 781T/m^2 Height of wave = 4.2m .No uplift pressure = $C_u = 0$

$$\mu = 0.7$$

$$1. H_{\text{limiting}} = \frac{\sigma_{\text{au}}}{\gamma_w (G_1 - C_u + 1)}$$

Put values

$$H_{\text{limiting}} = \frac{781 \times 1000}{1000 (2.6 - 0 + 1)} = 216.9\text{m} > H_w = 78\text{m}$$

So it is low gravity dam.

2. Top width "a"

$$\text{Free board} = 1.5 h_{\text{wave}} = 1.5 \times 4.2 = 6.3\text{m}$$

Height of dam = $H_D = H_w + F.B$

$$= 78 + 6.3$$

$$H_D = 84.3\text{m}$$

 $a = 14\%$ of H_D

$$a = 0.14 \times 84.3 = 11.8\text{m}$$

3. Base Width "b" (Without offset)

i- For No Sliding Criteria

$$b' = \frac{H_w}{\mu G_1} = \frac{78}{0.7 \times 2.6} = 42.85 \approx 43\text{m}$$

ii) For No tension criteria.

$$b' = \frac{Hw}{\sqrt{G}} = \frac{78}{\sqrt{2.6}} = 49 \text{ m.}$$

4) Depth of vertical portion on U/S side.

$$h' = 2a \sqrt{G - c_u}$$

$$= 2 \times 11.8 \sqrt{2.6 - 0}$$

$$h' = 38 \text{ m}$$

5) Upstream offset

$$a/16 = \frac{11.8}{16} = 0.73 \text{ m.}$$

6) Depth below water level to end of inclined portion in US = $3.14 a \sqrt{G}$

$$= 3.14 \times 11.8 \sqrt{2.6}$$

$$= 59.7 \text{ m.}$$

7) Total width of base of dam

$$b = b' + a/16$$

$$= 49 + 11.8/16 = 49.7 \text{ m.}$$

$$8) \tan \theta = b'/H = \frac{49}{78} = \tan^{-1}(0.62)$$

$$= 32.13^\circ$$

9) Depth of vertical portion on D/S (from WL on U/S side).

$$\tan \theta = a/d' = \frac{11.8}{d'}$$

$$\frac{49}{78} d' = 11.8$$

$$d' = \frac{11.8 \times 78}{49}$$

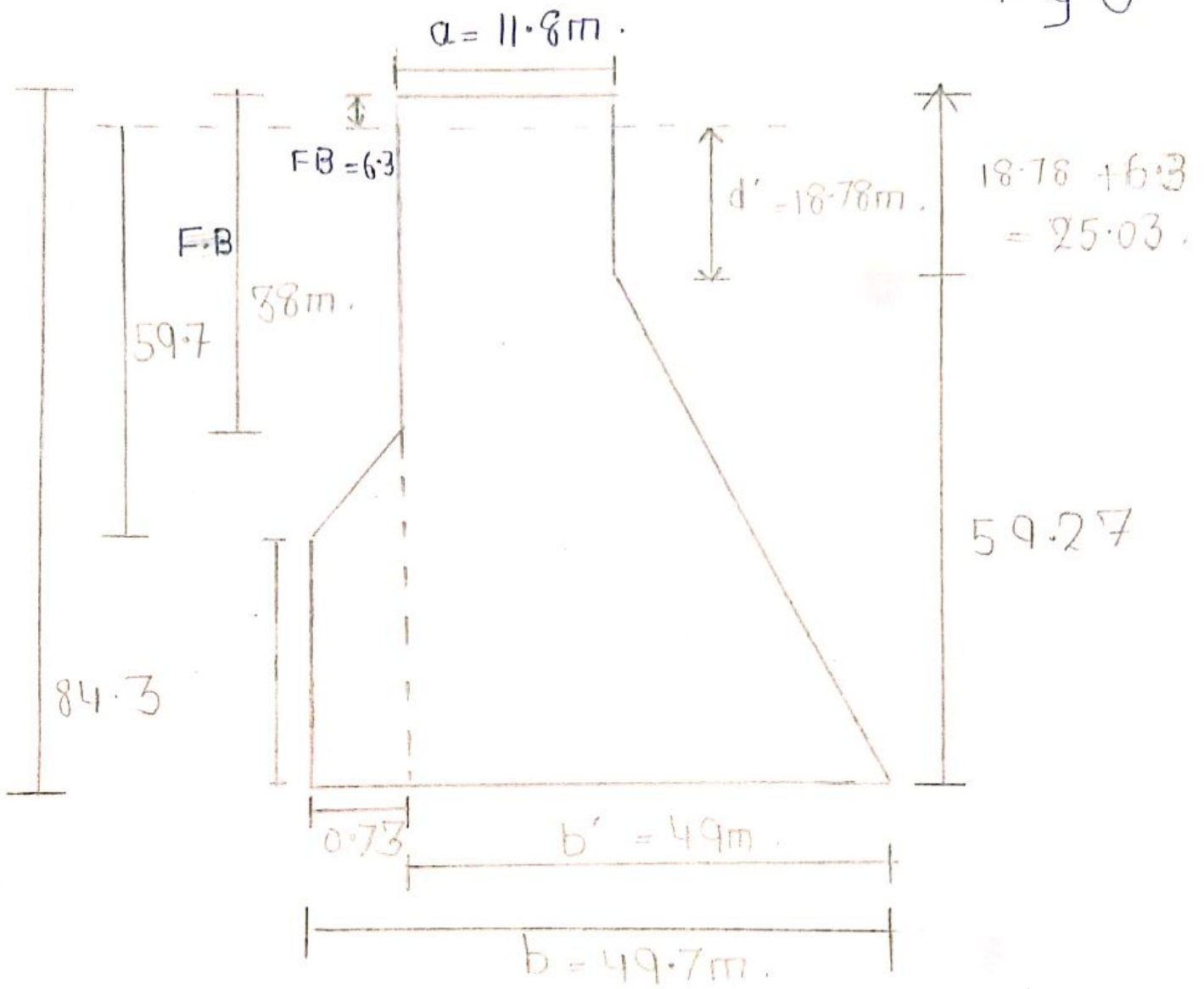
$$= 18.78 \text{ m.}$$

Depth of vertical portion

$$d = d' + F_B$$

$$= 18.78 + 6.3 = 25.08 \text{ m.}$$

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Dimension Analysis:-

If certain physical phenomena is governed by
 $f(x_1, x_2, \dots, x_n) = 0$

Where some/all of variables are dimensional

Then above phenomena can be represented as.

$(\pi_1, \pi_2, \dots, \pi_m) = 0$ Where all variables (π) are non-dimensional.

Buckingham Pi Theorem:-

$f(x_1, x_2, \dots, x_n) = 0 \Rightarrow \psi(\pi_1, \pi_2, \dots, \pi_m) = 0$

Where some/all x are dimensional

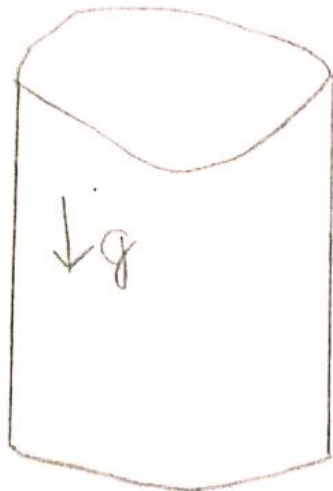
where all π are non-dimensional.

Where $m < n$, $m = n - k$.

Experiment shows for viscous flow

$$f(v, g, h, \nu) = 0$$

| | M | L | T |
|-------|---|---|----|
| v | 0 | 1 | -1 |
| g | 0 | 1 | -2 |
| h | 0 | 1 | 0 |
| ν | 0 | 2 | -1 |



$$n = 4 \quad k = 2, \quad m = 2$$

\checkmark L T $f(V, g, h, \nu)$ $n=4, k=2, m=2$ Pg 8

g 1 -2

h 1 0 Repeating variable - g, h.

\checkmark 2 -1 Non-repeating variable V, ν

$$\pi_1 = x_m (x_1)^{a_1} (x_2)^{a_2} (x_3)^{a_3} \dots (x_k)^{a_k}$$

$$\pi_1 = V(g)^a (h)^b$$

$$= L^0 T^0 = L T^{-1} (L T^{-2})^a (L)^b = L^{1+a+b} T^{-1-2a}$$

$$a = b = -1/2 \quad \pi = \frac{V}{\sqrt{gh}}$$

Similarly

$$\pi_2 = V(g)^a (h)^b$$

$$L^0 T^0 = L^2 T^{-1} (L)^b$$

$$2+a+b=0 = -1-2a$$

$$a = -1/2 \quad b = -3/2$$

$$\pi_2 = \frac{V}{\sqrt{gh}}$$

$$f(V, g, h, \nu) = 0 \quad f_1\left(\frac{V}{\sqrt{gh}}, \frac{V}{\sqrt{gh}^3}\right) = 0$$

$$\frac{V}{\sqrt{2gh}} = Fr \quad \text{Froude number}$$

$$\frac{V}{\sqrt{gh}^3} = \frac{V}{\sqrt{gh}} \frac{V}{\nu h} = \frac{Fr}{Re}$$

We may also write

$$f_2(Fr, Fr/Re) = 0 \quad Fr = \psi(Fr/Re)$$

viscous flow

$$Fr = \psi(Fr/Re)$$

Advantages of Dimension Analysis Pg 9

$$f(V, g, h, \nu) = 0 \Rightarrow Fr = \psi(Fr/Re)$$

$$Fr = \frac{V}{\sqrt{gh}} \quad \frac{Fr}{Re} = \frac{V}{\sqrt{gh}^3}$$

- Less number of experiments are necessary
- Experiment becomes inexpensive.
- Data reduction becomes easier.

Dimension analysis show

$$Fr = \psi(Fr/Re)$$

Q 4:

Fall velocity:-

When a grain falls down in still water it obtains a constant velocity when forward fluid drag force on the grain is equal to downward submerged weight of grain.

This constant velocity is defined as grain fall velocity of grain.

This is also called settling velocity.

Particle Density:-

Particle density effect fall velocity.

As with decreasing altitude air density increases at rate of 1% per 80 meters.

Viscosity of Water:-

Fluid velocity through porous media is approximately as inversely to kinematic viscosity. A decrease in viscosity therefore causes increase in velocity.

Turbulence of water:-

Turbulence effect fall velocity of water in reservoir because non-linearity and zig zag path effect flow of water and causes variation.

Particle Shape:-

Non-spherical particle fall upto 75% slower than equivalent sphere model. Non-spherical particle travels 44% faster spherical particle.

Particle concentration:-

When suspended concentration of sediment increases the velocity of each particle decreases due to modification of flow.

induced by other particles.

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Particle diameter:-

The diameter of a sphere particle have same specific gravity and terminal uniform setting velocity as given particle in same sedimentation.