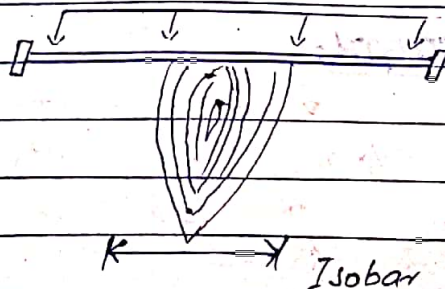


Q1 Define the following
(Part a)

ISOBAR:

- * It is a line or curve joining the points of same stresses (vertical) is termed as Isobar
- * Iso means same & bar means pressure / stress
- * It will always occur below the surface.
- * This will be the point where most of the deflection will be occurred due to vertical loads
- * It is contour of equal vertical stresses
- * The area covered by the isobar is called as pressure bulb.
- * Mostly the failure or settlement takes place here in this zone.
- * This is the zone in which the stresses have significant effect on the settlement of structure
- * The shape of isobar is just like on electric bulb or onion



EFFECTIVE STRESSES:

The stresses which are due to the self weight of the soil sample is termed as effective stresses " S_e "

Mathematically

$$S_e = \gamma \times z$$

γ = Unit weight of soil

z = depth at which vertical stresses are needed

COMPACTION:

• The process in which the soil particles are brought close to each other in order to improve the engineering properties of soil by some external efforts

• The compaction mostly take place in sand soil

• In this process the soil particles come close to each other because the air voids are reduced or removed in the soil

SHEAR STRENGTH:

• The resistance offered by the soil to the shear stresses before the failure of soil. It is termed

as shear strength

- ▽ Shear stresses produced when two surface slides with each other. The shear strength is the principal property of soil which directly or indirectly effect other properties of soil
- ▽ Such as bearing components more the shear strength of soil more will be its Bearing capacity
- ▽ Bearing capacity is ability of soil due to which it resists the load
- ▽ the failure of soil occur due to movement of partical soil no 1 due to breaking particals
- ▽ This shear strength is caused by the following factors.
 - the friction between the soil particals
 - the cohesion & adhesion b/cw the soil particals.
 - the structure of partical & its interlocking surface slides with eachother

SHEAR PARAMETER:

The shear strength cohesion & friction angle can be determined by different laboratory tests for different types of soil. Direct Shear test. Then horizontal force on shearing is applied till the failure

Q1 Write the assumptions of Boussinesq's
(Part B) load.

ANS: BUOSSINESQ'S THEORY :

This theory is based on following assumptions.

→ The soil medium is elastic
(the mould of elasticity throughout the soil sample)

→ The soil medium is homogenous
(the properties of soil are same at every point of the soil sample)

→ the soil is isotropic
(the properties of soil are same in every direction of soil sample)

→ the soil medium is semi infinite
(It is not total infinite)

$$S_z = \frac{Q}{z^2} \times KB$$

$$KB = \frac{3}{2\pi} \left[\frac{1}{1 + \left(\frac{r}{z}\right)^2} \right]^{5/2}$$

Q = load

z = depth at which vertical stresses is to be calculated

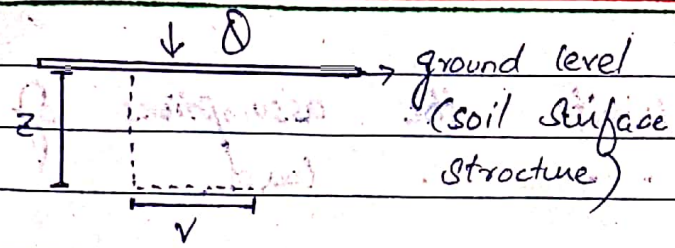
r = radial distance from the point of application of load to the point at which vertical stresses is to be calculated

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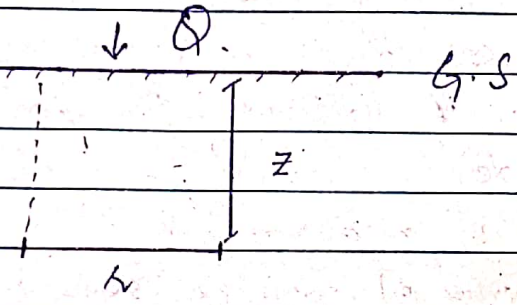
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CASE # 1



r is the distance b/w point where load is applied & the point where we are finding vertical stress.

$$S_z = \frac{Q}{z^2} \times K_B$$

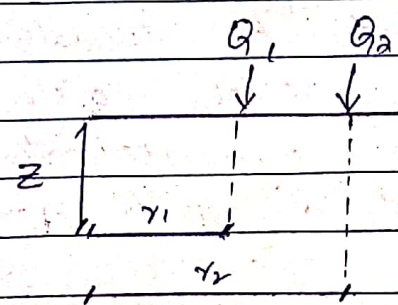
S_z = vertical stresses at depth z

Q = point or concentric load

K_B = Boussinesq's constant / coefficient

$$K_B = \frac{3/2 \pi}{\left[1 + \left(\frac{r}{z}\right)^2\right]^{5/2}}$$

CASE # 2



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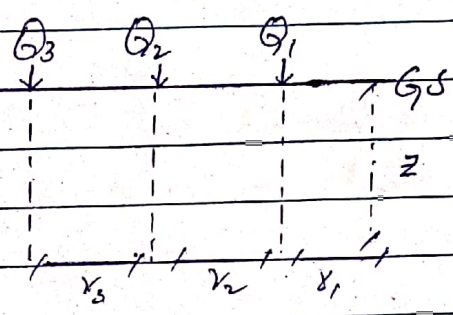
$$S_T = S_1 + S_2$$

$$= \left[\frac{Q_1 \times K_{B1}}{z^2} \right] + \left[\frac{Q_2 \times K_{B2}}{z^2} \right]$$

Total vertical stresses

$$\left[\frac{Q_1 \times \frac{3}{2} \pi}{z^2 \left[1 + \left(\frac{r_1}{z} \right)^2 \right]^{5/2}} \right] + \left[\frac{Q_2 \times \frac{3}{2} \pi}{z^2 \left[1 + \left(\frac{r_2}{z} \right)^2 \right]^{5/2}} \right]$$

CASE # 3



$$S_T = S_1 + S_2 + S_3$$

$$= \frac{Q_1 \times K_{B1}}{z^2} + \frac{Q_2 \times K_{B2}}{z^2} + \frac{Q_3 \times K_{B3}}{z^2}$$

$$= \left[\frac{Q_1 \times \frac{3}{2} \pi}{z^2 \left[1 + \left(\frac{r_2}{z} \right)^2 \right]^{5/2}} \right] + \left[\frac{Q_2 \times \frac{3}{2} \pi}{z^2 \left[1 + \left(\frac{r_2}{z} \right)^2 \right]^{5/2}} \right]$$

$$+ \left[\frac{Q_3 \times \frac{3}{2} \pi}{z^2 \left[1 + \left(\frac{r_3}{z} \right)^2 \right]^{5/2}} \right]$$

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Q2: (Part a) A sample of soil has a volume of 65 ml degree of saturation.

GIVEN:

$$V = 65 \text{ ml} = 0.000065 \text{ m}^3$$

$$W = 0.96 \text{ N}$$

$$W_d = 0.785 \text{ N}$$

$$G_s = 2.65$$

find:

$$S = ?$$

Sol:

$$\gamma_B = \gamma_w \times \frac{(G_s + e \times S)}{(1 + e)} \rightarrow \text{①}$$

As we know that $\gamma_B = \frac{W}{V}$

$$\gamma_B = \frac{W}{V} \Rightarrow \frac{0.96}{0.000065} \Rightarrow \gamma_B = 1469 \text{ N/m}^3$$

given value of $\gamma_w = 9800 \text{ N/m}^3$

$$e = \frac{V_v}{V_s}$$

As we know $\Rightarrow V_s = 0.000030 \text{ m}^3$

Also

$$\text{find } V_v \Rightarrow V = V_v + V_s$$

$$V_v = V - V_s$$

$$V_v = 0.000065 - 0.000030 \Rightarrow 0.000035 \text{ m}^3$$

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As we know that,

$$e = \frac{V_u}{V_s}$$

$$e = \frac{0.000035}{0.000030}$$

$$e = 1.167$$

As $\gamma_s = \frac{W_s}{V_s}$

But

$$G_s = \frac{\gamma_s}{\gamma_w}$$

$$\gamma_s = G_s \times \gamma_w$$

$$\gamma_s = 2.65 \times 9800$$

$$\gamma_s = 25970 \text{ N/m}^3$$

So $\gamma_s = \frac{W_s}{V_s}$

$$V_s = \frac{W_s}{\gamma_s}$$

$$V_s = \frac{W_s}{\gamma_s}$$

$$V_s = \frac{0.785}{25970.1}$$

$$V_s = 3.022 \times 10^{-5}$$

Put all the values in eq (1)

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eq - ①

$$Y_B = Y_w \times \frac{(G_s + e \times S)}{(1 + e)}$$

$$S = \frac{G_s \times Y_B - e}{e + Y_w}$$

$$S = \frac{2.65 \times 14769 - 1.167}{9800 - 1.167}$$

$$S = 0.527 \quad \text{or} \quad 52.7\% \text{ Answer}$$

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(10)

Q2: The following data was
(Part B) the compaction curve

Sol:

Soil sample (grams) Water content (%)

1890	5
2140	8
2170	9
2210	11
2220	12
2160	15
2070	20

As we know the volume of mould
= 950 cm^3

Weight of Sample (grams)	Water content (%)	Bulk unit wt (g/cm^3)
1890	5	$1890/950 = 1.989$
2140	8	$2140/950 = 2.253$
2170	9	$2170/950 = 2.284$
2210	11	$2210/950 = 2.326$
2220	12	$2220/950 = 2.337$
2160	15	$2160/950 = 2.274$
2070	20	$2070/950 = 2.179$

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As we know that.

$$\gamma_d = \frac{\gamma_b}{1+w}$$

"eg"

$$\gamma_d = \frac{1.98}{1+0.05} = 1.87$$

γ_d (g/cm³)

1.894

2.086

2.095

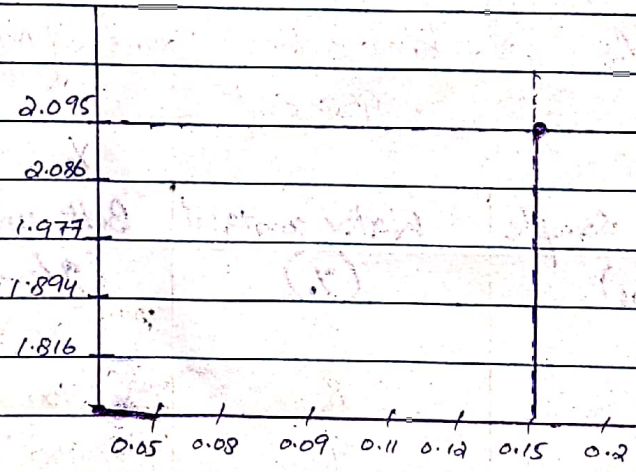
2.095

2.086

1.977

1.816

⇒ Compaction curve



(γ_d = Max value)

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QNO4 Explain CBR test investigation
(part a)

CALIFORNIA BEARING RATIO TEST: (CBR)

This method is used to find out the strength of subgrade used for the design of road. It is developed by California state of highway department.

In this method 5 kg of soil specimen is taken then water is added to it until it reaches to OMC then the CBR mould is cleared. Then mould is filled with prepared soil sample $\frac{1}{3}$ part of the mould is filled. The layer is compacted by giving 56 blows distribution. In this way the mould is filled in five layers. After the fifth layer the soil placement is stopped. Then the mould which containing the soil sample specimen is placed in CBR machine. Load is applied in such way that the penetration load value/rate is

1.25 mm/min or 0.05"/min

CBR value = $\frac{\text{load required for Penetration of the plunger in soil sample}}{\text{Standard load required for 0.1" Penetration of plunger in standard Material (Crushed stones)}} \times 100$

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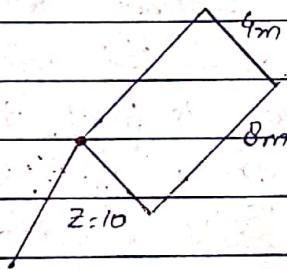
Vertical stresses under circular area

$$S_z = q + I_f$$

$$I_f = I_{c0} = \frac{1}{4} \left[1 + \left(\frac{D}{2z} \right)^2 \right]^{3/2}$$

$$I_f = I_c = \frac{1}{4} \left[1 + \left(\frac{r}{z} \right)^2 \right]^{3/2}$$

Vertical stresses under uniformly loaded rectangular or square area



$$S_z = \frac{q}{4\pi} \left[\frac{2mn(m^2+n^2+1)^{1/2} + (m^2+n^2+2)\tan^{-1}x}{(m^2+n^2+m^2n^2+1) - (m^2+n^2+1)} + \frac{2mn(m^2+n^2+1)^{1/2}}{(m^2+n^2-m^2n^2+1)} \right]$$

$$m = \frac{L}{z}, \quad n = \frac{B}{z}$$

the longer side will be L
the shorter side will be B

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Qo
(Part B)

Explain the following in detail

PROBING:

- * It consist of rod.
- * The dia of rod is $\frac{1}{4}$ - 1"
- * Having a handle at the top of apparatus, for pushing in the and out purpose

AUGER BORING:

This is performed by

- Hand operated Auger
- Power operated Auger
- This is simple method of soil exploration
- Max depth for this exploration 10m (32')

TEST PITS:

- ∅ Max depth of this pit is 5m-6m
- ∅ This test pit are performed where the bore is difficult
- ∅ Specially in case of gravelly soil

WASH BORING:

- ∆ It consist of steel pipe
- ∆ Its diameter equal to 2" - 8"

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- △ Its length is equal 5' to 10'
- △ In this method a pump is used to pump out soil + water is removed then the soil is tested in labs.

PERCUSSION BORING:

- + This percussion boring is performed upto a depth of 25m
- + this is also called cable tool drilling
- + This is the method of heavy equipment
- + This is an expensive expensive method, time consuming boring