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Section A

Hydraulic Engg

Assignment # 01

02

03

ASSIGNMENT 1

Ans 1 Venturi flume:

A venturi flume is a critical flow open flume with a constricted flow which causes a drop in the hydraulic grade line, creating a critical depth.

Due to solid matter contained in sewage, the discharge in sewer technology is usually measured with a venturi flume. This flume corresponds to a locally constricted channel normally without a bottom inset to force a transition between sub and supercritical flows.

Ans 2 Given data:

$$b = 3\text{m}, \quad Q = 12\text{m}^3/\text{sec}$$

a) Discharge per unit width:

$$q = \frac{Q}{b} = \frac{12}{3} = 4\text{m}^2/\text{sec}$$

then regular channel

$$h_c = \left(\frac{q^2}{g} \right)^{2/3} = \left(\frac{4^2}{9.81} \right)^{2/3}$$

$$h_c = 1.177\text{m}$$

$$\boxed{\text{critical depth} = 1.177\text{m}}$$

b) For rectangular channel:

$$E_c = \frac{3}{2} h_c$$

$$= \frac{3}{2} \times 1.17$$

$$\boxed{\text{mini specific energy} = 1.77}$$

③ As $E > \bar{E}$, there are two possible depths for a given specific energy.

$$\bar{E} = h + \frac{V^2}{2g}$$

$$\therefore V = \frac{Q}{A} = \frac{q}{h}$$

$$E = h + \frac{q^2}{2gh^2}$$

Substituting values in m-s units

$$4 = h + \frac{0.8155}{h^2}$$

For the subcritical (slow, deep) solution, the first term associated with potential energy.

$$h = 4 - \frac{0.8155}{h^2}$$

e.g. $h=4$ gives $h=3.98$ m

For the supercritical flow

$$h = \sqrt{\frac{0.8155}{4-h}}$$

$$h = 0.481 \text{ m}$$

Hence alternate depth are
3.95 m & 0.481

ASSIGNMENT 2

ANS 1 Given data:

$$d = 10 \text{ cm} \quad , \quad V = 6 \text{ m/s}$$

$$y_{\text{fall}} = ?$$

Solution:

By checking Froude
solution number

$$F_r = \frac{V}{\sqrt{g d}} \Rightarrow \frac{6}{\sqrt{9.81 \times 0.1}}$$
$$= 6.06$$

$$F_r = 6.06 > 1$$

Here flow is super-critical

$$E = y + \frac{v^2}{2g} = 0.1 + \frac{6}{2 \times 9.81}$$

$$E = 1.935 \text{ m}$$

For alternate depth

$$E = 1.935 \text{ m}$$

$$\boxed{y_{al} = 1.93}$$

Ans 2 Given data:

$$V_1 = 2 \text{ m/s}$$

$$y_1 = 3 \text{ m}$$

$$\Delta_2 = 60 \text{ cm} = 0.6 \text{ m}$$

$$\text{down step} = 15 \text{ cm} = 0.15 \text{ m}$$

Solution:

$$E_1 = y_1 + \frac{V_1^2}{2g}$$

$$= 3 + \frac{2^2}{2 \times 9.81}$$

$$\boxed{E_1 = 3.20 \text{ m}}$$

Now

$$E_2 = E_1 - \Delta_2$$

$$3.2 - 0.6$$

$$E_2 = 2.60 \text{ m}$$

Also

$$E_2 = y_2 + \frac{v^2}{2gy_2^2}$$

$$2.60 = y_2 + \frac{6^2}{2 \times 9.81 y_2^2}$$

$$y_2 = 2.24 \text{ m}$$

$$\Delta y = y_2 - y_1$$
$$= 2.24 - 3$$

$$\Delta y = -0.76 \text{ m}$$

So water surface drop
= 0.16 m

\Rightarrow For a downward stop of 15 cm
or 0.15 m we have

$$E_2 = E_1 - \Delta z = 3.20 - (0.15)$$

$$\boxed{E_2 = 3.35 \text{ m}}$$

Now

$$y_2 = 3.17 \text{ m}$$

$$\Delta y = y_2 - y_1 = 3.17 - 3$$

$$\Delta y = 0.17 \text{ m}$$

So water surface rise = 0.02 m

\Rightarrow The maximum upstream possible before effecting upstream water surface level is for

$$y_2 = y_c$$

$$y_c = 3 \sqrt{\frac{qv^2}{g}}$$

$$y_c = 3 \sqrt{\frac{b^2}{9.18}}$$

$$\boxed{y_c = 1.54 \text{ m}}$$

ASSIGNMENT 3

Ans 1 Given data:

$$y_1 = 3.6 \text{ m}, y_2 = 0.9 \text{ m}$$

$$b = 3.9 \text{ m}$$

Required:

a) Discharge $Q = ?$

b) Find no upstream & downstream.

Solution:

$$E_1 = E_2$$

$$y_1 + \frac{V_1^2}{2g} = y_2 + \frac{V_2^2}{2g} \rightarrow \textcircled{1}$$

Now

$$Q = A_1 V_1 = A_2 V_2$$

$$b y_1 V_1 = b y_2 V_2$$

$$y_1 V_1 = y_2 V_2$$

$$V_2 = \frac{y_1}{y_2} \times V_1$$

$$V_2 = \frac{3.6}{0.9} \times V_1$$

$$V_2 = 4 V_1 \rightarrow \textcircled{2}$$

put eq ② in ①

$$y_1 + \frac{v_1^2}{2g} = y_2 + \frac{v_2^2}{2g}$$

$$3.6 + \frac{(v_1)^2}{2g} = 0.9 + \frac{(4v_1)^2}{2g}$$

$$\frac{(v_1)^2}{2g} - \frac{16v_1^2}{2g} = 0.9 - 3.6$$

$$\frac{v_1^2 - 16v_1^2}{2g} = -2.7$$

$$\frac{-15v_1^2}{2g} = -2.7$$

$$\sqrt{v_1^2} = \sqrt{\frac{2.7 \times (2 \times 9.81)}{15}}$$

$$v_1 = 1.879 \text{ m/s} \rightarrow \text{③}$$

put ③ in ①

$$v_2 = 4v_1$$

$$v_2 = 4(1.879)$$

$$V_2 = 7.516 \text{ m/s}$$

$$a) A_1 V_1 = b y_2 V_2$$

$$= 3.9 \times 3.6 \times 1.879$$

$$Q_1 = 26.38 \text{ m}^3/\text{sec}$$

$$Q_2 = A_2 V_2 = b y_2 V_2$$

$$= 3.9 \times 0.9 \times 7.516$$

$$Q_2 = 26.38 \text{ m}^3/\text{sec}$$

$$Q = Q_1 = Q_2 = 26.38 \text{ m}^3/\text{sec}$$

b) Froude number at upstream

$$Fr_1 = \frac{V_1}{\sqrt{g y_1}} = \frac{1.879}{\sqrt{9.81 \times 3.6}}$$

$$Fr_1 = 0.31$$

Subcritical flow

b. Froude number \rightarrow at
downstream side

$$Fr_2 = \frac{V_2}{\sqrt{g y_2}} = \frac{7.516}{\sqrt{9.81 \times 0.9}}$$

$$\boxed{Fr_2 = 2.52} \quad \text{supercritical flow}$$