

Q. Differentiate b/w culvert and causeway.

Ans :- Culvert:

A culvert is a structure that allows water to flow under a road, railroad, trail or similar obstruction from one side to the other. Typically embedded so as to be surrounded by soil a culvert may be made from a pipe, reinforced concrete or other material.

Causeway:

A causeway is a track, road or railway on the upper point of an embankment across a low or wet place or piece of water. It can be constructed of earth, masonry, wood or concrete.

Q Define cross drainage works. why it is necessary?

Explain types of cross drainage

Ans: Irrigational canals while carrying water from headworks to crop field have to cross few natural drainage streams, nullahs etc. To cross those drainages safely by the canal some suitable structures are required to construct. Work required to construct to cross the drainage one called cross drainage works.

Necessary of cross drainage works:

- ⇒ The water shed canals do not cross natural drainage. but in actual orientation of the canal network, this ideal condition may not be available and the obstacles like natural drainage may be present across the canals. So the cross drainage works must be provided for running the irrigation system
- ⇒ At the crossing point the water of canal and the drainage get intermixed. So for the smooth running of the canal with its design discharge the cross drainage works are required
- ⇒ The site condition of the crossing point may be such that without any suitable structure, the water of the canal and drainage can not be diverted to their natural direction. So the cross drainage works must be provided to maintain their natural direction of flow.

Types of Cross Drainage Works.

Type I (Irrigation canal passes over the drainage)

- (a) Aqueduct
- (b) Siphon Aqueduct.

Type II (Drainage passes over the irrigation canal)

- (a) Super passage.
- (b) Siphon super passage.

Type III (Drainage and canal intersection each other of the same level)

- (a) Level crossing
- (b) Inlet and outlet.

Type I

(a) Aqueduct:

The hydraulic structure in which the irrigation canal is taken over the drainage i.e. (river, stream) etc. is known as aqueduct. This structure is suitable when bed level of canal is above the highest flood level of drainage.

(b) Siphon Aqueduct:

In a hydraulic structure where the canal is taken over the drainage, but the drainage water cannot pass clearly below the canal. It flows under siphonic action so it is known as siphon aqueduct.

Type II

(a) Super Passage:

The hydraulic structure in which the drainage is taken over the irrigation canal is known as super passage.

(b) Siphon Super passage:

The hydraulic structure in which the drainage is taken over the irrigation canal but the canal water passes below the drainage under the siphonic action is known as siphon super passage.

Type III

(a) Level crossing:

When the bed level of canal and the stream are approximately the same and quality of water in canal and stream is not much different.

(b) Inlet and outlet:

When irrigation canal meets a small stream or drain at same level, drain is allowed to enter the canal in inlet. At some distance from this inlet point a part of water is allowed to drain as outlet, which eventually meets the original stream.

Q Differentiate between weir and Barrage?

<u>Ans</u>	<u>Weir</u>	<u>Barrage</u>
①	Low cost	High cost.
②	Low control on flow	Relatively high control on flow and water levels by operation of gates.
③	No provision for transport communication across the river	Usually a road or a rail bridge can be conveniently and economically combined with a barrage wherever necessary.
④	Chances of silting on the upstream is more	Silting may be controlled by judicious operation of gates.
⑤	Afflux created is high due to relatively high weir crest	Due to low crest of the weir the flow during high floods is low. Since the gates may be lifted up fully even above the high flood level.

Q, Define Reynold's number. What will be the limit of Reynold's number for laminar, turbulent and neither laminar nor turbulent flow? Also define lower and higher critical velocity.

Ans Reynold's numbers,

The Reynold's number is the ratio of inertial forces to viscous forces within a fluid which is subjected to relative internal movement due to different fluid velocities.

If the Reynold's number is less than 2000, the flow is considered to be laminar. and is turbulent if it is greater than 4000.

When flow occurs between the laminar and Turbulent flow condition (Re 2300 to Re 4000) the flow condition is known as critical and is difficult to predict.

Here the flow is neither wholly laminar nor wholly turbulent.

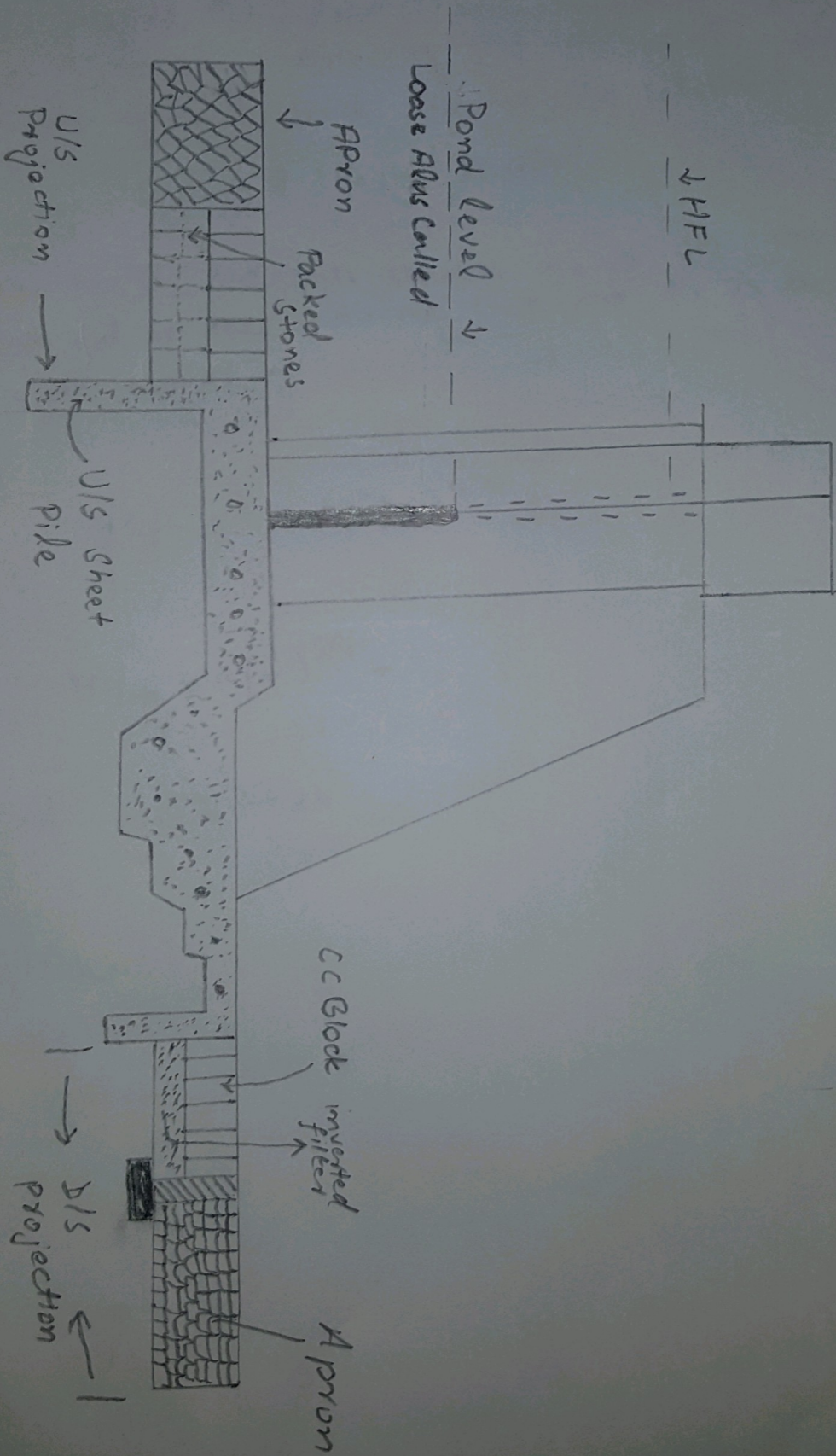
Lower and higher critical velocity:

The velocity at which the flow enters from laminar to transition period is known as lower critical velocity. The transition period is between laminar and turbulent flow.

higher critical velocity:

A velocity at which turbulent flow starts. or the velocity in which flow enters from transition period to turbulent flow is known as upper or higher critical velocity.

⇒ BARRAGE



Q How would you predict/analyse maximum or equilibrium scour depth based on experimental formulas

Ans Several formulae based on experimental results have been proposed to predict the maximum or equilibrium scour depth (y_s , below general bed level) around bridge pier. In general these assume the relationship

$$y_s/b' = \phi (y_0/b', Fr, d/b')$$

where b' is the pier width y_0 is the upstream flow depth, d is the sediment size and Fr is the flow Froude number.

$$y_s/b' = 4.2 (y_0/b')^{0.78} Fr^{0.52}$$

$$y_s/y_0 = 13 (b')^{5/7} - 1$$

$$y_s = 2.314 d b'$$

Q4

Given data:

$$L \cdot L = 15 \text{ kip/ft}^2 = 1500 \text{ lb/ft}^2$$

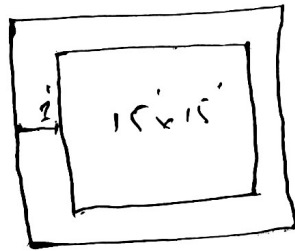
$$D \cdot L = 300 \text{ lb/ft}^2$$

$$\gamma_w = 100 \text{ lb/ft}^3$$

$$\phi = 30^\circ$$

$$\text{Use} = 1:2:4 \quad \text{Concrete} = \text{M15}$$

$$f_y = 60 \text{ ksi}$$



(1)

$$W = D \cdot L + L \cdot L + \text{self wt of slab}$$

$$\text{Self wt of top slab} = 1 \times 15 = 15 \text{ lb/ft}^2$$

$$W = 1500 + 300 + 15 = 1815 \text{ lb/ft}^2$$

(2)

$$K_a = \frac{1 - \sin \phi}{1 + \sin \phi}$$

$$= \frac{1 - \sin(30^\circ)}{1 + \sin(30^\circ)}$$

$$K_a = \frac{1.866}{0.133} = 14.03'$$

(3) Lateral pressure due to $(L \cdot L + D \cdot L)$

$$= (L \cdot L + D \cdot L) \times K_a$$

$$= (1500 + 300) \times 14.03$$

$$= 25254 \text{ lb/ft}^2$$

$$= \boxed{25.254 \text{ kip/ft}^2}$$

(4) Lateral pressure due to soil

$$= K_a \times \gamma H$$

$$= 14.03 \times 100 \times 16$$

$$= 22448 \text{ lb/ft}^2$$

$$= \boxed{22.448 \text{ kip/ft}^2}$$

(5) Lateral pressure @ top =
Lateral pressure due to $(D \cdot L + L \cdot L) = \boxed{25.254 \text{ kip/ft}^2}$

~~$(D \cdot L + L \cdot L) = 25.254$~~

(6) Lateral pressure @ bottom
= Lateral pressure due to $(D \cdot L + L \cdot L)$ + total
pressure due to soil.

$$= 25.254 + 22.448$$

$$= \boxed{47.702 \text{ kip/ft}^2}$$

