

FINAL EXAMINATION

NAME : BAST KHAN

ID : 7812

SECTION : A

SUBJECT : IRRIGATION ENGINEERING

TEACHER NAME : PROF. DR. JAHANGER DORRANI

QNO: 01

Part: a

Anti Water - Logging measures

Following are the anti water measures.

1 - Lining of Canals & water Course:

Lining of canal with imperious material e.g: Concrete, polythene etc so it to control excessive seepage from unlined Canal

2 - Reducing intensity of irrigation:

A small portion of Land should be received Canal water in one proper season remaining area can be received water in the next season by rotation.

3 - Optimum use of water:

Cultivatos / farmer should be educated so that to use less water

4 - Improving natural drainage of area:

water should not allow to stay in one area.

5- Pumping or Tubewells or Vertical Drainage:

Water Logging Can be Controlled by Left irrigation Canal irrigation substituted by tube-well irrigation.

6- Adoption of Sprinkler Method of irrigation:

Water Logging Can also be controlled by Adoption of Sprinkler Method of irrigation

Q1 **Part : b** : Differentiate btw Saline and alkaline Soils:

The main difference between Saline alkaline Soils is that Saline Soils have a pH Less than 8.5 and an exchangeable Sodium percentage Less than 15, while alkaline Soils have a pH greater than 8.5 and an exchangeable Sodium percentage higher than 15

Meanwhile neutral Soil have pH 7.

→ Colour of the Saline is white or light gray while the Colour alkaline Soil is mostly black Colour. Organic Contents in Saline Soil is high. white alkaline is Low.

Q:1

Part : C Reclaim Salt affected Lands

→ There are different salts which are soluble in water like Sodium Chloride (NaCl) Sodium Sulphate (Na_2SO_4) and Na_2CO_3 .

→ When water table rises up or roots are in Capillary zone the ground water, moves upward and salts are deposited in root zone and surface of soil and it can be look a thin crust on the surface after evaporation of water and it affects the soil badly. *

→ The Land is said to have Reclaimed when Sodium Carbonate is present in the soil, Gypsum is added before leaching

→ Sodium Sulphate is formed which is leached out easily

Q.No: 02 :

part: a :

Following are some steps procedure for designing.

Step: 01 : $Q = Av \rightarrow \textcircled{1}$

Assumption the trial value of D and put in eq $\textcircled{1}$

$$V_0 = 0.546mD^{0.64}$$

Step: 02 From equation $\textcircled{1}$

$$A = Q/v$$

$$\text{where } A = BD + D^2/2$$

$$P = B + D5^{1/2}$$

For assumed D determine B

$$\text{Find } R = A/p$$

Step: 03 Substitute the value of R in eq 2 (Kutter's and Chazy's Eqn) to obtain v which will be the actual velocity for assumed dimension

Step: 04: If the velocity worked out from Eqn. 2 agrees with that of obtained with the Eqn. 3 (Kennedy's Eqn) Then the assumed depth is correct. otherwise Repeat the procedure with changed value of D.

Q: 02

Part: b

Given that:

$$\text{Discharge} = 30 \text{ cumecs}$$

$$m = 1$$

$$\text{Slope} = S = 1/5000$$

Assume: Depth = D = 2.3m

Kutter's rugosity Coefficient = N = 0.0225

Critical velocity relation $m = 1$

Assume: Side Slope = Z = $\frac{1}{2}$

Solution: $V_k = 0.55mD^{0.64}$ $V_k = \text{velocity}$

$$V_k = 0.55 \times 1 \times (2.3)^{0.64} \Rightarrow V_k = 0.94 \text{ m}$$

$$\text{Area} = A \frac{D}{V_k} = \frac{30}{0.94} \Rightarrow A = 31.91 \text{ m}^2$$

Calculate B ,

$$A = BD + 0.5D^2$$

$$\text{For } z = \frac{1}{2} = 0.5$$

$$31.91 = (B + (2.3) + 0.5(2.3)^2)$$

$$31.91 = 2.3B + 2.64$$

$$2.3B = 31.91 - 2.64$$

$$B = \frac{29.27}{2.3} \Rightarrow \boxed{B = 12.72\text{m}}$$

Wetted Perimeter & Hydraulic mean depth

$$P = B + \sqrt{5}D$$

$$P = 12.72 + \sqrt{5}(2.3)$$

$$\boxed{P = 17.86\text{m}}$$

$$R = \frac{A}{P} = \frac{31.91}{17.86} = 1.78\text{m}$$

$$\boxed{R = 1.78\text{m}}$$

V_c = Roughness Coefficient

$$V_c = \frac{23 + \frac{1}{N} + \frac{0.00155}{S}}{1 + \left(\frac{23 + \frac{0.00155}{S}}{2.5}\right) \frac{N}{\sqrt{R}}} \sqrt{RS}$$

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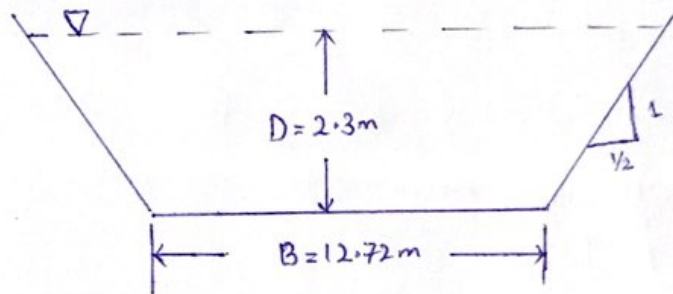
$$V_c = \frac{23 + \frac{1}{0.0225} + \frac{0.00155}{\sqrt{5000}}}{1 + \left(23 + \frac{0.00155}{\sqrt{5000}}\right) \frac{0.0225}{\sqrt{1.78}}} \left(\sqrt{1.78} \times \frac{1}{5000} \right)$$

$$V_c = \frac{23 + 44.44 + 7.75}{1 + (23 + 7.75) 0.0168} (0.000264) = \frac{75.19}{1.51} (0.00026)$$

$$V_c = 0.14 \text{ m}$$

$$B = 12.72 \text{ m}$$
$$D = 2.3 \text{ m}$$

Q No. 63



Q No: 03

Part: a :-

Lacey's Theory differentiated between initial regime and final regime but this theory is applicable to final regime

Initial Regime :

→ when only bed slope of channel changes but the cross section remains same then also no silting or scouring takes place too. But this is rare cases.

→ The shape of the regime channel is semi elliptical for coarse silt and semi circle for fine silt.

Final Regime : when a channel is constructed (with defective slope) it tries to throw off the incoherent silt on the bed to increase their slope and attain longitudinal slope.

→ The channel after attaining its section and longitudinal slope, will be said to be in final regime

Q: 03

09

Part: b :-

Given data:

$$\text{Discharge} = 30 \text{ Cumecs}$$

d_{mm} = mean diameter of silt particles

$$= 0.56 \text{ mm}$$

Solution:

By Lacey's Theory

$$f = 1.76 \times \sqrt{d_{mm}} \quad (\Rightarrow f = \text{Silt Factor})$$

$$f = 1.76 \times \sqrt{0.56} = \boxed{f = 1.317}$$

$$V = \left(\frac{Qf^2}{140} \right)^{1/6} \quad \text{By putting the values}$$

$$V = \left(\frac{30 \times (1.317)^2}{140} \right)^{1/6} \Rightarrow V = \left(\frac{0.372}{140} \right)^{1/6}$$

$$\boxed{V = 0.848 \text{ m/sec}}$$

$$A = \frac{Q}{V} \Rightarrow A = \frac{30}{0.848}$$

$$\boxed{A = 35.377 \text{ m}^2}$$

$$P = 4.75 \sqrt{Q} \quad (\text{Formula})$$

$$P = 4.75 \sqrt{30} \Rightarrow P = 26.01 \text{ m}$$

P-T-O

(10)

Q.3 part b continue

$$D = \frac{P - \sqrt{P^2 - 6.944A}}{3.742}$$

by putting values

$$D = \frac{26.01 - \sqrt{(26.01)^2 - 6.944(37.04)}}{3.742}$$

$$D = \frac{26.01 - \sqrt{676.52 - 257.20}}{3.742}$$

$$D = \frac{26.01 - \sqrt{419.32}}{3.742}$$

$$D = \frac{26.01 - 20.477}{3.742} \Rightarrow \frac{5.533}{3.742}$$

$$\boxed{D = 1.478 \text{ m}}$$

$$B = P - \sqrt{5D}$$

$$B = 26.01 - \sqrt{5 \times 1.478}$$

$$\boxed{B = 22.70 \text{ m}}$$

$$R = \frac{S}{2} \frac{V}{f} \Rightarrow R = \frac{S}{2} \left(\frac{0.848}{1.317} \right)$$

$$\boxed{R = 1.609 \text{ m}}$$

P-T-O →

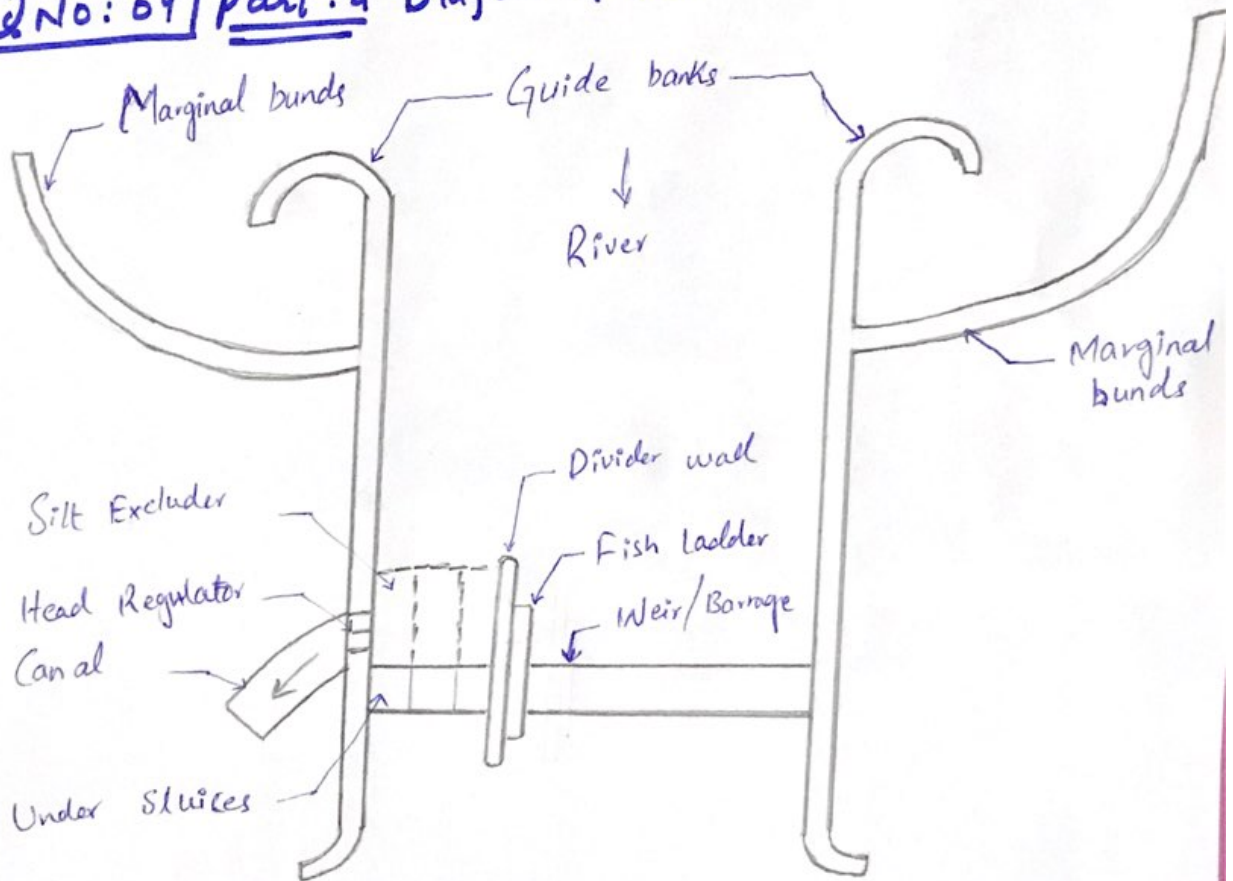
$$S = \frac{f^{\frac{5}{3}}}{3340 Q^{\frac{1}{6}}} \quad \text{--- (Formula)}$$

by putting values

$$S = \frac{(1.317)^{\frac{5}{3}}}{3340 (30)^{\frac{1}{6}}} = \frac{1.58}{5886.19}$$

$$S = 0.000268 \quad \text{Ans}$$

Q No: 04 Part: a Diagram of Headworks with components



Part: a :- Components of headwork :

- ① - Weir :
It is a wall/dam type which is constructed across a river or stream which is used to raise the level of water flows over the top of weir.
- ② - Barrage : It is the type of low head, diversion dam which consists of a number of large gates that can be open or closed to control the amount of water.
- ③ - Fish Ladder : It is a small elevated steps/wall with water releases to enable fish to migrate/move in the upper stream or down stream.
- ④ Divide wall : It is a long wall constructed at right angles to the weir or barrage on the upstream side. The wall is extended to cover the canal head regulator.
- ⑤ Silt excluder : It consists of a number of rectangular tunnels running parallel to the head regulator and terminating near the under sluiced weir.

⑥ - Under Sluices: They are the openings provided at the base of the weir or barrage. It is kept at a lower level of the weir.

⑦ - Guide Banks: It is defined as the site of barrages, weirs etc. to guide the river flow through the confined water way without causing damage to the structure and its approaches.

→ Q No: 04

Part: b

Function of Head Regulators:

- It regulates the supply of water entering the canal.
- It prevents the river floods from entering the canal.
- The entry of silt into the canal is controlled.
- Silt gets deposited in the pocket, and only the clear water enters the regulator bays.

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→ To serve as a meter for measuring discharge of water

→ Consists of a number of piers which divide the total width of the canal into a number of spans which are known as bays.

: THE END :