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Section A

Subject Advance fluid
Mechanics

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Wahed

Q. No 1

Part (a)

Define drag with its components
..... laminar & turbulent boundary
layers.

Ans:-

A body which is wholly immersed in a homogenous fluid may be subjected to two kind of forces arising from relative motion b/w body & fluid. These forces are termed as drag & lift depending on forces parallel or lift angle to motion.

Drag force on submerged body can have 2 components.

Pressure Drag:-

It is equal to integration of components in direction of motion of all pressure forces exerted on surface of body.

$$F_p = C_p \int \frac{1}{2} \rho v^2 \cdot A \quad (\text{where } C_p \text{ depend on shape}).$$

Friction drag:-

It is equal to integration of component of all shear stress along the surface in direction of motion.

$$F_p = c_f \cdot \frac{\rho V^2}{2} \cdot BL$$

(c_f depend on velocity)

Friction Drag of Boundary layers:-

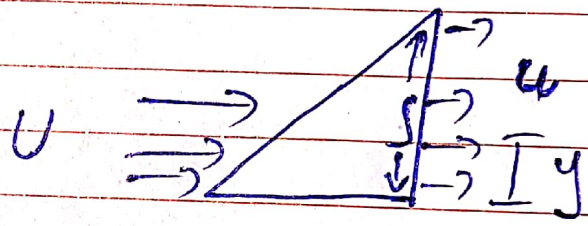
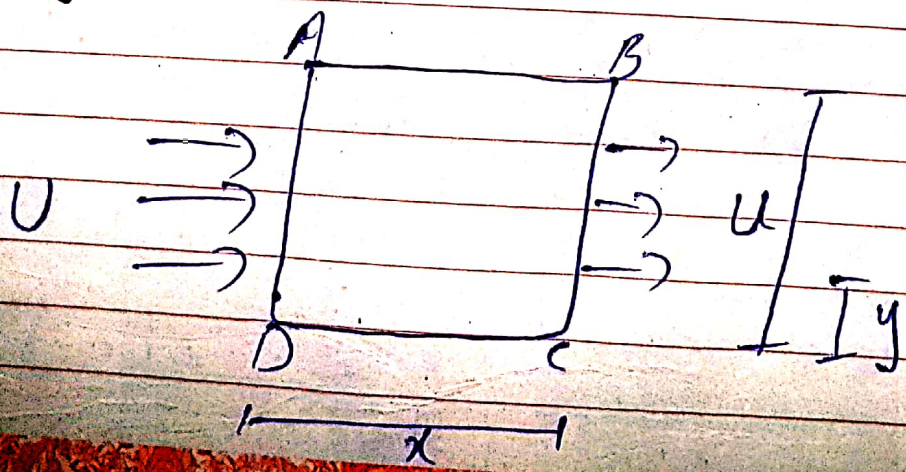


Figure shows growth of Boundary layer along one side of smooth plate in steady flow of incompressible fluid consider value δ where is the thickness of boundary layer & U is undisturbed velocity.



As we have $\sum F_x = 0$
where

$$F_u = \frac{\Delta P}{\Delta t} = \frac{\Delta m v}{\Delta t} \quad \therefore m = \rho V$$

$$F_u = \frac{\rho \cdot \text{Vol} \cdot V}{\Delta t} = \rho Q v$$

$$F_u = \rho Q v$$

- $F_u = \text{rate of change of } BC + AB - AD$

$$AD = \rho U (UBS)$$

$$BC = \rho \int_B (u^2 dy)$$

$$AB = \rho U (UBS) - \rho \int_0^B u dy$$

$$AB = \rho U (UBS) - \rho \int_0^B u dy$$

$$F_u = \rho \int_0^B (u - u) dy$$

integration on b/s $\rightarrow \text{①}$

where u is a function of boundary layer velocity distribution

Now to find shear stress

$$\tau = \frac{F_u}{A} = \frac{dF_u}{B du} = \frac{dF_u}{B du}$$

$$\bar{Z}_0 = \int \frac{\rho u^2 ds}{\rho dx} = \int u^2 \alpha \frac{ds}{dx}$$

$$\bar{Z}_0 = \int u^2 \alpha \frac{ds}{dx}$$

Laminar boundary layer:-

$$\frac{u}{U} = f\left(\frac{y}{\delta}\right) \rightarrow \textcircled{1}$$

$$\frac{y}{\delta} = \eta \Rightarrow y = \delta \eta$$

$$dy = \delta d\eta \rightarrow \textcircled{2}$$

$$\frac{u}{U} = f(\eta)$$

$$du = U df(\eta) \rightarrow \textcircled{3}$$

For laminar flow

$$\bar{Z}_0 = \mu \frac{du}{dy} \rightarrow \textcircled{4}$$

$$\bar{Z}_0 = \mu U \frac{df(\eta)}{\delta d\eta}$$

$$\bar{Z}_0 = \frac{\mu U \delta}{s} \rightarrow \textcircled{5}$$

As we have $\tau_0 = \rho u^2 \alpha \frac{ds}{du}$

Compare both

$$\rho u^2 \alpha \frac{ds}{dx} = \frac{\mu U B}{\delta}$$

$$\delta ds = \frac{\mu B dx}{\rho u^2 \alpha}$$

Int on b.s

$$\frac{\delta^2}{2} = \frac{\mu B}{\rho u^2 \alpha} x + C \quad \because C = 0$$

$$\delta = \sqrt{\frac{2B}{\rho}} \cdot \sqrt{\frac{\mu x}{\rho u^2 \alpha}} \quad \because Ru = \frac{U u \delta}{\mu}$$

$$B = 1.63, \quad d = 0.135$$

$$\delta = \frac{4.91 \mu}{\sqrt{R_x}} \rightarrow (6)$$

where (R_x) is local reynold no.

As we have

$$\tau_0 = \frac{\mu U B}{\delta}$$

Put eq (6) in (5)

$$\tau_0 = 0.332 \frac{\mu U}{u} R x$$

now $F_f = B \int_0^L \tau_0 du$

where $\tau_0 = 0.332 \frac{\mu U}{u} R x$

$$R x = \frac{u U^2}{u}$$

then putting values

$$F_f = 0.664 \sqrt{\mu U^3}$$

As we have

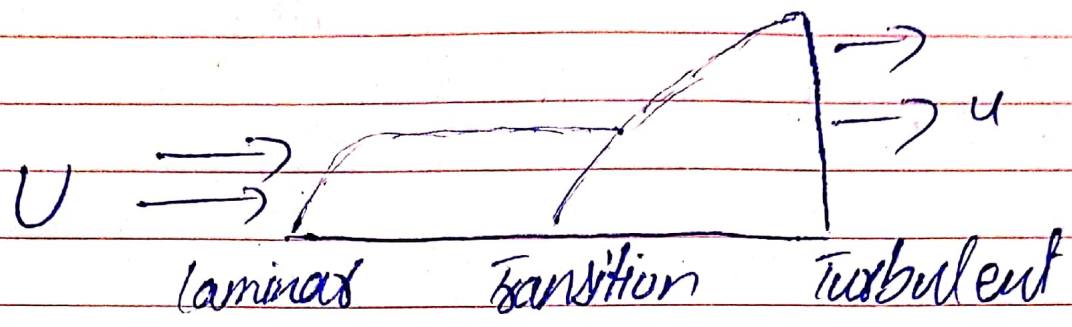
$$F_f = c_f \frac{\rho U^2}{2} B L$$

thus equating both side

$$c_f = \frac{1.328 \sqrt{\mu}}{\rho U} = \frac{1.328}{R x}$$

For laminar $R < 500,000$.

Turbulent Boundary layers:-



The figure shows that velocity distribution of boundary layer which is steeper near walls & flatter through out remainder of layer.

The shear stress is greater in turbulent than in laminar.

Thus $\tau_0 = 3f \frac{\rho V^2}{8}$

where V is the average velocity to obtain relation b/w average & τ_0 max we have.

$$\frac{V}{U_{max}} = \frac{1}{1 + 1.33\sqrt{f}} \quad \therefore f = 0.023$$

$$\frac{V}{U_{max}} = \frac{1}{1 + 1.33/\sqrt{0.023}}$$

$$U = 1.935 \text{ V}$$

$$v = \frac{U}{1.935}$$

$$\epsilon_f = \frac{0.316}{(R_h)^{3/4}}$$

$$\therefore R_h = \left(\frac{DV}{v} \right)$$

$$\tau_0 = \frac{f \int v^2}{8}$$

$$\tau_0 = \frac{0.316}{\left(\left(\frac{D}{v} \right) \left(\frac{0}{1.935} \right) \right)^{1/4}} \cdot \frac{f}{8} \left(\frac{U}{1.935} \right)^2$$

$$\tau_0 = \frac{0.0239 U^2}{\left(\frac{f}{v} \right)^{1/4}} \rightarrow \text{①}$$

As we have

$$\tau_0 = f U^2 \alpha \frac{d\delta}{dx} \rightarrow \text{②}$$

eq ① & ②

$$u=0 \quad , \quad \delta=0$$

$$\delta = \left(\frac{0.0287}{\alpha} \right)^{4/5} \left(\frac{v}{Uu} \right)^{1/5} \cdot x$$

$$\alpha = 0.0972$$

$$S = \frac{0.377}{(Rh)^{1/5}} \cdot \lambda \rightarrow \textcircled{3}$$

$$\tau_0 = 0.0587 S \frac{V^2}{g} \left(\frac{V}{Uu}\right)^{1/5}$$

$$F_f = B \int_0^L \tau_0 du$$

$$F_f = 0.0735 S \frac{U^2}{g} \left(\frac{V}{UL}\right)^{1/5} \cdot BL$$

$$F_f = c_f \cdot \frac{V^2}{g} BL$$

equating b/s

$$c_f = \frac{0.0735}{(R)^{1/5}} \quad (500000 < R < 10^7)$$

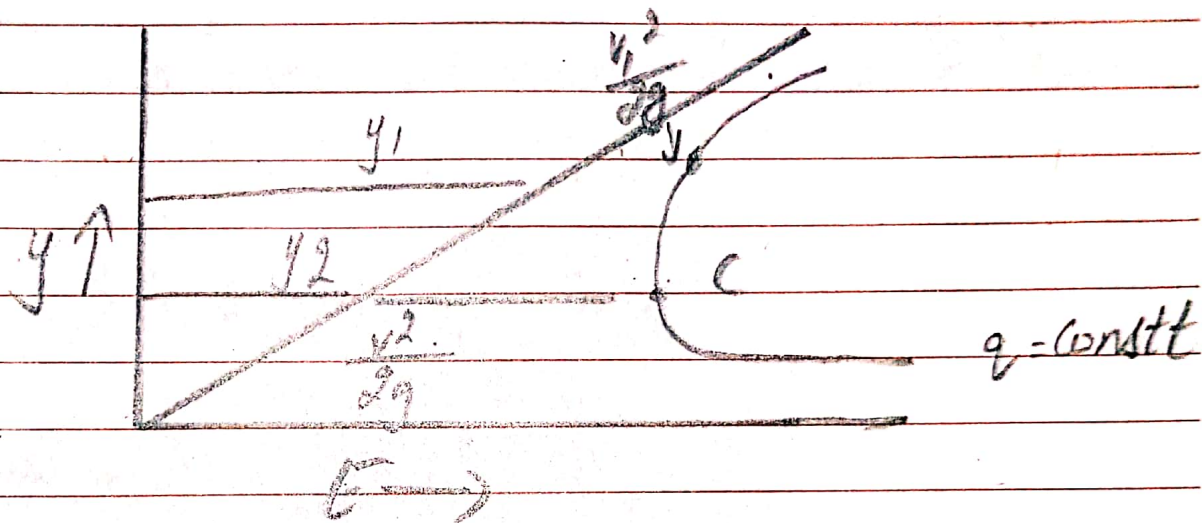
for $R > 10^7$

$$c_f = \frac{0.455}{(\log R)^{2.59}}$$

Q. No 1

Part (b)

Derive equation for critical section of a channel.



This is specific energy eq:-

For particular q , there will be two kind of possible values of y for given E . The eq is cubic with three roots with third being negative giving no values thus two alternative depths represents two totally different flow regions slow E_1 deep an upper position E_1 Fast E_1 shallow on lower position. Point represent dividing point between two regima of flow.

Thus for given 'q' value of E is minimum. Flow at this point is critical flow. Depth of flow at this point is critical depth. $1/c$ velocity at this point is critical velocity.

Thus relation of critical depth can be found as

$$E = y + \frac{1}{2g} \left(\frac{q^2}{y^2} \right)$$

For minimum specific energy

$$\frac{dE}{dy} = 0$$

$$\frac{dE}{dy} = 1 - \frac{q^2}{g y^3}$$

$$\frac{dE}{dy} = 1 - \frac{q^2}{g y^3}$$

$$1 = \frac{q^2}{g y^3} \quad \Rightarrow \quad q^2 = g y^3$$

$$y_{cr} = \left(\frac{q^2}{g} \right)^{1/3}$$

$$\text{As } q = Vy \quad , \quad Vc^2 = gy^3$$

$$\text{OR } Vc = \sqrt{gyc}$$

$$yc = \frac{Vc^2}{g}$$

$$\text{Now } \frac{yc}{g} = \frac{Vc^2}{g^2}$$

$$E_{min} = yc + \frac{Vc^2}{2g} = yc + \frac{yc}{2}$$

$$\frac{3}{2} yc \quad \text{OR} \quad ycb = \frac{2}{3} \text{ constt}$$

Depth of Flow Depth of Flow	Sub critical $y > yc$	Critical $y = yc$	Super critical $y < yc$
Velocity Slop	$V < Vc$ mild slop $S_0 < S_c$	$V = Vc$ critical slop	$V > Vc$

Q.2

Given :-

Depth of Rectangular channel (d) = ?

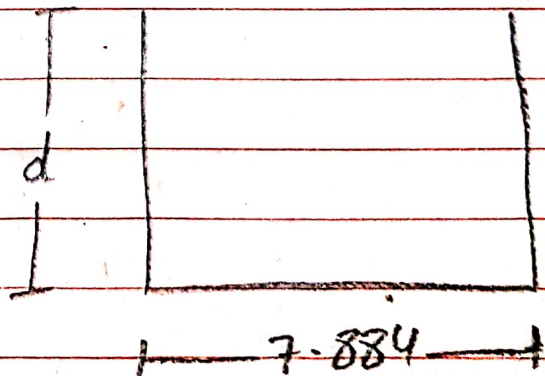
Flow rate (Q) = $3.5 \text{ m}^3/\text{sec}$ Slope of Bed (S_0) = 0.0008 $n = 0.0219$ Width of Bed = 7884 mm
= 7.884

Critical depth = ?

Flow sub critical or super critical = ?

Sol:-

$$\begin{aligned} \text{Area} &= 7.884 \times d \\ &= 7.884d \end{aligned}$$



$$\begin{aligned} \text{Perimeter} &= d + 7.884 + d \\ &= 7.884 + 2d \end{aligned}$$

$$\begin{aligned} \text{Hydraulic Radius (Rh)} &= A/P \\ &= \frac{7.884d}{7.884 + 2d} \end{aligned}$$

By Using Manning Equation

$$Q = \frac{1}{n} A R_h^{2/3} S_0^{1/2}$$

Putting values

$$3.5 = \frac{1}{0.0219} \times 7.884d \times \left(\frac{7.884d}{2d + 7.884} \right)^{2/3} \times (0.0008)^{1/2}$$

$$d = 0.555 \text{ m}$$

$$\text{Area} = 7.884(0.555)$$

$$= 4.37 \text{ m}^2$$

$$\text{Perimeter} = 7.884 + 2(0.555)$$

$$= 8.994 \text{ m}$$

$$\text{Hydraulic Radius (Rh)} = \frac{4.37}{8.994}$$

$$= 0.485 \text{ m}$$

Finding critical depth :-

$$y_c = \left(\frac{q^2}{g} \right)^{1/3}$$

$$\text{As } q = Q/B$$

$$= \frac{3.5}{7.884}$$

$$= 0.443 \text{ m}^2/\text{sec}$$

$$\Rightarrow y_c = \left(\frac{(0.443)^2}{9.81} \right)^{1/3}$$

$$= 0.271$$

$$\text{As } y > y_c$$

$$0.555 > 0.271$$

So Flow is sub-critical.

Q. No 3

Given data :-

$$\begin{aligned}\text{Wide of smooth plate} &= 900 \text{ mm} \\ &= 0.9 \text{ m}\end{aligned}$$

$$\begin{aligned}\text{Length of smooth plate} &= 800 \text{ mm} \\ &= 0.8 \text{ m}\end{aligned}$$

$$\text{Specific gravity} = 0.89$$

$$\text{Undisturbed velocity} = 5 \text{ m/s}$$

$$\text{Kinematic viscosity} = 0.93 \times 10^{-4} \text{ m}^2/\text{s}$$

Required :-

$$\text{Friction drag} = ?$$

Sol:-

$$\nu = 0.93 \times 10^{-4} \text{ m}^2/\text{s}$$

At $x = L$,

$$R = \frac{LU}{V}$$

$$= \frac{0.8 \times 5}{0.93 \times 10^{-4}}$$

$$= 43010.75$$

less than $R < 500,000$

Now $cf = \frac{1.328}{TR}$

$$= \frac{1.328}{43010.75}$$

$$= 6.40 \times 10^{-3}$$

$$= 0.0064$$

$$F_f = cf \int \frac{U^2}{2} BL$$

$$= 0.0064 \times 0.89 \times 1000 \times \frac{(5)^2}{2} \times 0.2 \times 0.8$$

$$F_f = \boxed{11.39 \text{ N}}$$

To find thickness of boundary layer

$$\frac{\delta}{x} = \frac{4.91}{\sqrt{143010.75}}$$

$$= 0.023$$

$$\delta = 0.023 \times 0.8 \rightarrow \text{full length}$$

$$= 0.0184 \text{ m}$$

$$\bar{F}_f = 0.664 \text{ BIP} \sqrt{L U^3}$$

$$= 0.664 \text{ BIP} (\rho \times v) L U^3$$

$$= 0.664 \times 0.2 \times 10.89 \times 1000 \times (0.89 \times 1000 \times 0.93 \times 10^{-4}) \times 0.8 \times (5)^3$$

$$= 11.39 \text{ N}$$