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Section: "A"

Semester 6th

Assignment 1 Hydraulic Engineering

Submitted To

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Assignment - 1

Q No: 1

What is venturi flume?
Explain with detail.

Venturi Flume:

A venturi flume is a critical flow open flume with a constricted flow which causes a drop in hydraulic grade line creating a critical depth.

It is used in flow measurement of very large flow rate usually gives in million of cubic unit.

A venturi meter would normally measure in meters.

Measurement of discharge with venturi flume required two measurement one upstream and one at a throat.

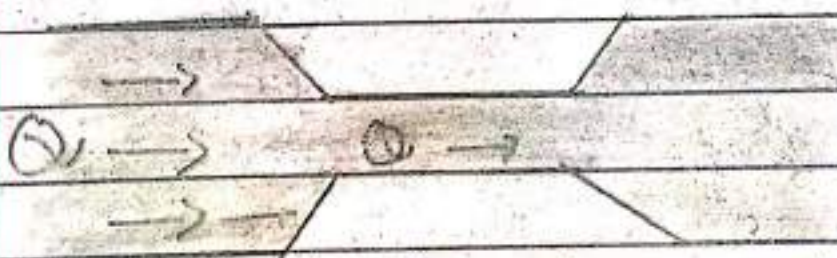
If the flume passes in a sub-critical static through flume

If the flow are designed so as pass flow from sub-critical to single measurement at throat is sufficient from computation of discharge

To ensure occurrence of critical depth at throat flumes are usually discharge in such a way from a hydraulic pump on down stream side of structure

These flumes are called standing wave flumes.

It causes drop in hydraulic gradient



Flow through a venturi flume

Assignment # 1

Q No: 2

A 3m wide channel carries total discharge of $12 \text{ m}^3/\text{sec}$

Calculate

- ① critical depth
- ② minimum specific energy
- ③ Alternative depth when $\Sigma = 4m$

Given data

width of channel (b) = 3m

Discharge $Q = 12 \text{ m}^3/\text{sec}$

Required

- a) critical depth
- b) minimum specific energy
- c) Alternative depth when $\Sigma = 4m$

Solve

① Critical Depth

$$As \Rightarrow q = Q/b$$

$$\frac{12}{3} = q$$

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$$Q = 4 \text{ m}^2 \text{ / sec}$$

Using formula

$$y_c = \left(\frac{Q^2}{g} \right)^{1/3}$$

$$y_c = \left(\frac{4^2}{9.81} \right)^{1/3}$$

$$y_c = 1.177 \text{ m}$$

2) The minimum specific energy

$$Q = AV \quad \text{--- (1)}$$

and

$$Q = Q_b \quad \text{--- (2)}$$

Equating eq (1) and (2)

$$Q = Q$$

$$AV = Q_b$$

$$\text{by } Vy = Q$$

$$V = Q/y_c$$

Assignment #2 Page - 5

$$V = \frac{4}{1.177}$$

$$V = 3.398 \text{ m/sec}$$

$$E_{\min} = y + \frac{V^2}{2g}$$
$$1.177 + \frac{13.3981^2}{2(9.81)}$$

$$E_{\min} = 1.76$$

③ The Alternate depth when $E = 4\text{m}$

As $E > E_c$, there are two possible depth for a given specific energy.

$$E = h + \frac{V^2}{2g} \quad \text{where}$$

$$V = \frac{Q}{A} = \frac{q}{h} \quad (\text{for rectangular channel})$$

$$E = h + \frac{q^2}{2gh^2}$$

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Substituting value in meter-second

$$4 - h + 0.8155/h^2$$

For the sub-critical solution the first term associated with potential energy dominates so rearrange as

$$h = 4 - \frac{0.8155}{h^2}$$

Iteration from $h = 4$ give
 $h = 3.948 \text{ m}$

From the sub critical solution the second term associated with K.E. dominates so arrange as

$$h = \sqrt{\frac{0.8155}{4-h}}$$

Iteration from $h = 0$ given
 $h = 0.4814 \text{ m}$

Answer: Alternate depth are
3.95 and 0.481 m

Problem # 1

Water flow at the depth of 10 cm with velocity of 6 m/s in rectangular channel. Is flow critical or subcritical?

What is its alternate depth?

Given Data

Depth = 10 cm

Velocity = 6 m/s

Required

- ⇒ Type of flow
- ⇒ Alternate depth

Solve

First we have check Froude Number

$$Fr = \frac{V}{\sqrt{gy}} = \frac{6 \text{ m/s}}{\sqrt{9.81 \times 0.1}}$$

$$= 6.06 > 1$$

So flow is super critical.

Q # 10 (part 8)

Alternate Depth

as we know

$$E = y + \frac{V^2}{2g}$$

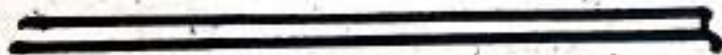
$$= 0.1 + \frac{6^2}{2 \times 9.81}$$

$$= 1.935 \text{ m}$$

The alternate depth for

$$E = 1.935 \text{ m gives}$$

$$y_{alt} = 1.93 \text{ m}$$



Problem - 2

Water flow with velocity of 4.2 m/s and depth of 3m. ... 100 sec?

Given Data

$$\begin{aligned} \text{Depth } y_1 &= 3 \text{ m} \\ U_1 &= 4.2 \text{ m/s} \end{aligned}$$

$$\text{Elevation} = \Delta z = 60 \text{ cm} = 0.6 \text{ m}$$

$$\text{Down step} = 15 \text{ cm} = 0.15 \text{ m}$$

Required

Depth and elevation changes
Max siz of upset.

Sol

$$E_1 = y_1 + \frac{U_1^2}{2g}$$

$$E_1 = 3 + \frac{4.2^2}{2 \times 9.81}$$

$$E_1 = 3.2 \text{ m}$$

Now

$$E_2 = E_1 - \Delta z = 3.20 \text{ m} - 0.60 \text{ m}$$

$$= 2.60 \text{ m}$$

Also

$$E_2 = y_2 + \frac{v^2}{2g}$$

$$= y_2 + \frac{(6 \text{ m}^3/\text{s})^2}{2 \cdot 9.81 \text{ m/s}^2 \cdot y_2^2}$$

$$E_2 = 2.60 \text{ m}$$

So

$$y_2 = 2.24 \text{ m}$$

$$\Delta y = y_1 - y_2 = 0.76 \text{ m}$$

So water surface drops 0.76 m

For a downward step of 15 cm we have

$$E_2 = E_1 - \Delta z$$

$$= 3.20 \text{ m} - (-0.15) \text{ m}$$

~~Q. 11. Pump: 11~~

$$E_2 = 3.35 \text{ m} \quad \text{condition: } \textcircled{1}$$

Given $y_2 = 3.17$ and $\Delta y = y_2 - y_1$
 $\Delta y = 0.17 \text{ m}$ ~~is~~ water surface
rises = 0.02 m

The maximum upstep possible
before affecting upstream water
surface level is for

$$y_2 = y_c$$

$$y_c = \sqrt[3]{\frac{Q^2}{g}}$$

$$y_c = \sqrt[3]{\frac{6^3}{9.8}}$$

$$y_c = 1.54 \text{ m}$$



Q: Problem:- A water passing from slice gate ----- 3.9m

Given data

~~Discharge~~

Depth at upstream = $y_1 = 3.6\text{m}$

Depth at downstream = $y_2 = 0.9\text{m}$

width of slice gate $b = 3.9\text{m}$

Required

① Discharge

② Froude number upstream & Down stream.

Sol

As we know

$$E_1 = E_2$$

$$y_1 + \frac{U_1^2}{2g} = y_2 + \frac{U_2^2}{2g} \quad \text{--- (1)}$$

Also

$$Q = A_1 U_1 = A_2 U_2$$

$$b_1 y_1 U_1 = b_2 y_2 U_2$$

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$$\gamma_1 U_1 = \gamma_2 U_2$$

$$U_2 = \frac{\gamma_1}{\gamma_2} \times U_1$$

Put in eq — (1) $U_2 = 4U_1$ — (2)

$$\frac{\gamma_1 + U_1^2}{2g} = \frac{\gamma_2 + U_2^2}{2g}$$

$$3.6 + \frac{U_1^2}{2g} = 0.9 + \frac{(4U_1)^2}{2g}$$

$$3.6 + \frac{U_1^2}{2g} = 0.9 + \frac{16U_1^2}{2g}$$

$$\frac{U_1^2}{2g} - \frac{16U_1^2}{2g} = 0.9 - 3.6$$

$$\frac{U_1^2 - 16U_1^2}{2g} = -2.7$$

$$\frac{-15U_1^2}{2g} = -2.7$$

$$\sqrt{U_1^2} = \frac{\sqrt{2.7 \times 2(9.81)}}{15}$$

$$U_1 = 1.879 \text{ m/sec}$$

Put value of U_1 in eq-②

$$U_2 = 4U_1$$

$$U_2 = 4(1.879)$$

$$U_2 = 7.516 \text{ m/sec}$$

As

$$\begin{aligned} Q &= A_1 U_1 = b y_1 U_1 \\ &= 3.9 \times 3.6 \times 1.879 \\ &= 26.38 \text{ m}^3/\text{sec} \end{aligned}$$

$$\begin{aligned} Q_2 &= A_2 U_2 = b y_2 U_2 \\ &= 3.9 \times 0.9 \times 7.516 \\ Q_2 &= 26.38 \text{ m}^3/\text{se} \end{aligned}$$

$$Q = Q_1 = Q_2 = 26.38 \text{ m}^3/\text{sec}$$

1) Froude Number at upstream side

$$Fr_1 = \frac{V_1}{\sqrt{gH_1}} = \frac{1.879}{\sqrt{9.81 \times 3.6}} = 0.31 \text{ (sub critical flow)}$$

2) Froude Number at down stream side

$$Fr_2 = \frac{V_2}{\sqrt{gH_2}} = \frac{7.516}{\sqrt{9.81 \times 9.8}}$$

$$Fr_2 = 2.52$$

(super critical flow)