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Subject : STEEL Structure

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Question#01:

Design Philosophies;

A general Statement assuming safety in engineering design.

Resistance (of materials & x-section)  $\geq$  Effect of applied loads ----- (1)

In eq (1) It is essential that both sides are evaluated for same conditions e.g: If effect of load is to produce compressive stress on soil then it should be compared with bearing capacity of soil

Allowable Stress Design (ASD)

- Safety in the design is obtained by specifying, that the effect of the loads should produce stresses that is a fraction of the yield stress  $f_y$ , say one half

- This is equivalent to

$$\begin{aligned}
 FOS &= \text{Resistance, } R / \text{Effect of load, } Q \\
 &= f_y / 0.5 f_y \\
 &= 2
 \end{aligned}$$

Mathematical description of ASD

$$\frac{\phi R_n}{\gamma} \geq \sum Q_i$$

$R_n$  = Resistance or Strength of the component being design

$\phi$  = Resistance Factor or Strength Reduction Factor

$\gamma$  = over load or load factors

$\frac{\gamma}{\phi}$  = Factor of Safety Fos

$Q_i$  = Effect of applied loads

## LRFD;

★ To overcome the deficiencies of ASD, the LRFD method is based on;

Strength of Materials

- 1) It consider the variability not only in resistance but also in effects of loads
- 2) It provides measure of safety related to probability of failure

→ Safety in the design is obtained by specifying that the reduced Nominal Strength of a designed structure is less than the effect of factored loads acting on the structure

$$\phi R_n \geq n \sum \gamma Q_i$$

where;

$R_n$  = Resistance or Strength of component being designed

$Q_i$  = Effect of applied loads

$n$  = Takes into account ductility, redundancy and operational important

$\phi$  = Resistance factor or Strength Reduction factor

$\gamma$  = overload or load factors

$\frac{\gamma}{\phi}$  = Factor of safety

### Advantages of Using Allowable Stress Design:

Following are some advantages of Allowable Stress design method

- 1- Elastic analysis for loads become compatible for design
- 2- old famous book are according to this method
- 3- Experienced engineers are used to this method
- 4- In past it was the only method for design purpose
- 5- This method is included in AISC-05 Specification as an alternate method

## Dis-advantages of Using ASD Method:

- 1) Implied in the ASD method is the assumption that the stress in the member is zero before any loads are applied i.e. no residual stresses exist from forming the members
- 2) ASD does not give reasonable measure of strength, which is more fundamental measure of resistance than is allowable stress
- 3) Another drawback in ASD is that safety is applied only stress level. loads are considered to be deterministic (without variation)

## Advantages of LRFD:

- 1) LRFD accounts for both variability in resistance and load
- 2) It achieves fairly uniform levels of safety for different limit states

## Dis-Advantages of LRFD:

- 1) Its disadvantages is change in design philosophy from previous method

## Q#02:-

### Types of Connections;

#### Slip critical connections;

The force by friction connections transmits surfaces by the clamping action of the bolts produced b/w the faying surfaces.

→ Slip-critical connections are recommended for joints subjected to stress reversal, severe stress fluctuation, impact, vibration or where slip is objectionable.

→ Slip critical connection become bearing type connection after the slip occurs so every slip critical connection is essentially a bearing type connection also.

#### Bearing type connections:

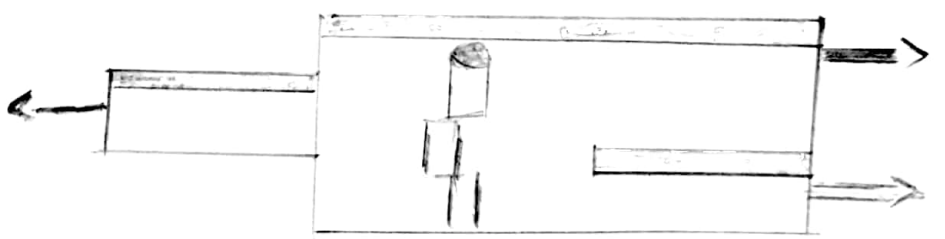
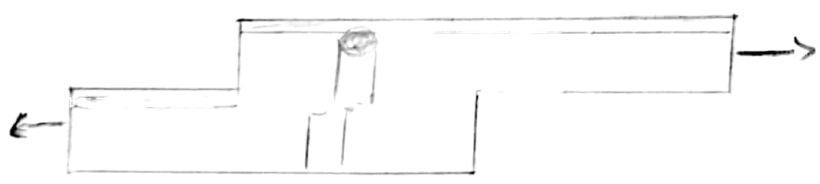
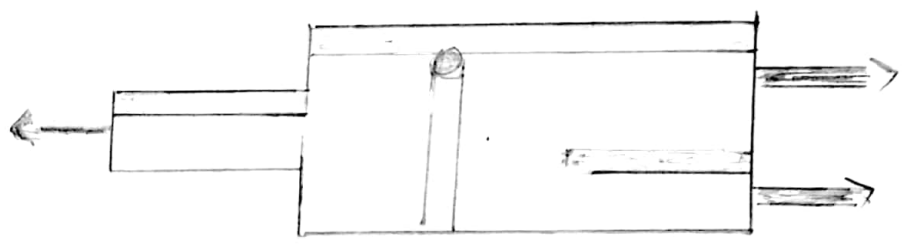
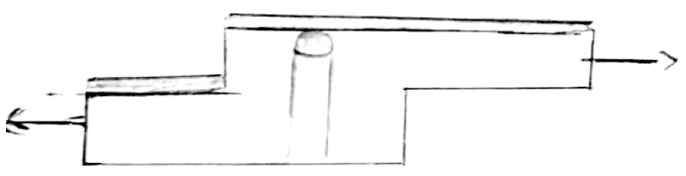
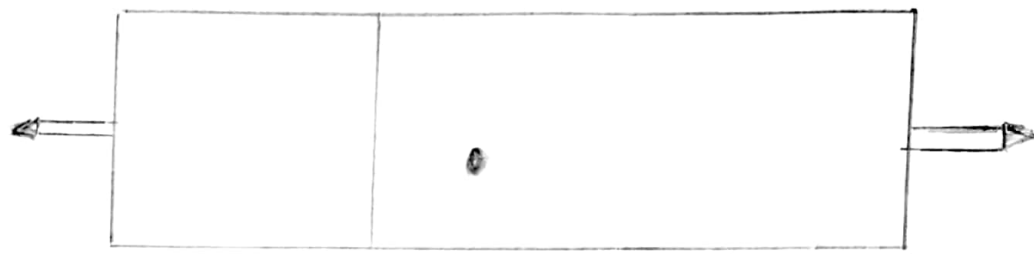
Load is transferred by shearing and bearing on the bolt.

② Capacity in shear depends on whether shear plane intersects the body of bolt or threaded portion.

③ Bearing type connection is most widely used general type connection in which the load is resisted by the bolt body without any friction b/w faying surfaces.

## Shear failure of Bolts:

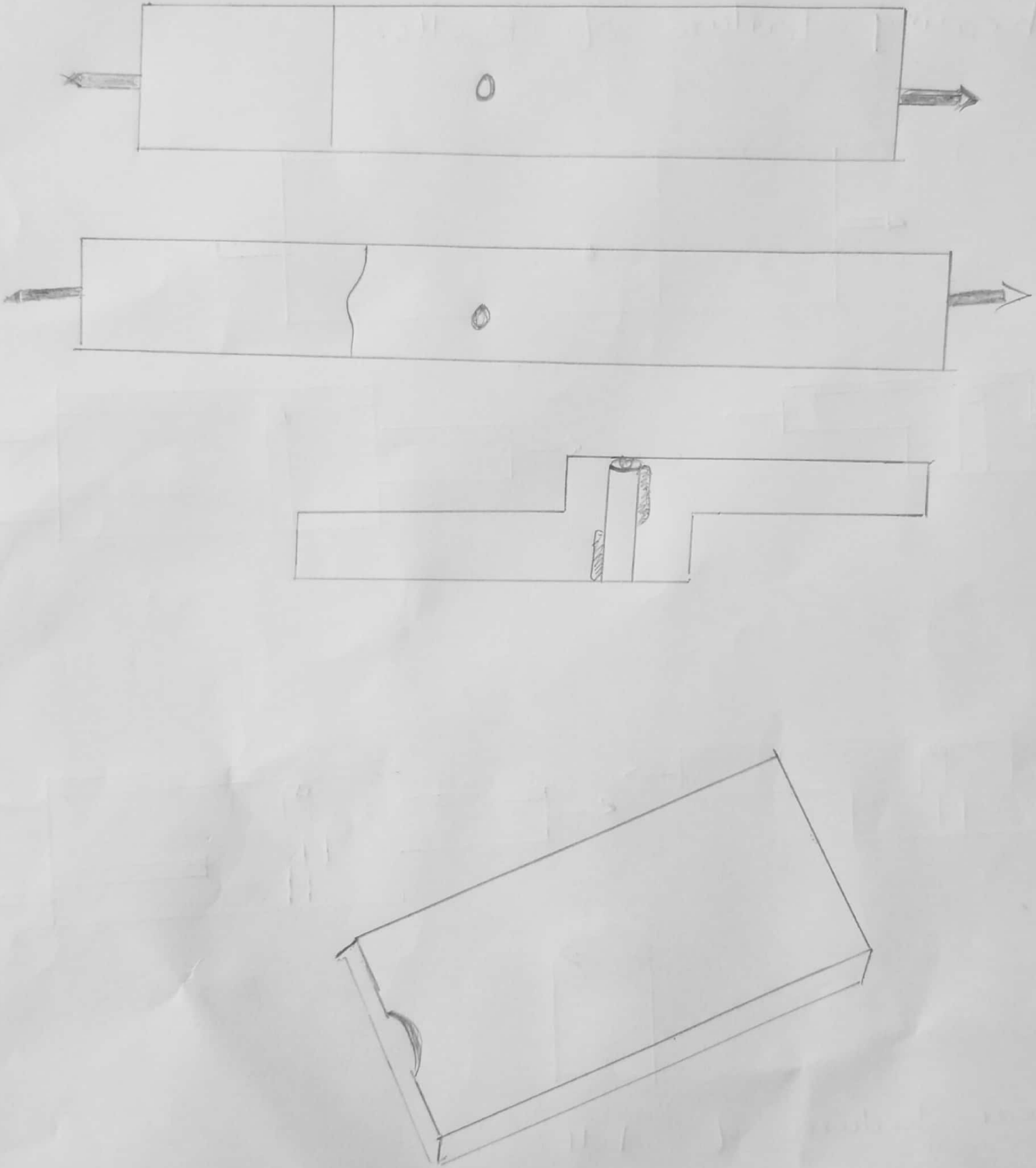
The shear stress in bolt may exceed the working shear stress in the bolt. Shear stresses are generated because the plates slip due to applied forces.





## Bearing failure of plates:

The plate may be crushed when the bearing stress in the plate exceeds the working bearing stress

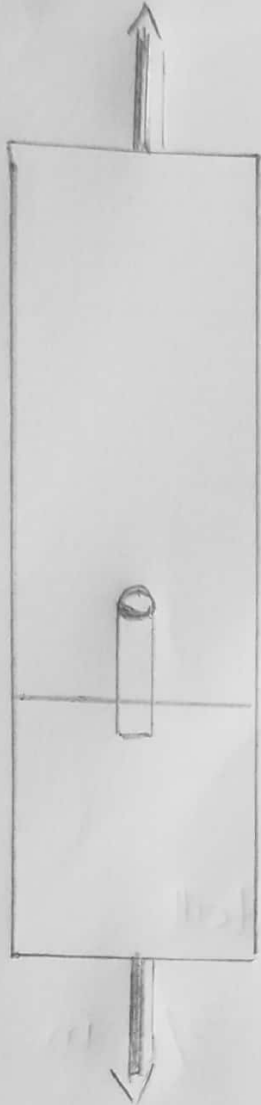


④

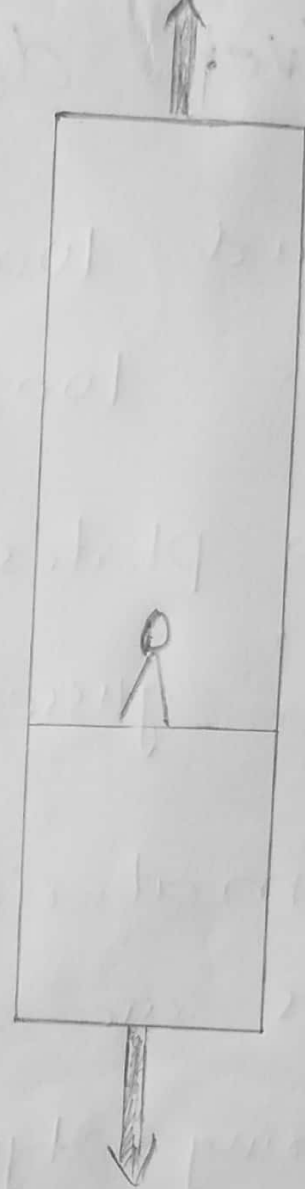
## Tension or Tearing failure of plates:

The tensile stress in the plate at the net cross-section may exceed the working tensile stress.

Tearing failure occurs when bolts are stronger than the plates.



Shearing Failure  
edge of plate



Transverse  
Tension failure.

## Question #03:

### Given Data:

Dead load = 130K

Live load = 265K

Two plates C10x30

1" gusset plate

All material is A36 Steel.

Bolts are A325 with  $\frac{3}{4}$  in dia

Beaming type connection

Threads excluded from Shear Plane

Use three lines of bolts.

ASD Method

## Required data:

No: of Bolts required = ?

Appropriate layout

Solution:

$$\begin{aligned}\text{Design force} &= D.L + L.L \\ &= 130 + 265 \\ &= 395 \text{ k}\end{aligned}$$

⇒ Bolt Design:

For  $3/4"$  dia bolts

$$\text{Area} = \pi/4 (D^2) = \pi/4 (3/4)^2$$

$$\boxed{\text{Area} = 0.4418 \text{ in}^2}$$

Shear Design:

Shear Strength of bolts when threads are excluded from Shear plane, from table

$$F_v = 30 \text{ ksi}$$

$$R_v = \text{Area} \times F_v$$

$$= 0.4418 \times 30$$

$$= 13.25 \text{ k per Shear Surface}$$

As there are two Shear Surface per bolts

$$\Rightarrow \text{No: of bolts} = \frac{\text{Design Force}}{2 \times R_v} = \frac{395}{2 \times 13.25}$$

$$= 14.90$$

So 15 bolts

Bearing:-

(5)

Bearing Strength,  $F_p = 1.2 F_u$

$$F_u = 58$$

$$F_p = 1.2 \times 58$$

$$F_p = 69.6 \text{ ksi}$$

⇒ For Channel

$$R_p = d \cdot t \cdot F_p$$

$$t_w = 0.673$$

$$R_p = 3/4 \times 0.673 \times 69.6$$

$$\boxed{R_p = 35.13 \text{ K}}$$
 for single bearing surface

As there are 15 bolts so 30 surfaces

Capacity:

$$30 \times 35.13$$

$$= 1053.9 \text{ K} > 395 \text{ K} \quad [\text{OK}]$$

⇒ For Gusset plate

$$R_p = d \cdot t \cdot F_p$$

$$= 3/4 \times 1 \times 69.6$$

$$\boxed{R_p = 52.2}$$

Capacity:

$$15 \times 52.2 = 783 \text{ K} > 395 \text{ K}$$

[okay]

Spacing:- For  $\frac{3}{4}$ " dia bolts min. edge distance from table 2.8 =  $1 \frac{1}{4}$ "

$$\text{Also end distance} = 1 \frac{1}{2} d \\ = 1 \frac{1}{2} \left( \frac{3}{4} \right)$$

So  $= 1.13 \text{ in} < 1.25"$

Edge distance,  $L_e = 1 \frac{1}{4}"$  or  $1.25"$

→ Centre to Centre distance

$$L = 3d \\ = 3 \left( \frac{3}{4} \right)$$

$$L = 2.25"$$

Channel

$$L_e = \frac{2P}{F_u t}$$

$$1.25 = \frac{2 \times P}{58 \times 0.673}$$

$$P = 24.4 \text{ k}$$

$$L = \frac{2P}{F_u t} + \frac{d}{2}$$

$$2 = \frac{2P}{58 \times 0.673} + \frac{3/4}{2}$$

$$P = 31.7 \text{ k}$$



6

As the bolts are arranged in three rows and five bolts per row

Capacity:

$$2(3 \times 24.4 + 12 \times 31.7)$$

$$= 907.2 \text{ k} > 395 \text{ k} \quad [\text{OK}]$$

Gusset Plate:

$$L = \frac{2P}{F_u t}$$

$$1.25 = \frac{2P}{58 \times 1}$$

$$P = 36.25 \text{ k}$$

$$L = \frac{2P}{F_u t} + d/2$$

$$2 = \frac{2P}{58 \times 1} + \frac{3/4}{2}$$

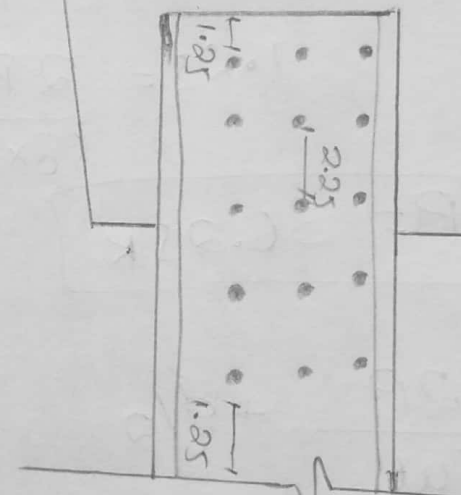
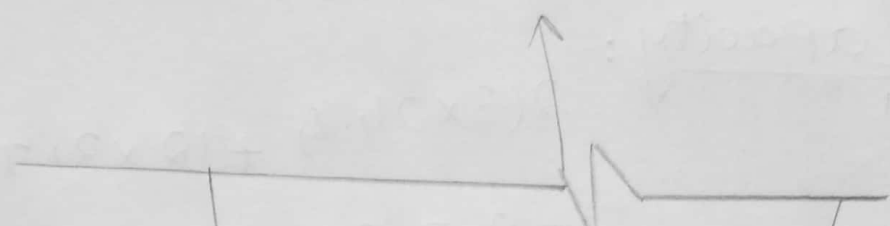
$$P = 47.134$$

Capacity:

$$3 \times 36.25 + 12 \times 47.134$$

$$674.358 \text{ k} > 395 \text{ k} \quad [\text{OK}]$$

395K



395K