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I.D

7666

SUBJECT

FLUID MECHANICS  
II

SUBMITTED  
TO

ENG-ABDUL  
WAHEED

FINAL PAPER



Que :- 1 a.

Define drag

layer?

Forced on Immersed bodies:-

A body which is wholly immersed in a homogenous fluid may be subjected to 2 kind of forces arising from relative motion b/w body and fluid. These forces are termed as drag and lift. Depending on forces either parallel or rigid angle to motion.

Drag force on submerged body can have 2 components.

\* Pressure drag:-

It is equal to the integration of component in direction of motion of all pressure force exerted on surface of body.

$$F_p = C_{p.f} \cdot \frac{v^2}{2} \cdot A$$

where 'cp' depend on shape.



\* Friction Drag:

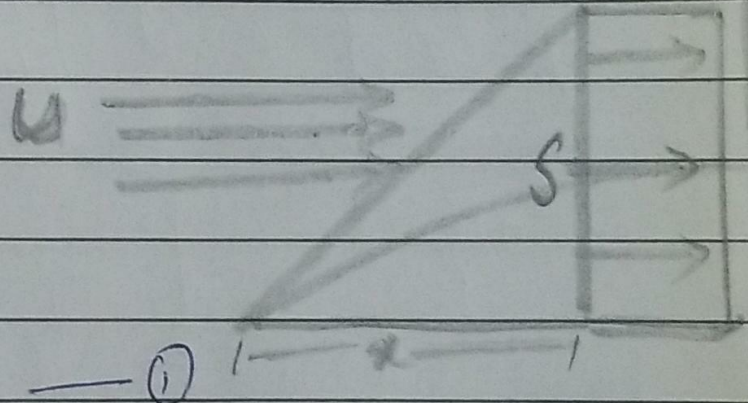
It is equal to integration of component at all shear stress of ~~only~~ along the surface in direction of motion.

$$F_p = C_t \cdot \int \frac{v^2}{2} (BL) \quad \text{"Ct" depends on velocity}$$

→ Friction drag of boundary layer:

$$\bar{\tau}_0 = \mu \frac{u \frac{df(n)}{dn}}{\delta \frac{dn}{dn}}$$

$$\bar{\tau}_0 = \frac{\mu UB}{\delta} \quad \text{--- (1)}$$



As we know that  $\tau_0 = \int U^2 \times \frac{ds}{dx}$   
compare both

$$\int U^2 \times \frac{ds}{dx} = \frac{\mu UB}{\delta}$$

$$\delta ds = \frac{\mu B}{U} dx$$

$$\therefore C = 0$$



Pg (3)

Integration on both sides

$$\frac{f^2}{2} = \frac{uB}{\rho u \alpha} dx + C$$

$$\therefore C = 0$$

$$f = \sqrt{\frac{2B}{d}} \sqrt{\frac{u x}{\rho u}}$$

$$B = 1.69, \quad d = 0.135$$

$$f = \frac{491}{\sqrt{R_n}} x$$

where " $R_n$ " is local Reynold number  
As we have

$$F_x = \int B U^2 S dx$$

where  $d$  is a function at boundary  
layer velocity distribution

Now to find shear stress

$$\tau = \frac{f x}{A} = \frac{d f x}{B dx} = \frac{d f x}{B dx}$$



Pg (4)

$$Z_0 = \int \frac{\rho U^2 ds}{\rho dx} = \int \frac{\rho U^2 ds}{dx}$$

$$Z_0 = \int \frac{\rho U^2 ds}{dx} \longrightarrow \text{general equation}$$

Laminar boundary layer:

$$\frac{u}{U} = f\left(\frac{y}{s}\right) \text{ --- eq (1)}$$

$$\frac{y}{s} = \eta \Rightarrow y = s\eta$$

$$dy = s d\eta \longrightarrow \text{eq (2)}$$

$$\frac{u}{U} = f(\eta)$$

$$du = u df(\eta) \longrightarrow \text{eq (3)}$$

for Laminar flow

$$Z_0 = \mu \frac{du}{dy} \text{ --- eq (4)}$$



Pg (5)

$$Z_0 = \frac{-u \cdot u \, df(n)}{\int d(n)}$$

$$Z_0 = \frac{u u B}{\int} \rightarrow \text{eq (5)}$$

As we have  $Z_0 = \int v^2 \alpha \frac{ds}{dx}$

Comparing both

$$\int u^2 \alpha \frac{ds}{dx} = \frac{u u B}{\int}$$

$$\int ds = \frac{u B}{\int u \alpha} dx$$

$$\frac{\int^2}{2} = \frac{u B}{\int u \alpha} x + C$$

$$\int = \sqrt{\frac{2B}{\int}} \cdot \sqrt{\frac{u x}{\int u}}$$

$$\int = \frac{4.91}{\sqrt{R_n}} \rightarrow \text{eq (6)}$$



Pg 6

Now eq (6) and (5)

$$\tau_0 = 0.332 \frac{\mu U}{x} \sqrt{R_n}$$

Now

$$F_f = \beta \int \tau_0 dx$$

where  $\tau_0 = 0.332 \frac{\mu U}{x} \sqrt{R_n}$

$$R_n = \frac{x u_f}{\mu}$$

then putting the value

$$F_f = 0.664 \sqrt{\rho \mu U^3}$$

As we have

$$F_f = C_f \cdot \rho \cdot \frac{v^2}{2} \cdot BL$$

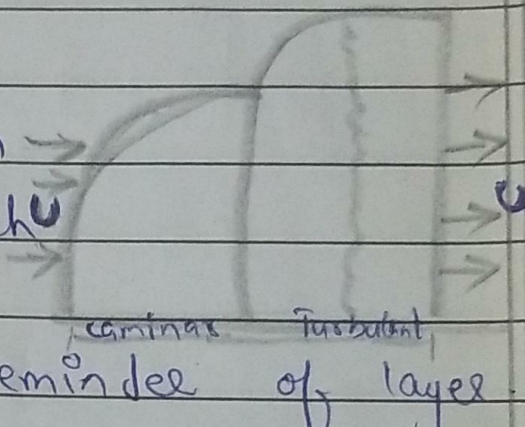
$$C_f = 1.328 \frac{\mu}{\sqrt{\rho U}} = \frac{1.328}{R_n}$$



For Laminar  $R < 500,000$ .

## Turbulent Boundary Layer:-

In this fig show the velocity distribution of boundary layer which is steeper near walls and flatter through out remainder of layer.



The shear stress is greater in turbulent than in laminar. thus;

$$\tau_0 = \frac{f}{8} \rho v^2$$

where  $v$  is the average velocity to obtain relation between average and maximum

As we have

$$v = \frac{1}{1 + 1.33\sqrt{f}} \quad f = 0.028$$



$$\frac{V}{U_{\max}} = \frac{1}{1 + 1.33 \sqrt{0.028}}$$

$$U = 1.235 \text{ V}$$

$$V = \frac{U}{1.235}$$

Now

$$f = \frac{0.316}{(R_h)^{3/4}}$$

$$\tau_0 = f \rho \frac{v^2}{8}$$

$$\tau_0 = \frac{0.316}{\left(\frac{D}{4}\right)} \left(\frac{0}{1.235}\right)^{3/4}$$

$$\tau_0 = \frac{0.023 \rho v^2}{\left(\frac{25}{2}\right)^{3/4}} \rightarrow \text{eq (1)}$$

As we have general equation



Pg 9

$$Z_0 = \int U^2 \propto \frac{ds}{dx} \quad \text{--- (2)}$$

Eq (1) & eq (2)

$$x=0, \quad s=0$$

$$s = \left( \frac{0.0287}{\alpha} \right)^{4/5} \left( \frac{v}{Ux} \right)^{1/5} \cdot x$$

$$d = 0.0979$$

$$s = \frac{0.377}{(R_n)^{1/5}} \cdot x \quad \text{--- eq (3)}$$

$$Z_0 = 0.0587 \int \frac{v^2}{2} \left( \frac{v}{Ux} \right)^{1/5}$$

Now

$$F_f = B \int_0^L Z_0 dx$$

$$F_f = 0.0735 \int \frac{v^2}{2} \left( \frac{v}{Ux} \right)^{1/5} B.L$$



Pg (16)

$$F_f = C_f \cdot \rho \cdot \frac{v^2}{2} \cdot B d$$

$$C_f = \frac{0.0735}{(R)^{1/5}}$$

$$\therefore 500,000 < R < 10^7$$

For

$$R > 10^7$$

$$C_f = \frac{0.455}{(\log R)^{2.52}}$$



Que :- 1 b

Derive equation .....  
..... Channel?

Answer :-

As

Specific energy  
$$E = y + \frac{v^2}{2g}$$

The flow  $Q$  per unit width  $b$   
can be expressed as

$$q = \frac{Q}{b}$$

Now average velocity will be

$$V = \frac{Q}{A} = \frac{qb}{by} = \frac{q}{y}$$

Thus

$$E = y + \frac{v^2}{2g} \Rightarrow y + \frac{1}{2g} \left( \frac{q^2}{y^2} \right)$$

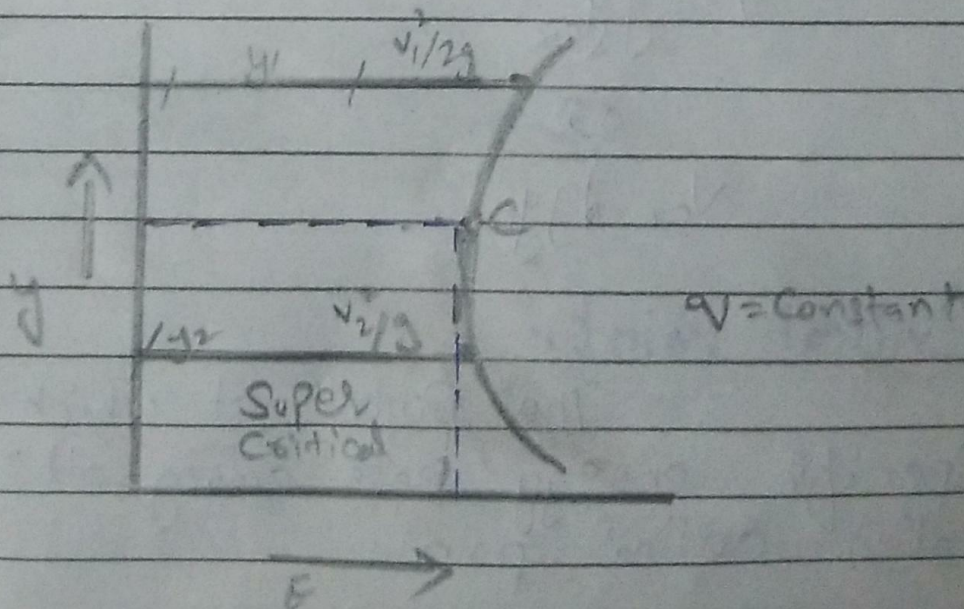
$$(E - y) = \frac{1}{2g} \left( \frac{q^2}{y^2} \right)$$



OR

$$(E - y) y^2 = \frac{q^2}{2g}$$

Thus plot of  $E$  vs  $Y$  will be parabolic. For particular  $q$ , there will be two kind of possible values of  $y$  for given  $E$ . The equation is cubic with three roots with 3rd root being negative point  $C$  represents dividing point between two regime of flow. Thus for given  $q$ , & value of  $E$  is minimum and flow at that point is critical flow. Depth of flow at that point is critical depth  $y_c$  and velocity at that point is critical velocity  $V_c$ .





Thus Thus

$$E = y + \frac{1}{2g} \left( \frac{v^2}{y^2} \right)$$

for minimum specific energy  $\frac{dE}{dy} = 0$

Thus

$$\frac{dE}{dy} = \frac{1 - \frac{v^2}{g y^3}}{2g} = 0$$

$$\Rightarrow \frac{v^2}{g} = 1 \Rightarrow v^2 = g y^3$$

$$\frac{v^2}{g} = y^3$$

$$y_{\text{cr}} = \left( \frac{v^2}{g} \right)^{1/3}$$

Now  $v^2 = g y^3$

$\frac{v}{y}$

$$v = \sqrt{g y} \Rightarrow v^2 y^2 = g y^3$$

or

$$v^2 = g y_{\text{cr}}$$

or

$$v_{\text{cr}} = \sqrt{g y_{\text{cr}}}$$

## Critical Point.

The point at which specific energy least among all is called critical point.



Que - 2:-

Find depth . . . . .  
 . . . . . Critical?

Given data:-

Depth of rectangular channel (d) = ?

Flowsrate (Q) = 3.5 m<sup>3</sup>/sec

Slop of bed = 0.0008

n = 0.0219

Width of Bed = 7666 mm

$$\ll \Rightarrow \frac{7666}{1000} \Rightarrow 7.666$$

Critical depth = ?

Flow sub critical or super critical = ?

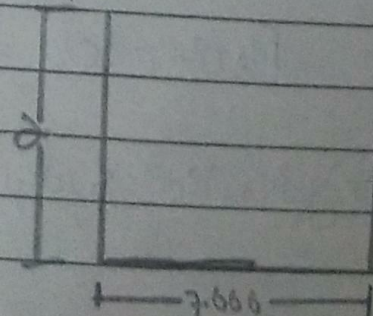
Sol:-

$$\text{Area} = 7.666 \times d$$

$$7.666d$$

$$\text{Perimeter} = d + 7.666 + d$$

$$7.666 + 2d$$





$$\text{Hydraulic Radius } (R_h) = A/P$$

$$\Rightarrow \frac{7.666}{7.666 + 2d}$$

By using Manning Equation

$$Q = \frac{1}{n} A R_h^{2/3} S_0^{1/2}$$

Putting values

$$3.5 = \frac{1}{0.0219} \times 7.666d \times \left( \frac{7.666d}{2d + 7.666} \right)^{2/3} \times (0.0008)^{1/2}$$

$$d = 0.5662$$

$$\text{Area} = 7.666(0.5662)$$

$$\text{Area} = 4.3404 \text{ m}^2$$

$$\text{Perimeter} = 7.666 + 2(0.5662)$$

$$7.666 + 1.1324$$

$$\text{Perimeter} = 8.7984$$

$$\text{Hydraulic radius} = \frac{4.3404}{8.7984}$$

$$\text{Hydraulic radius} = 0.4933 \text{ m}$$



Find critical depth:-

$$y_{cr} = \left( \frac{q_v^2}{g} \right)^{1/3}$$

$$\text{As } q_v = Q/B$$

$$= \frac{3.5}{7.666}$$

$$\Rightarrow 0.456 \text{ m}^2/\text{sec}$$

$$y_{cr} = \left( \frac{(0.456)^2}{9.81} \right)^{1/3}$$

$$y_{cr} = \left( \frac{0.207}{9.81} \right)^{1/3}$$

$$y_{cr} \Rightarrow 0.2763$$

$$\text{As } y > y_{cr}$$

$$0.5662 > 0.2763$$

So flow is sub-critical



Que 3:-Find . . . . .  $0.93 \times 10^{-4} \text{ m}^2/\text{s}$ Given data:-width (B) = 200 mm  $\Rightarrow$  0.2 mlength (L) = 800  $\Rightarrow$  0.8 m

Specific gravity = 0.87

Undisturbed velocity ( $u$ ) = 5 m/secKinematic velocity =  $0.93 \times 10^{-4} \text{ m}^2/\text{sec}$ Req:-

Friction Drag (FD) = ?

Sol:-

Checking whether flow is laminar or not by Reynold's Number.

$$R = \frac{Dv}{\nu}$$

For smooth flat plate

$$D = L, \quad v = U$$

So

$$R = \frac{LU}{\nu}$$



Putting Values

$$= \frac{0.8 \times 5}{0.93 \times 10^{-4}} = 43010$$

43010 < 500,000  $\rightarrow$  Laminar

By using formula

$$F_f = C_f \cdot \rho \cdot \frac{v^2}{2} \cdot BL$$

where

$$C_f = \frac{1.328}{\sqrt{R}} \Rightarrow \frac{1.328}{\sqrt{43010}} \Rightarrow 0.0064$$

$$\rho = \frac{\rho_{\text{soil}}}{\rho_{\text{water}}} = 0.89 = \frac{\rho_{\text{soil}}}{1000}$$

$$\rho_{\text{soil}} = 0.89 \times 1000$$

$$\rho_{\text{soil}} = 890 \text{ kg/m}^3$$

$$F_f = C_f \cdot \rho \cdot \frac{v^2}{2} \cdot BL$$

Putting values

$$= 0.0064 \times 890 \times \frac{(5)^2}{2} \times 0.8 \times 0.8$$

$$F_f = 11.39 \text{ N}$$