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I.D NO

7614

PAPER

FLUID II

SUBMITTED TO

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Q No 1:

a) Define Drag with its components. Write down the equations for friction drag coefficient both in laminar and turbulent boundary layer.

b) Derive equation for critical depth, critical velocity of rectangular section of a channel.

Answer:

### Forced On Immersed Bodies:

A body which is wholly immersed in a homogeneous fluid may be subjected to two kind of forces arising from relative motion b/w body & fluid. These forces are termed as drag & lift, depending on forces either parallel or right angle to motion.

Drag force on submerged body can have two components.

1): PRESSURE DRAG: (Fp)

It is equal to the integration of component in direction of motion of all pressure forces exerted on surface of body.

$$F_p = C_p \cdot \int \cdot \frac{v^2}{2} \cdot A$$

Where "Cp" depends on "shape".

2): Friction Drag:

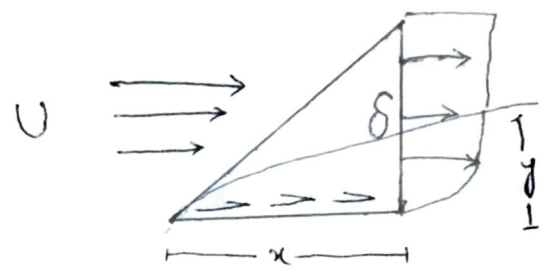
It is equal to the integration of component of all shear stress along the surface in direction of motion.

$$F_f = C_f \cdot \int \cdot \frac{v^2}{2} \cdot BL$$

Where Cf depend on "viscosity".

Friction drag on boundary layer:

$$\tau_0 = \mu u \frac{df(\eta)}{\delta d(\eta)}$$



$$\tau_0 = \frac{\mu U B}{\delta}$$

As we have  $\tau_0 = \int u^2 x \frac{d\delta}{dx}$

Compare both

$$\int u^2 x \frac{d\delta}{dx} = \frac{\mu U B}{\delta}$$

$$\delta d\delta = \frac{\mu B}{\int u^2 x} dx$$

Integration on both sides;

$$\frac{\delta^2}{2} = \frac{\mu B}{\int u^2 x} dx + C$$

$$\delta = \sqrt{\frac{2B}{d}} \cdot \sqrt{\frac{\mu x}{\int u^2}} \quad \because C = 0$$

$$B = 1.63, \quad d = 0.135$$

$$\delta = \frac{4.91}{\sqrt{R_m}} x$$

Where " $R_m$ " is local Reynold number.

As we have;

$$F_x = \int B U^2 \delta dx$$

Where  $\delta$  is a function at boundary layer velocity distribution.

Now to find shear stress:

$$\tau = \frac{F_x}{A} = \frac{dF_x}{B dx} = \frac{dF_x}{B dx}$$

$$\tau_0 = \int B U^2 \delta \frac{d\delta}{B dx} = \int U^2 \alpha \frac{d\delta}{dx}$$

$$\tau_0 = \frac{\int U^2 \alpha d\delta}{dx} \rightarrow \text{general Equation}$$

Laminar boundary Layer:

$$\frac{u}{U} = f\left(\frac{y}{\delta}\right) \quad \text{--- eq ①}$$

$$\frac{y}{\delta} = \eta \quad \Rightarrow \quad y = \delta \eta$$

$$dy = \delta d\eta \longrightarrow \text{eq (2)}$$

$$\frac{u}{U} = f(\eta)$$

$$du = U df(\eta) \longrightarrow \text{eq (3)}$$

For Laminar flow;

$$\tau_0 = \mu \frac{du}{dy} \longrightarrow \text{eq (4)}$$

$$\tau_0 = \mu \cdot \frac{U df(\eta)}{\delta d(\eta)}$$

$$\tau_0 = \frac{\mu U^2}{\delta} \longrightarrow \text{eq (5)}$$

As we have  $\tau_0 = \int_0^{\delta} \nu^2 \alpha \frac{d\delta}{dx}$

Comparing both

$$\int_0^{\delta} \nu^2 \alpha \frac{d\delta}{dx} = \frac{\mu U^2}{\delta}$$

$$\delta d\delta = \frac{\mu U^2}{\rho \nu \alpha} dx$$

(6)

$$\frac{\delta^2}{2} = \frac{\mu \beta}{\rho u \alpha} x + C$$

$$\delta = \sqrt{\frac{2\beta}{\alpha}} \cdot \sqrt{\frac{\mu x}{\rho u}}$$

$$\delta = \frac{4.91}{\sqrt{Re_x}} \rightarrow \text{eq (6)}$$

put eq (6) in eq (5)

$$\tau_0 = 0.332 \frac{\mu u}{x} \sqrt{Re_x}$$

Now,

$$F_f = B \int \tau_0 dx$$

where  $\tau_0 = 0.332 \frac{\mu u}{x} \sqrt{Re_x}$

$$Re_x = \frac{x u \rho}{\mu}$$

then putting the values;

$$F_f = 0.664 \sqrt{\int \mu L U^3}$$

As we have;

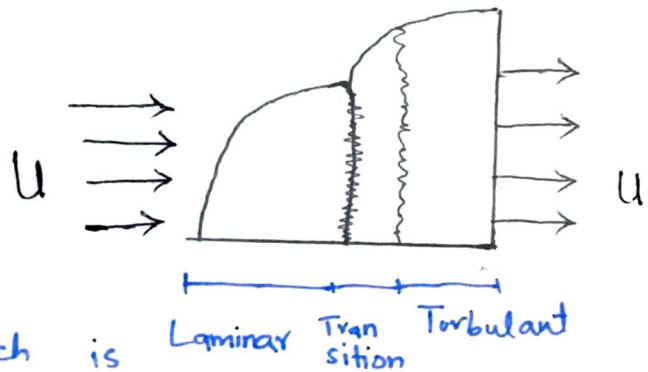
$$F_f = C_f \cdot \int \cdot \frac{v^2}{2} \rho L$$

$$C_f = 1.328 \sqrt{\frac{\mu}{\rho L U}} = \frac{1.328}{R_h}$$

For Laminar  $R < 500,000$ .

### Turbulent Boundary Layer:

In this fig: show the velocity distribution of boundary layer which is



steeper near walls and flatter through and remainder of layer.

The shear stress is greater in turbulent than in laminar thus;

$$\tau_0 = \int f \frac{v^2}{8}$$

Where "v" is the average velocity to

obtain relative b/w average & maximum.



As we have;

$$\frac{V}{U_{\max}} = \frac{1}{1 + 1.33\sqrt{f}} \quad \therefore f = 0.028$$

$$\frac{V}{U_{\max}} = \frac{1}{1 + 1.33\sqrt{0.028}}$$

$$U = 1.235V$$

$$V = \frac{U}{1.235}$$

Now;

$$f = \frac{0.316}{(Rh)^{3/4}}$$

$$\tau_0 = f \int \frac{V^2}{8}$$

$$\tau_0 = \frac{0.316}{\frac{D}{\nu} \left( \frac{0}{1.235} \right)^{1/4}}$$

$$\tau_0 = \frac{0.023 \int V^2}{\left( \frac{2f}{\nu} \right)^{1/4}} \rightarrow \text{eq (1)}$$

As we have general equation

$$\tau_0 = \int V^2 \propto \frac{ds}{dx} \rightarrow \text{eq (2)}$$

Eq ① & eq ②

$$x=0, \quad \delta=0$$

$$\delta = \left( \frac{0.0287}{\alpha} \right)^{4/5} \left( \frac{\nu}{u_{\infty}} \right)^{1/5} x$$

$$d = 0.0972$$

$$S = \frac{0.377}{(R)^{1/5}} \cdot x \rightarrow \text{eq ③}$$

$$\tau_0 = 0.0587 \int \frac{\nu^2}{2} \left( \frac{\nu}{u_{\infty}} \right)^{1/5}$$

Now;

$$F_f = B \int_0^L \tau_0 dx$$

$$\bar{F}_f = 0.0735 \int \frac{\nu^2}{2} \left( \frac{\nu}{u_{\infty}} \right)^{1/5} \cdot BL$$

$$\bar{F}_f = C_f \cdot \int \cdot \frac{\nu^2}{2} \cdot BL$$

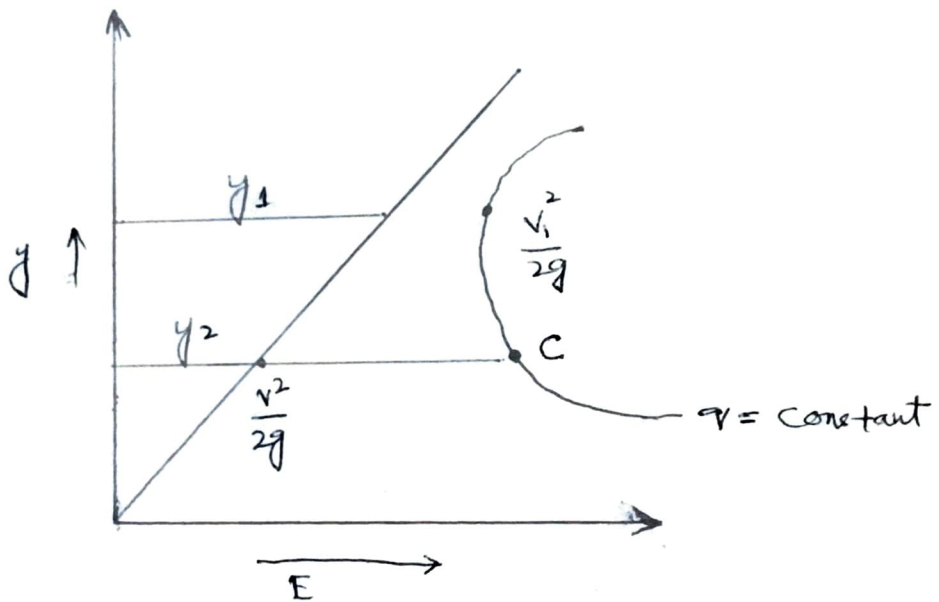
$$C_f = \frac{0.0735}{(R)^{1/5}} \quad \therefore (5,00,000 < R < 10^7)$$

For  $R > 10^7$

$$C_f = \frac{0.455}{(\log R)^{2.52}}$$



PART NO (B) :-



This is Specific Energy Equation :-

For particular  $q$ , there

will be two kind of possible values

of  $y$  for given  $E$ . The equation is

Cubic with three roots with third being negative giving no values. Thus two alternatives depth represents two totally different flow regimes; slow & deep. And an upper portion fast & shallow on lower portion.

Point represent dividing point b/w two regimes of flow. Thus for given "q" values of E is minimum & flow at this point is critical flow depth of flow at this point is critical depth & velocity at this point is critical velocity.

Thus relation of critical depth can be found as;

$$E = y + \frac{1}{2g} \left( \frac{q^2}{y^2} \right)$$

For minimum Specific energy

$$\frac{dE}{dy} = 0$$

$$\frac{dE}{dy} = 1 - \frac{2}{2g} \left( \frac{q^2}{y^3} \right)$$

$$\frac{dE}{dy} = 1 - \frac{q^2}{gy^3}$$

$$1 = \frac{q^2}{gy^3} \Rightarrow q^2 = gy^3$$

$$y_{cr} = \left( \frac{q^2}{g} \right)^{1/3}$$

As  $q = vy$  ,  $vc^2 = gy^3$

or  $vc = \sqrt{gy_c}$

$$y_c = \frac{vc^2}{g}$$

Now;  $\frac{y_c}{2} = \frac{vc^2}{2g}$

$$E_{min} = y_c + \frac{vc^2}{2g} \Rightarrow y_c + \frac{y_c}{2}$$

$$= \frac{3}{2} y_c \quad \text{OR} \quad y_{cr} = \frac{2}{3} \text{ constant.}$$

	Sub-critical	critical	super critical
Depth of flow	$y > y_c$	$y = y_c$	$y < y_c$
velocity slope	$v < v_c$ mild slope $S_0 < S_{c0}$	$v = v_c$ critical slope	$v > v_c$

Q No 2:

Find depth of the rectangular channel

if water flows at the rate of  $3.5 \text{ m}^3/\text{s}$ with bed slope of  $0.0008$  and  $n = 0.0219$ .

Width of bed is your student ID number

in mm. Also find the critical depth, critical

velocity? Is flow sub-critical or super critical?

GIVEN DATA:

$$\text{Flow rate} = Q = 3.5 \text{ m}^3/\text{sec}$$

$$\text{Slope of bed} = S_0 = 0.0008$$

$$n = 0.0219$$

$$\begin{aligned} \text{Width of bed} \\ \text{depth} &= 7614 \text{ mm} \\ &= 7.614 \end{aligned}$$

REQUIRED:

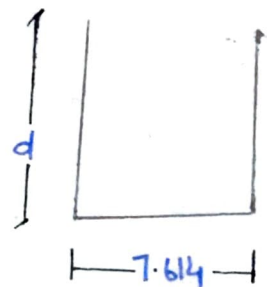
1): To find depth of rectangular channel.

2): Is flow sub-critical or super critical = ?

SOLUTION:

As we know that

$$\begin{aligned} \text{Area} &= 7.614 \times d \\ &= 7.614d \end{aligned}$$



$$\begin{aligned} \text{Perimeter} &= d + 7.614 + d \\ &= 7.614 + 2d \end{aligned}$$

$$\begin{aligned} \text{Hydraulic Radius} = R_h &= A/p \\ &= \frac{7.614d}{7.614 + 2d} \end{aligned}$$

By using Manning Equation

$$Q = \frac{1}{n} A R_h^{2/3} S_0^{1/2}$$

putting the values

$$3.5 = \frac{1}{0.0219} \times 7.614d \times \left( \frac{7.614d}{2d + 7.614} \right)^{2/3} \times (0.008)^{1/2}$$

$$d = 0.5688 \text{ m}$$

$$\text{AREA} = 7.614 (0.5688)$$

$$\text{Area} = 4.3308 \text{ m}^2$$

$$\text{PERIMETER} = 7.614 + 2(\cancel{0.5688}(4.3308))$$

$$\text{PERIMETER} = 7.614 + 2(0.5688)$$

$$= 7.614 + 1.1376$$

$$\text{Perimeter} = 8.7516$$

$$\text{Hydraulic Radius} = R_h = \frac{4.3308}{8.7516}$$

$$\text{Hydraulic Radius} = 0.494 \text{ m}$$



Q No 3:

Find the friction drag on one side of a smooth plate with 200 mm wide and 800 mm length placed longitudinally in stream of oil with specific gravity of 0.89. The undisturbed velocity is 5 m/s and kinematic viscosity is  $0.93 \times 10^{-4} \text{ m}^2/\text{s}$ .

GIVEN DATA:

$$\text{Wide} = d = 200 \text{ mm} = 0.2 \text{ m}$$

$$\text{Length} = L = 800 \text{ mm} = 0.8 \text{ m}$$

$$\text{Specific gravity} = G_s = 0.89$$

$$\text{velocity} = v = 5 \text{ m/sec}$$

REQUIRED:

$$\text{Kinematic viscosity} = \nu_k = 0.93 \times 10^{-4} \text{ m}^2/\text{s}$$

To find friction drag.

SOLUTION:

As we know that

Checking flow is laminar or not.

By Reynold number

$$R = \frac{DV}{\nu}$$

For smooth plate

$$D = L, \quad V = U$$

So;

$$R = \frac{LU}{\nu}$$

putting the values

$$= \frac{0.8 \times 5}{0.93 \times 10^{-4}}$$

$$= 43010$$

$$43010 < 500,000 \longrightarrow \text{Laminar}$$

By using formula:

$$F_f = C_f \cdot J \cdot \frac{V^2}{2} \cdot BL$$

where

$$C_f = \frac{1.328}{\sqrt{R}}$$

$$= \frac{1.328}{\sqrt{43010}} = 0.0064$$

→

$$S = \frac{J_{\text{soil}}}{J_{\text{water}}}$$

$$0.89 = \frac{J_{\text{soil}}}{1000}$$

$$J_{\text{soil}} = 0.89 \times 1000$$

$$J_{\text{soil}} = 890 \text{ Kg/m}^3$$

→

$$F_f = C_f \cdot J \cdot \frac{V^2}{2} \cdot BL$$

putting the values

$$F_f = 0.0064 \times 890 \times \frac{(5)^2}{2} \times 0.2 \times 0.8$$

$$F_f = 11.39 \text{ N}$$

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