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Q construct the mohr's circle diagram & find principle stresses & maximum in plane shear stress for the stress of point C located at the center of uniformly distributed load & 1 inch below the top fiber of beam cross section shown in fig. How ever construct the mohr circle it is necessary to draw shear stress & flexural stress variation diagram for maximum shear force & bending movement respectively. Compare result from mohr circle with stress transformation equation.

Note :- I.O.D is 7278

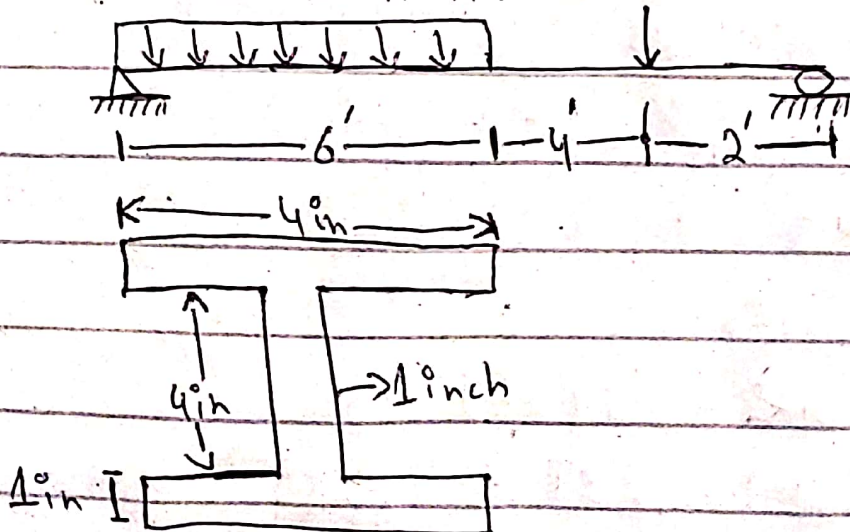
so load is 78 lb

$P = 78 \text{ lb}$

$$0.5 = 0.5 \times 78 = 39 \text{ lb/ft}^2$$

$$2P = 2 \times 78 = 156 \text{ lb}$$

$$0.5 \times P = 39 \text{ lb/ft}^2 \quad 2P = 156 \text{ lb}$$



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Solⁿ → first find Reaction forces R_A & R_B

$$R_A + R_B = 220 \text{ lb}$$

$$\rightarrow \sum M = 0$$

$$-(39 \times 6 \times 3) - (156 \times 10) + 12 R_B = 0$$

$$R_B \times 12 - 858 = 0$$

$$R_B = \frac{858}{12}$$

$$R_B = 71.5$$

now we know that

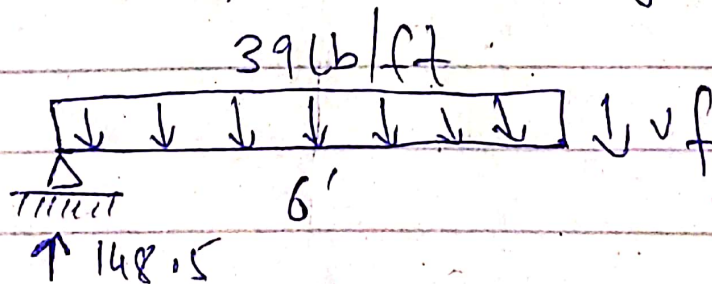
$$R_A + R_B = 220 \text{ lb}$$

$$R_A = 220 - R_B$$

$$R_A = 220 - 71.5$$

$$R_A = 148.5$$

Now shear force at change point
beam



shear force at 6th from left

$$\sum F_y = 0 \uparrow +$$

$$148.5 - (39 \times 6) - v \text{ at } 6' = 0$$

$$148.5 - 234 = v \text{ at } 6'$$

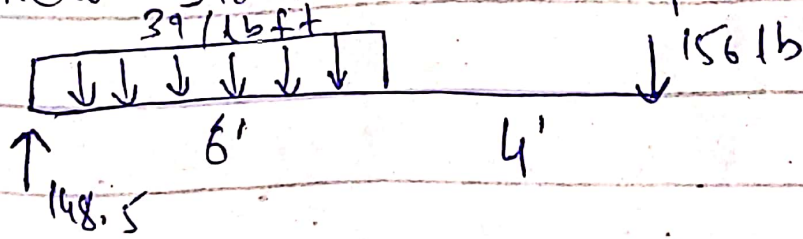
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$$V_{ft} = -85.5$$

Now shear stress at 10 ft

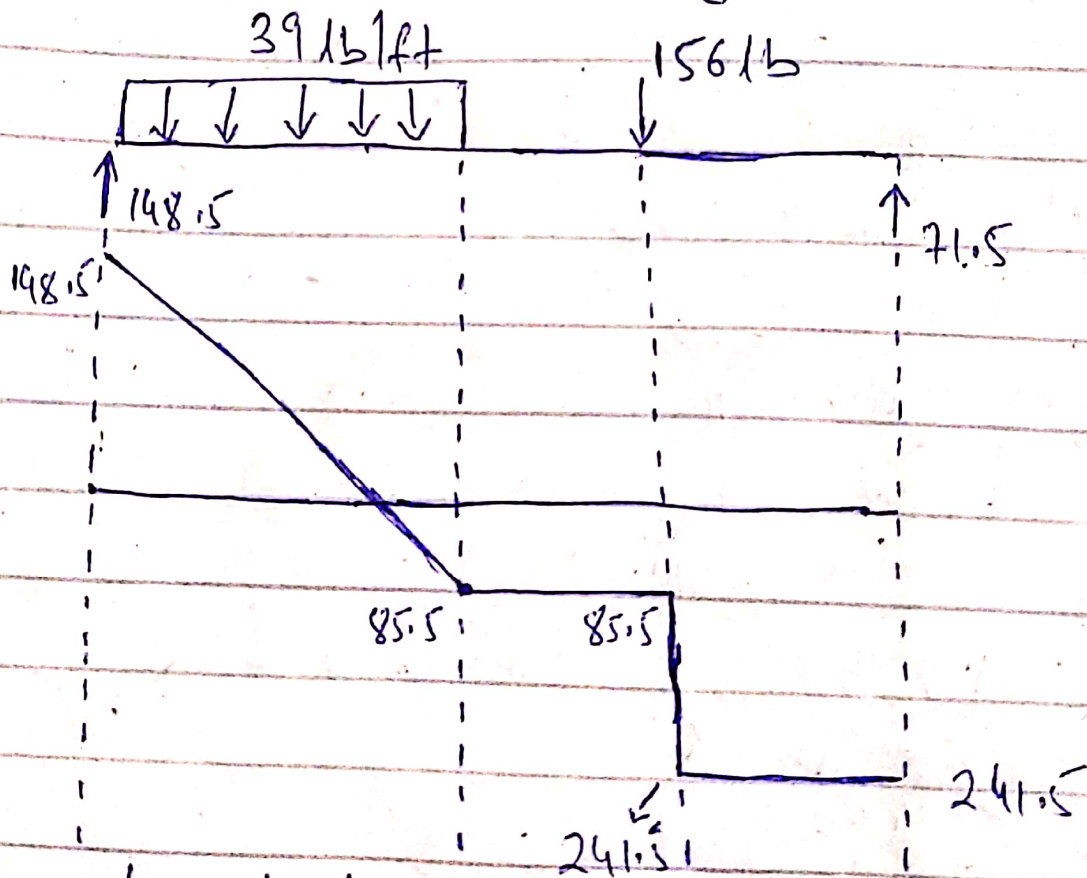


$$\sum F_y = 0 \uparrow$$

$$148.5 - (39 \times 6) - 156 - V_{10'} = 0$$

$$V_{10ft} = \cancel{209} \text{ (error)} 241.5 \text{ (-ve)}$$

Shear force & moment diagram

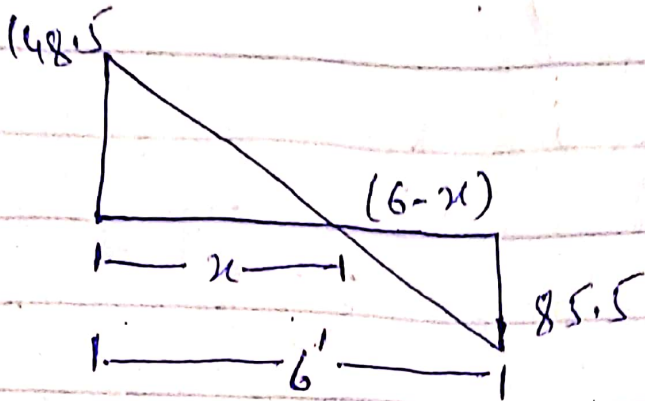


Moment at change point

find zero shear point first

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(4)



by similar triangles

$$\frac{148.5}{x} = \frac{85.5}{(6-x)}$$

$$891 = 148.5x + 85.5x$$

$$x = 891$$

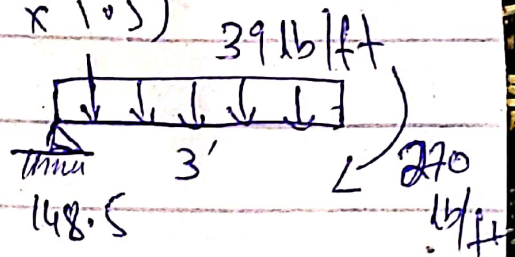
234

$$x = 3.80$$

(i) at center UDL 3ft from left

$$M = -148.5 \times 3 + (39 \times 3 \times 1.5)$$

$$M = 270 \text{ lb-ft}$$

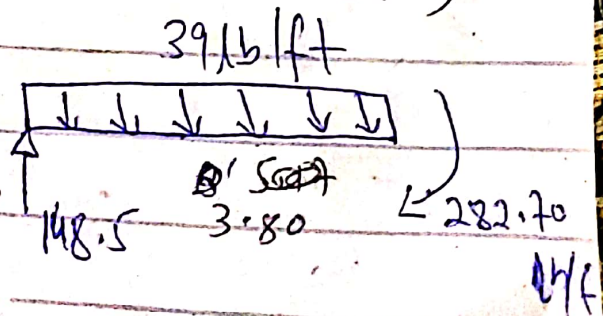


ii. at distance 3.80 ft from left

$$M = (-148.5 \times 3.80) + (39 \times 3.80 \times \frac{3.80}{2})$$

$$M = 282.70$$

$$M = 282.72 \text{ lb-ft}$$



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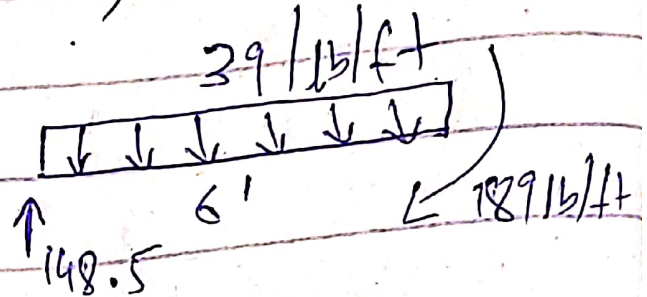
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at a distance of 6' from left

$$\rightarrow M = (-148.5 \times 6) + (39 \times 6) \times 3$$

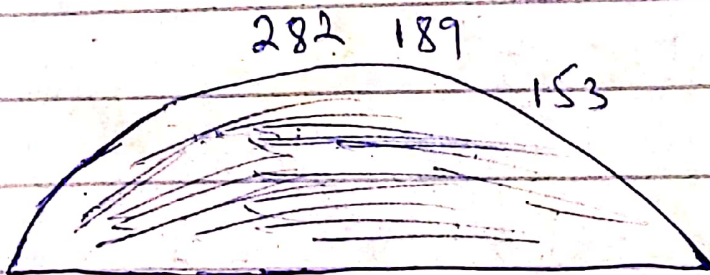
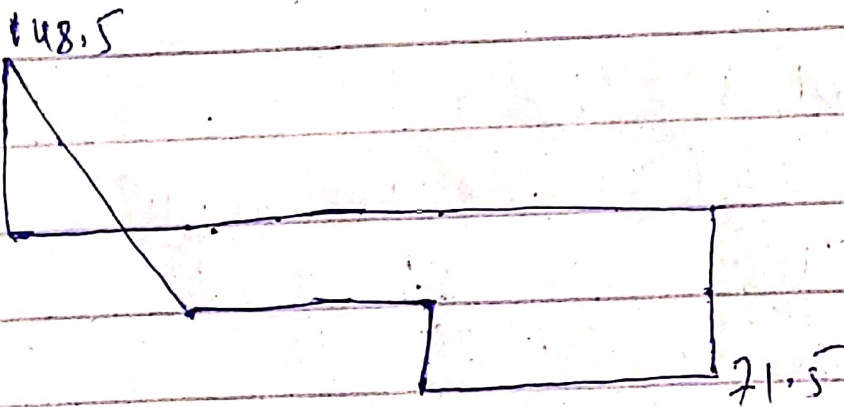
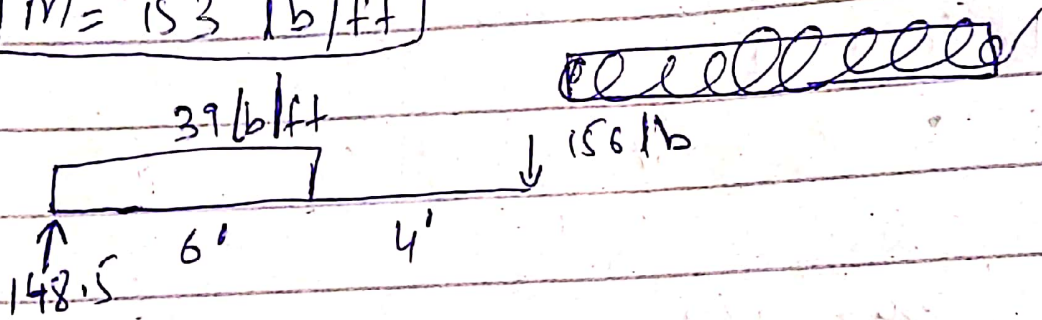
$$M = 189 \text{ lb/ft}$$



at a distance of 10' from left

$$\rightarrow M = (-148.5 \times 10) + (39 \times 6 \times 7)$$

$$M = 153 \text{ lb/ft}$$



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Shear stress :-

As per the question the maximum shear stress $T = \frac{VQ}{It}$ occurs

where the maximum shear force lies in above diagram max-shear force is 31.5 lb.

$$+\uparrow \sum f_y = 148.5 - 39 \times 3 - V = 0$$
$$\boxed{V = 31.5 \text{ lb}}$$

find moment of inertia :-

$I_{xx} = I_{xx_1} + I_{xx_2} + I_{xx_3}$ from given question

$$I_{xx_1} = \frac{1}{12} (4)(1)^3 + 4(2.5)^2 = 25.33$$

$$I_{xx_2} = \frac{1}{12} (4^3)(1) + 4(0)^2 = 5.33$$

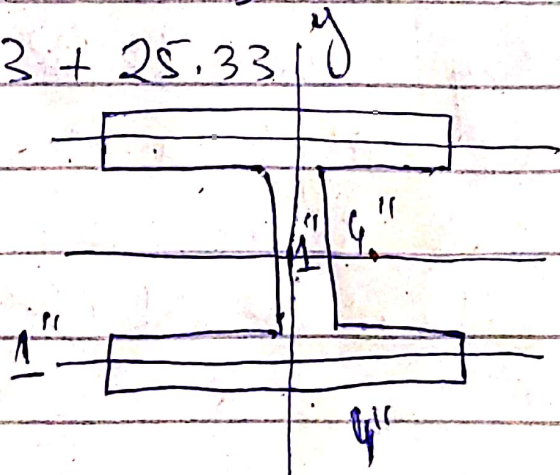
$$I_{xx_3} = \frac{1}{12} (1^3)(4) + (3-5.5)^2 4 = 25.33$$

$$I_{xx_3} = \frac{1}{12} (1^3)(4) + (3-5.5)^2 4 = 25.33$$

$$I_{xx} = I_{xx_1} + I_{xx_2} + I_{xx_3}$$

$$I_{xx} = 25.33 + 5.33 + 25.33$$

$$I_{xx} = 56 \text{ in}^4$$



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$$\begin{aligned} I_{yy} &= I_{yy1} + I_{yy2} + I_{yy3} \\ &= \frac{bh^3}{12} + \frac{bh^3}{12} + \frac{bh^3}{12} \\ &= \frac{4^3 \times 1}{12} + \frac{1^3 \times 4}{12} + \frac{1 \times 4^3}{12} \end{aligned}$$

$$I_{yy} = 11 \text{ in}^4$$

finding shear stress at point C C 1" below from top fibre

$$\tau_{xy} = \tau_{yx} = \frac{VQ}{IB}$$

$$Q = Ay$$

$$A = 1 \times 4$$

$$A = 4 \text{ in}^2$$

$$Q = 1 \times 4 \times 2.5$$

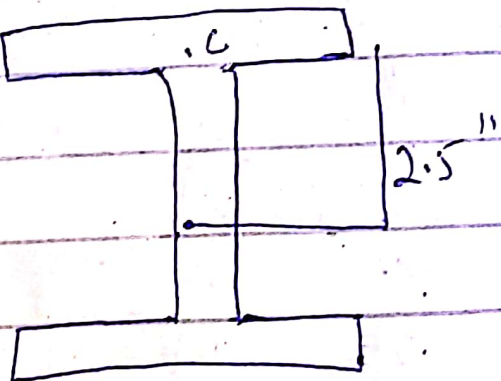
$$Q = 10 \text{ in}^3$$

$$\tau_{xy} = \tau_{yx} = \frac{47.66 \times 10}{56 \times 4}$$

$$\tau_{xy} = 2.12 \text{ lb/in}^2$$

$$\sigma_x = \frac{My}{I}$$

$$= \frac{12 \times 242.61 \times 2}{56}$$



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$$\sigma_x = 103.97 \text{ lb/in}^2$$

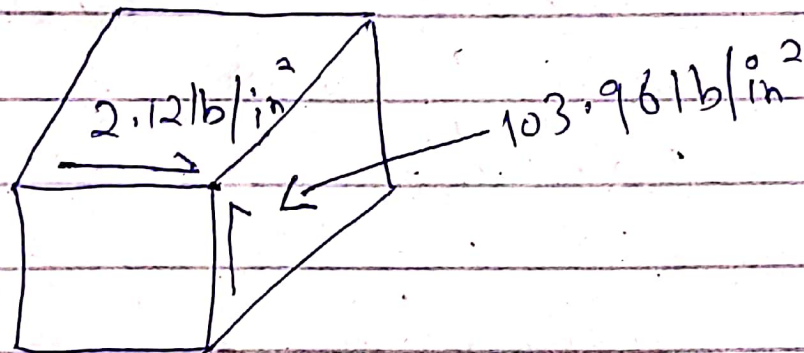
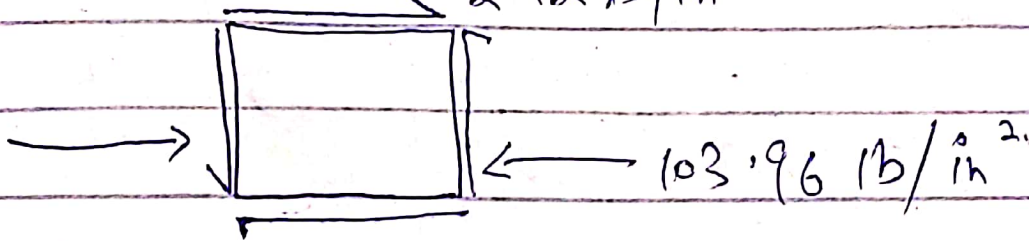
fluxure stress at point C

$$\sigma = 103.97 \text{ lb/in}^2$$

stress stress at point "C"

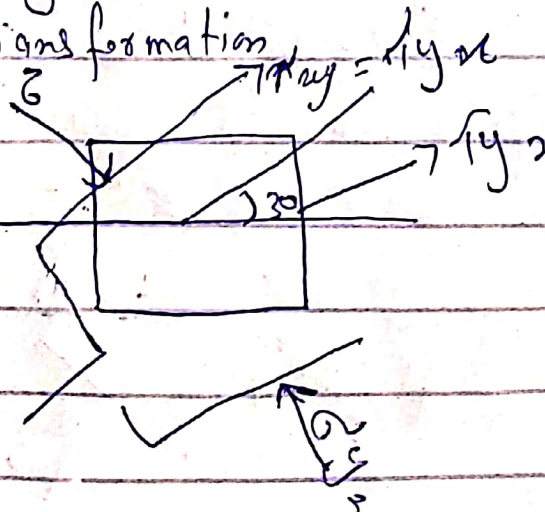
$$\tau = 2.12 \text{ lb/in}^2$$

$$2.12 \text{ lb/in}^2$$



Assume that element rotates at 30° rotation

Assume derive following equation for stress transformation



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$$\sigma_{x'} = \frac{\sigma_x + \sigma_y}{2} + \frac{(\sigma_x - \sigma_y)}{2} (\cos 2\theta) + \tau_{xy} \sin 2\theta$$

$$\sigma_{x'} = \frac{-103.76 + 0}{2} + \frac{(-103.76 - 0)}{2} (\cos 2 \times 30^\circ) + 2.12 \sin 60^\circ$$

$$\sigma_{x'} = -172.07 \text{ lb/in}^2$$

for σ_y

$$\sigma_{y'} = \frac{\sigma_x + \sigma_y}{2} - \frac{(\sigma_x - \sigma_y)}{2} (\cos 2\theta) - \tau_{xy} \sin 2\theta$$

$$\sigma_{y'} = \frac{-103.6 + 0}{2} - \frac{(-103.6 - 0)}{2} (\cos 60^\circ) - 2.12 \sin 60^\circ$$

$$\sigma_{y'} = -121.00 \text{ lb/in}^2$$

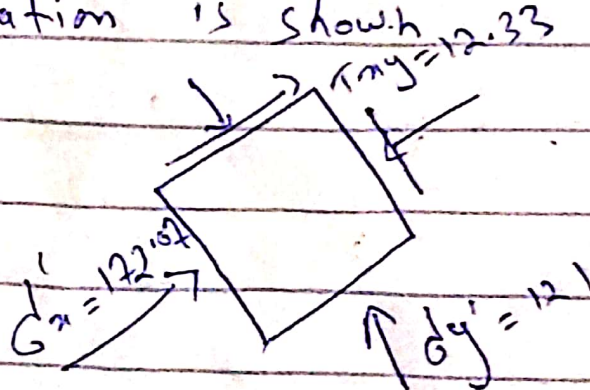
for $\tau_{x'y'}$

$$\tau_{x'y'} = -\frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$$

$$\tau_{x'y'} = \frac{-103.6 - 0}{2} \sin 60^\circ + 2.12 \cos 60^\circ$$

$$\tau_{x'y'} = 12.33 \text{ lb/in}^2$$

Now stress state after 30° clockwise orientation is shown



2. Find principles stress :-

We know that the principles stress equation as

$$\sigma_{1,2} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$\sigma_{1,2} = \frac{-103.67 + 0}{2} \pm \sqrt{\left(\frac{-103.67 - 0}{2}\right)^2 + (2.21)^2}$$

$$\sigma_{1,2} = -51.83 \pm 51.88$$

$$\sigma_y = \sigma_1 = 0.05116 \text{ lb/in}^2$$

$$\sigma_x = \sigma_2 = -51.83 - 51.88 = -103.716 \text{ lb/in}^2$$

or

first find $\theta_p = ?$

$$\tan 2\theta_p = \frac{\tau_{xy}}{(\sigma_x - \sigma_y)/2} = \frac{2.21}{(-103.67)} = -2.13$$

clockwise

put in general equation

$$\sigma'_{x \max} = \frac{-103.67 + 0}{2} + \frac{(-103.67 - 0)}{2} \cos(2) + 2.21 \sin 2(-2.70)$$

$$\boxed{\sigma'_{x \max} = 103.67}$$

Max in plane shear stress in this case

$$\tan 2\theta_s = - \frac{(\sigma_x - \sigma_y)/2}{\tau_{xy}}$$

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$$\tan 2\theta_s = 23.21$$

$$\theta = 175.11 \text{ Anticlockwise.}$$

Put this in the general equation

for τ_{xy}'

$$\tau_{xy}' = \frac{(\sigma_x - \sigma_y) \sin 2\theta + 2\tau_{xy} \cos 2\theta}{2}$$

$$\tau_{xy}' = \frac{(-103.67 - 0) \sin 2(175.11) + 2.21 \cos 2(175)}{2}$$

$$\tau_{xy}' = 6.57 \text{ bin [Max in Plane shear stress]}$$

Mohr's circle

Center coordinate

$$(h, k) = \left[\frac{-103.67 + 0}{2}, 0 \right]$$

$$= [-51.33, 0]$$

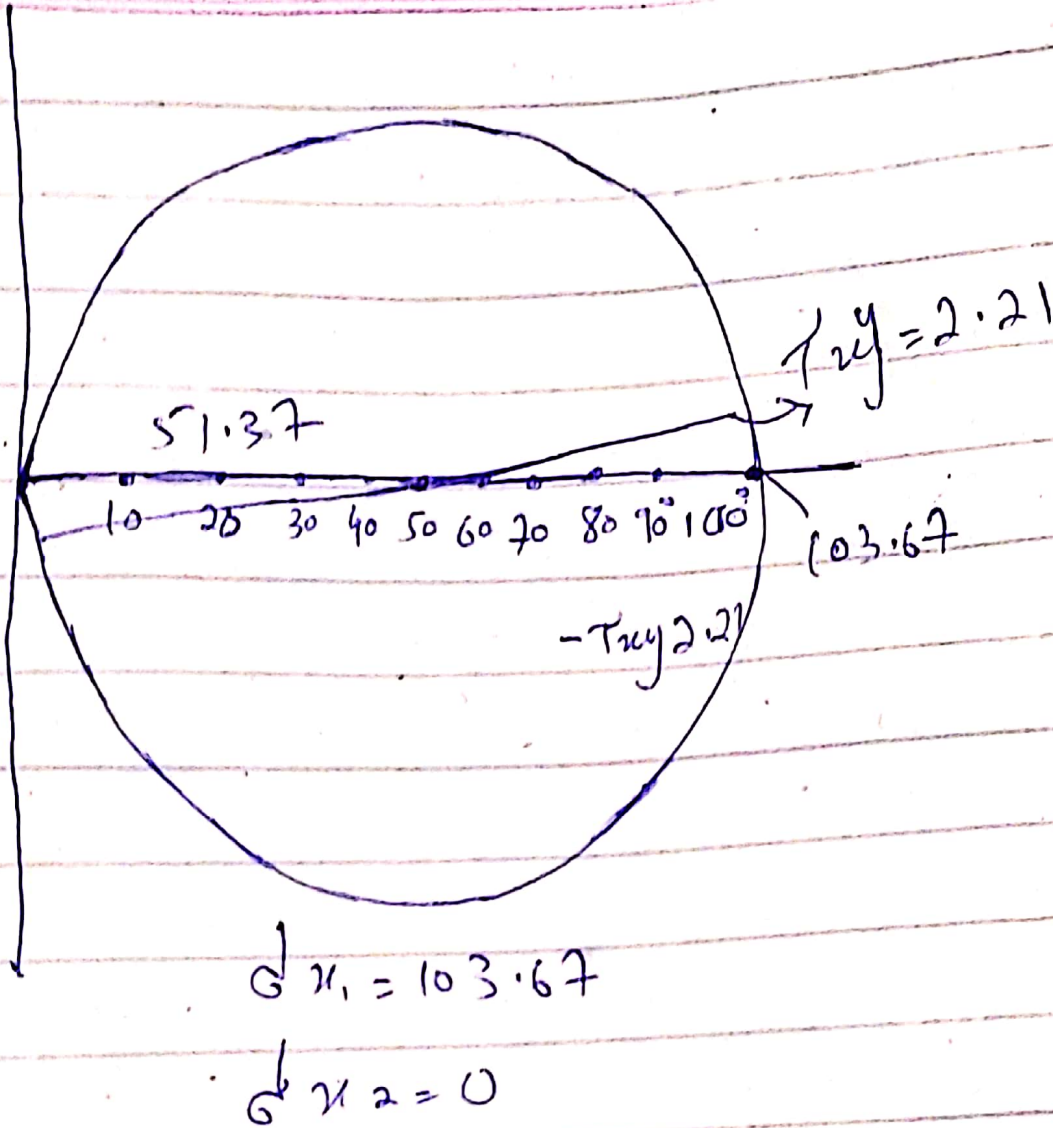
Radius of Mohr circles is

$$r = \sqrt{\frac{\sigma_x - \sigma_y}{2} + \tau_{xy}^2}$$

$$r = \sqrt{\frac{(-103.67 - 0)^2}{2} + (2.21)^2}$$

$$r = 51.33 \text{ ih}$$

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As shown from Mohr's circles
 The value obtain that of Principal
 stress & maximum shear stress are
 almost same with value obtained
 from transformation equation.