

(1)

Q.

" Difference b/w culvert and causeway "

" Culvert :-

Culvert is define as the that part of the structure which an opening through embankment for the conveyance of water through by means of pipe or other channels.

The culvert is many type is made from pipe, reinforced concrete or other materials.

OR

The culvert is a tunnel carrying a stream under a road or railway.

A culvert may be acts as a bridge on which the traffic is passed on it.

" Cause way :-

The cause way is define as it is

a track, road or railway on the upper part of an embankment across a low or wet places.

It is also define as Cause way is road that is constructed across and above water and marsh land. It is also constructed of earth on different materials like concrete or stone etc.

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part (B)

Cross Drainage work:

It is define as an irrigation canal while carrying water from head works to drop fields have to cross flow natural drainage stream. To cross these drainage safely by the canal some suitable structure are required to construct work required to construct to cross the drainage are called cross

## Drainage works.

IV The necessity of cross drainage works are given below:-

\* The water shed canals do not cross natural drainage but in actual orientation of the canal network this ideal condition may not be available and obstacles like natural drainage may be present across the canal, so the cross drainage works must be provided for ~~an~~ running the irrigation system.

\* That point at which canal and drainage water are intermixed so for the smooth running of the canal with its design discharge the cross drainage works are required.



\* The site condition of the crossing point may be such that without any suitable surface, the water of the canal and drainage can not be divided to their natural directions. So the cross drainage work must be provided to maintain their natural directions of flow.

\* Types of cross drainage work:

(1) Irrigation canal passes over the drainage.

(a) Aqueduct

(b) Siphon Aqueduct

(2) Drainages passes over the irrigation canal.

(a) Super passage

(b) Siphon super passage

(3) Drainage & canal intersect to each other.



- (a) Level crossing  
(b) Inlet & out let

(1) (d) Aqueduct:

The hydraulic structure in which the irrigation canal is taken over the drainage (such as river, stream etc) is known as Aqueduct.

(b) Siphon Aqueduct:

In a hydraulic structure where the canal is taken over the drainage, but the drainage water cannot pass clearly below the canal. It flows under the siphon action, so it is known as Siphon Aqueduct.

(2)

(a) Super Passage:

The hydraulic structure in which the drainage is taken over the irrigation canal is

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as a super passage.

(b) Siphon super passage:-

The hydraulic structure in which the drainage is taken over the irrigation canal, but the canal water is passed below the drainage water siphonic action is known as siphon super passage.

(3) (a)

Level crossing:-

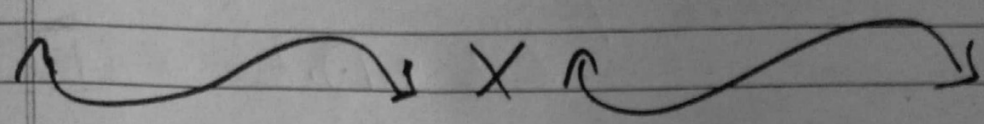
When the bed level of the canal & stream are approximately in canal & stream is not much different, the cross drainage work constructed is called level crossing.

(b) inlet & outlet:- ~~when~~

when irrigation canal meet a small stream or drain at same level, drain is allowed to

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enter the canal as in inlet. At some distance from this inlet point a part of water is allowed to drain as outlet.



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given Data:

~~L.L = 1.5 kP/ft<sup>2</sup>~~  
~~D.L = 300 lb/ft<sup>2</sup> = 0.3 kP/ft<sup>2</sup>~~

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given Data:

Dimension = 15ft x 15ft  
live load = 1.5 kP/ft<sup>2</sup> = 1500 lb/ft<sup>2</sup>  
Dead load = 300 lb/ft<sup>2</sup>  
unit of soil =  $\gamma_s$  = 100 lb/ft<sup>3</sup>  
Angle of response =  $\theta$  = 30°  
grade of concrete = 1:2:4 = m15  
 $f_y$  = 60 ksi  
Thickness of slab = 0.92ft

Required Data:

To Design box culvert.



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### \* Load Calculation:

as we know that,

$$\text{Total load carrying on Top Slab} = \text{self wt. of slab} + \text{L.L} + \text{D.L} \quad \text{--- (1)}$$

$$\text{Self wt of Top Slab} =$$

$$\text{Thickness} \times \text{grade of concrete}$$

$$= 0.92 \times 15$$

$$= \boxed{13.8 \text{ kN/m}^2}$$

Put in eq (1)

$$= 300 + 1500 + 13.8 = \boxed{1813.8 \text{ kN/m}^2}$$

### \* Coefficient of earth pressure:

$$K_a = \frac{1 - \sin \theta}{1 + \sin \theta} = \frac{1 - \sin 30^\circ}{1 + \sin 30^\circ}$$

$$= \boxed{0.33}$$

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\* Lateral pressure due to dead and live load:

= Total vertical load

$$( \text{Dead} + \text{live} ) \times K_a$$

$$= (300 + 1500) \times 0.33$$

$$= \boxed{594 \text{ lb/ft}^2}$$

Lateral pressure due to soil:

$$K = \gamma \times h$$

$$= 0.33 \times 100 \times 15 + 0.92$$

$$= \boxed{525.36 \text{ lb/ft}^2}$$

\* Lateral pressure due to Top:

$$D.L + L.L$$

Which we calculate above which is 594.

\* Lateral pressure at bottom:

lateral pressure due to (D.L + L.L) + taken pressure

ate: \_\_\_\_\_

ay: 

M	T	W	T	F	S	S
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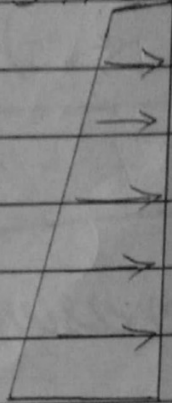
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due to soil.

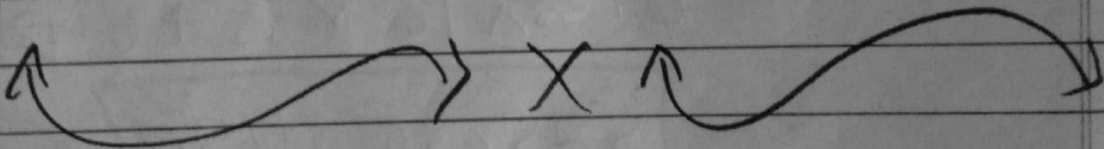
$$594.36 + 525.39$$

$$= \boxed{1119.75 \text{ lb/ft}^2}$$

594 lb/ft



1119.75 lb/ft<sup>2</sup>



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D=

Draw sketch Diagram:

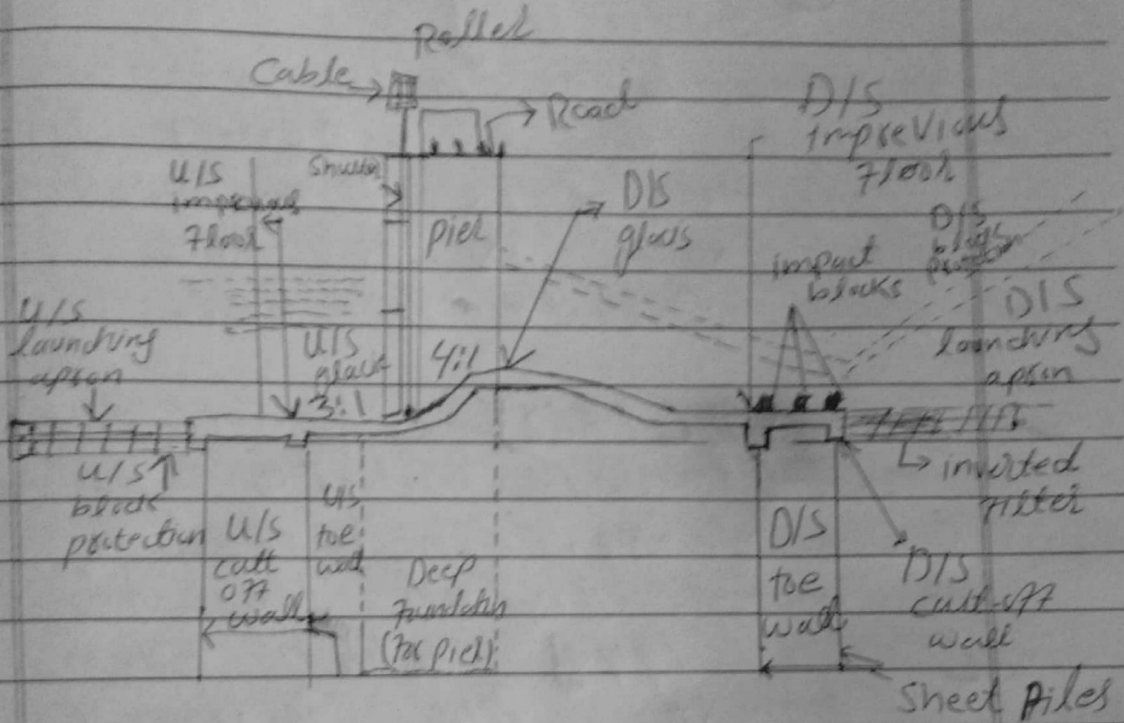
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(a) Draw sketch Diagram Components:



\* Component part of Barrage \*

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1 part (B)

Several formulae based on experimental results have been proposed to predict the maximum "equilibrium" scour depth ( $y_0$  below general bed level). around bridge piers. In

general, these assume the relationship

$$y_s b = \phi(y_s b', F_r, d/b)$$

where  $b'$  is the pier width,  $y_0$  is the upstream flow depth,  $d$  is the sediment size, and  $F_r$  is the Froude number.

Laursen (1962) experimental results underestimate the scour depth, compared to many Indian experiments (Inglis 1949) which are. (approach in normal to the bridge piers).

$$y_s b' = 4.2 (y_s b')^{0.78} F_r^{0.52}$$

The Indian field data suggest that the scour depth should be taken as twice the regime scour depth.

So ~~1/2~~  $\frac{y_s}{y_0} =$

$$y_s / y_0 = (B/b')^{1.7} - 1$$

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Predict the maximum equilibrium scour depth.

In a relative deep flow a first-order estimate of clear local sand scour (around pier) be obtained as,

$$Y_s = 2.3 K_a b'$$

Where  $K_a$  = angularity coefficient which is a function of the pier alignment.

\* Scour around bridge piers:

Several formulae based on experimental results have been proposed to predict the "maximum" or "equilibrium" scour depth ( $Y_s$  - below general bed level) around bridge piers. In general assume the relationship

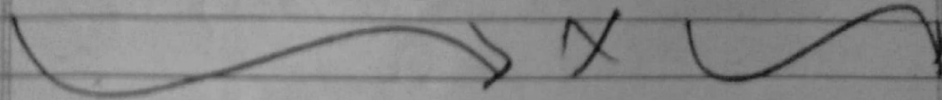
$$Y_s/b' = \phi C Y_s/b' = F V d/b'$$

Where  $b'$  is the pier



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width,  $y_0$  is the up stream flow depth,  $d$  is the sediment size, and  $F_r$  is the flow Froude number.



$\Phi = 1/2$   
 $\Phi = (a)$

Weir:

\* Weir is the structure constructed across the river to rise the level of water.

- \* Weir are cheaper structure.
- \* Weir is mostly constructed in hilly areas only to be rise the level of water.
- \* In weir after some time setting problems occurs.

Barrages:

\* Barrages is a

Structure constructed across a river in which adjustable gates (sluice gates) are installed over a dam to maintain the water surface different level at different time.

- \* Barrages in which much expensive than weir.
- \* Barrages is constructed near the city, which controlled the flow of water with the help of gates.
- \* In Barrages no settling problem occurs due to gates.

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## Reynold Number.

Reynold number is define as:

It is the ratio of inertial force and viscous forces.

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It has dimensionless quantity because it is the ratio of two similar quantities.

It is represented by "Rn".

Formula of "Rn"

$$Rn = \rho v d / \mu$$

$\rho$  = density

$v$  = velocity

$d$  = diameter

$\mu$  = viscosity

\* Laminar Flow:

The flow which Reynolds number value less than 2000 is known as laminar flow.

\* Turbulent Flow:

The flow which have Reynolds number value greater than 4000 is known as turbulent flow.

? Neither laminar nor turbulent  
The flow which have



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Reynold number range  
2000 - 4000 is known  
as neither laminar  
nor Turbulant Flow.

★ Lower critical velocity:  
it is also called  
sub-critical velocity,  
which have Froude  
number less than 1  
is known as lower  
critical velocity.

~~or~~  $Cx < 1$

lower critical velocity.

★ Higher critical velocity:

The flow which have  
Froude number greater  
than 1 is known as  
higher critical velocity.

It is also called  
super critical velocity.

$Cx > 1$  super critical velocity

↖ X ↗  
end.

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