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Section : "C"

Assignment : Hydraulic Structure

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## Question No. 01 part (a)

## Answer @ No (01) part (a)

Difference between Culvert and Causeway:

A culvert is a structure that allows water to flow under a road, rail, road, trail or similar obstruction from one side to the other side.

While a causeway is of course a raised road usually built on embankment after running across water or swampy land.

- Causeways is a road that raised as to above water and marshland etc. while culvert is transverse channel under road or railway for the draining of water.

- Culvert works as a bridge pass on it. While causeway is not a bridge because it supports a roadway between piers.

Question No. 02 part (b)

Answer question No. 02 part (b)

CROSS DRAINAGE WORK:

Cross drainage work is a structure constructed when there is a crossing of canal and natural drain to prevent the drain water from mixing into canal water.

By mixing two or three streams into one and only one cross drainage work to be constructed making the structure economical.

Necessity of Cross Drainage:

The Cross drainage work is required to dispose off the drain water so that the canal supply uninterrupted.

The canal at a cross drainage work is generally taken either over or below the drainage.

At the crossing point the water of the canal and the drainage get inter mixed. So for smooth running of the canal with its design discharge the cross drainage works are required.

## TYPES OF DRAINAGE WORK:

### Type - I

Irrigation Canal passes over the drainage:

Aqueduct: The hydraulic structure in which the irrigation canal is taken over the drainage (such as river, stream etc. is known as an aqueduct.

## Siphon Aqueduct :

- In a hydraulic structure where the canal is taken over the drainage but the drainage water can't pass clearly below the canal. It flows under siphonic action. So it is called siphon aqueduct.

## Type II

Drainage passes over the irrigation canal :

- Super passage :
- The hydraulic structure in which the drainage is taken over the irrigation canal is known as super passage. The structure is suitable when the bed level of drainage is above the full supply level of the canal. The water of the canal passes clearly below the drainage.



## Siphon Super Passage:

- The hydraulic structure in which the drainage is taken over the irrigation canal but the canal water passes below the drainage water under siphonic action is known as Siphonic Super Passage.

## TYPE III

Drainage and Canal intersect each other at the same level:

- Level Crossings.  
When the bed level of canal and the stream are approximately the same and quality of water in canal and stream is not much different the cross drainage work constructed is called level crossing.

where water of canal mix.

### inlet and outlet:

When irrigation canal meets a small stream or drain at same level, drain is allowed to enter the canal as inlet. At some distance from this inlet point, a part of water is allowed to drain as outlet which eventually meets the original stream.



## Question No. 02 part (a)

### Answer question No. (b2) part (a)

Differentiate between weir and barrage.

\* A weir is a concrete or masonry structure which is constructed across open channel. While barrage is a special kind of dam which consists of a line of large gates that can be opened or closed to control the amount of water.

\* A weir is an impermeable barrier that is built across a river to rise the water level at upstream side.

on the other hand barrage involves adjustable gates installed over a dam to maintain the water level.

\* Barrage are built near the cities so that the amount of water flowing in the river can be controlled by opening and closing the gates. While the weir is constructed is built near those are where tourist destination and preservation area to allow fish into swim upstream.

## Question No. (02) Part (b)

Answer Question No. 02  
Part (b)

Reynold's number:

It is the ratio of Inertia force to the viscous force of the flowing fluid.

$$Re = \left( \frac{\rho V L}{\mu} \right)$$

Limits of Reynold's number:

When the Reynold's number is less than (2000) the flow is laminar. This is also known as viscous flow.

When the Reynold's number is between (2000) and (4000) the flow is transition. not laminar and not turbulent.

When the Reynold's number

is greater than (4000) the flow is turbulent.

\* Lower critical velocity.  
when Froude number  $Fr < 1$  then such type of flow is called lower critical flow in this case critical depth is lower than scours depth  $y > y_c$  and  $V > V_c$  while velocity is lower critical velocity.

\* Higher critical velocity:  
when  $Fr > 1$  then such flow is called higher critical flow at this point critical depth is greater than scours depth  $y_c > y$  and velocity is called high critical velocity.



## Question No. (03) part (b)

Answer Question No. (03)  
Part (b)

Scour around bridge  
Piers:

Formula based on experimental result have been proposed to predict the maximum and equilibrium scour depth ( $y_s$  below given general bed level) around bridge piers. In general these assume the relationship:

$$y_s/b' = \phi (y_0/b', Fr, d/b')$$

where  $b'$  is the pier width  
 $y_0$  is the upstream flow depth  
 $d$  is the sediment size and  $Fr$  is the flow Froude number.

Lussen's experimental results underestimate the scour depth



Compared to many Indian experiment. which suggest the formula (approach flow is normal to the bridge pier).

$$y_s/b' = 4.2 (y_0/b')^{0.78} Fr^{0.52}$$

The Indian field data also suggest that the scour depth should be taken as twice the regime scour depth.

In the case of live beds a stream with bedload transport the formula.

$$y_s/y_0 = (B/b')^{5/7} - 1$$

predict the maximum scour depth.

## Question No. (04)

### Answer Question No. (04)

Solution:

Given Data:

$$\text{Dimension} = 15 \times 15$$

$$\text{L.L} = 1.5 \text{ kip/ft}^2 \rightarrow 1500 \text{ lb/ft}^2$$

$$\text{D.L} = 300 \text{ lb/ft}^2$$

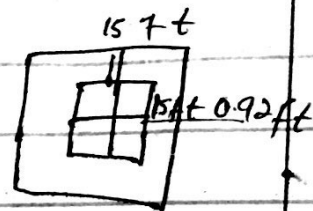
Unit weight of soil

$$\text{Required:} = 100 \text{ lb/ft}^2$$

$$\text{ratio } 1:2:4$$

$$f_y = 60 \text{ ksi}$$

① Load calculation:



Total load carrying on top slab.

$$= 0.92 \times 15$$

$$= 13.8$$

Total Load

$$\text{Live load} + \text{D.L} + 13.8$$

$$= 1500 + 300 + 13.8$$

$$= 1813.8 \text{ lb/ft}^2$$

Coefficient of earth pressure:

$$K_a = \frac{1 - \sin \phi}{1 + \sin \phi} = \frac{1 - \sin 30}{1 + \sin 30} = 0.33$$

(i) Lateral pressure due to (L.L + DL)

$$\begin{aligned} \text{Total vertical Load} \times K_a \\ &= 1800 \times 0.33 \\ &= 594 \text{ lb/ft}^2 \end{aligned}$$

(ii) Lateral pressure due to soil =  $K_a \times \gamma h$

$$\begin{aligned} &= 0.33 \times 15.92 \times 100 \\ &= 525 \text{ lb/ft}^2 \end{aligned}$$

$$\begin{aligned} \text{Lateral pressure at top} &= \\ \text{Lateral pressure due to (D.L + L.L)} \\ &= 594 \text{ lb/ft}^2 \end{aligned}$$

Bottom = Lateral pressure due to D.L + L.L + Lateral pressure due to soil.

$$\begin{aligned} &= 594 + 525 \\ &= 1119 \text{ lb/ft}^2 \end{aligned}$$

