

NAME

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Section

A

Semister

4th

Subject

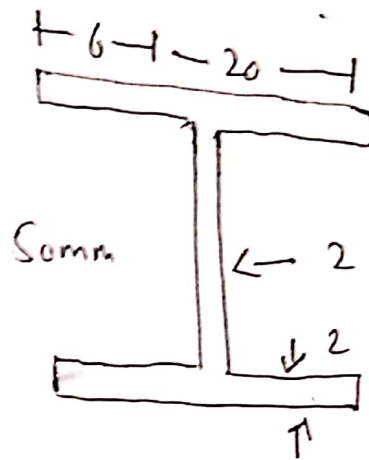
MOS II

Department

Civil engineering

Question 01 (1)

Part (a)



Required : location of shear centre;

Sol: As we know

$$e = \frac{t_f h^2 b^2}{4I}$$

and

$$I = 2 \left(\frac{bh^3}{12} + Ay^2 \right) + \left(\frac{bh^3}{12} + Ay^2 \right)$$
$$= 2 \left[\frac{26(2)^3}{12} + (20 \times 2)(25)^2 \right] + \left[\frac{2(50)^3}{12} + 0 \right]$$

$$I = 50034.66 + 20833$$

$$I = 70867.99 \text{ mm}^4$$

$$e = \frac{2(50)^2(26)^2}{4(70867.99)} = \boxed{11.02 \text{ mm}}$$

(2)

Question No 01

Part (b)

Data: $H = 26 \text{ ft}$

\Rightarrow Assume diameter

$$D = 22 \text{ ft}$$

\Rightarrow tangential stress = 600 lb/ft^2

\Rightarrow Specific weight of water tank = 62.4 lb/ft^3

We have to find the thickness.

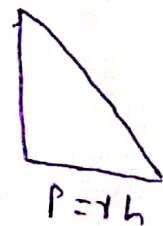
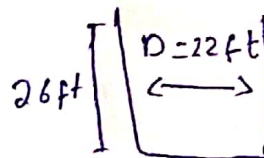
Thickness = ?

Solution: The pressure developed

by water = $P = \gamma h$

$$\sigma_t = \frac{PD}{2t}$$

$$\sigma_t = \frac{PD}{2t} \Rightarrow \frac{\gamma h D}{2t}$$



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(2)

$$2t \times b_t = r h D$$

$$2t = \frac{r h D}{b_t}$$

$$t = \frac{r h D}{b_t \times 2}$$

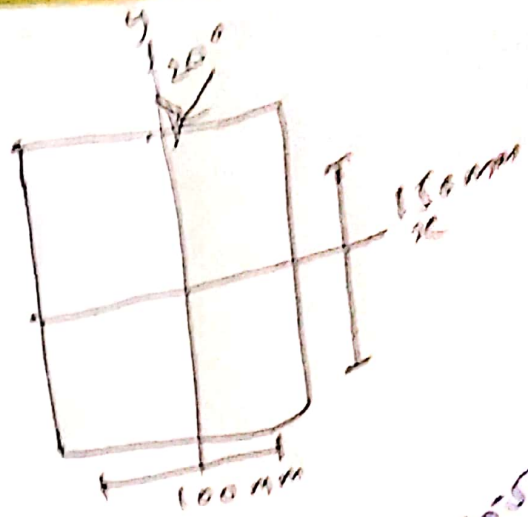
$$t = \frac{(62.4) \times (26 \times 12) \times (22 \times 12)}{(12)^2}$$

$$6000 \times 2$$

$$t = 0.24''$$

Q2 Part (a)

(4)



Moment of Inertia:

$$I_z = \frac{bh^3}{12} = \frac{0.1(0.15)^3}{12} = 2.8125 \times 10^{-5}$$

Now

$$I_y = \frac{bh^3}{12} = \frac{0.15(0.1)^3}{12}$$

$$I_y = 1.25 \times 10^{-5}$$

$$\theta = \frac{M_z y}{I_z} + \frac{M_y z}{I_y}$$

$$\theta = \frac{M \cos \theta}{I_z} + \frac{M \sin \theta}{I_y}$$

where

$$M = \cos \theta = P \cos \theta = M_z$$

$$= 1.2 \cos 60^\circ = M_z$$

$$M_z = 1.8510$$

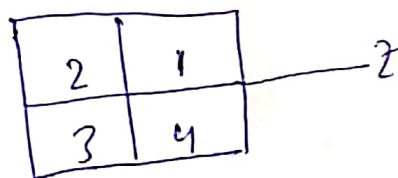
$$M \sin \theta = P \sin \theta = My \quad (5)$$

$$My = -11.8563$$

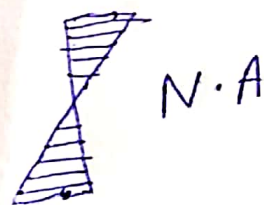
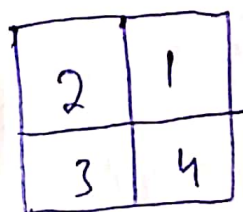
$$\sigma = \left(\frac{Mz}{Iz} \right) + \left(\frac{My}{Iy} \right)$$

$$\sigma = \frac{1.851}{2.812 \times 10^{-5}} + \left(\frac{-11.8563}{1.25 \times 10^{-5}} \right) = 88267.8 \text{ N/m}^2$$

Sign Convention



If we take Compression as negative and tension as positive and the beam is a simply supported.



Quadrant 1, 2 -ive
Quadrant 3, 4 +ive

In case of (b) unsymmetrical loading the neutral axis lies at an angle of " α " to the principle axis and the algebraic sum of stress at N.A is zero

$$\sigma = \frac{M \cos \theta y}{I_z} + \frac{M \sin \theta z}{I_y} \quad \text{--- (1)}$$

In this case, N.A passes through z, y , so

$$\sigma = \frac{M \cos \theta y}{I_z} + \frac{M \sin \theta z}{I_y}$$

Let consider point 'A' on N.A lies in quadrants, where

- Bending stress due to $P \cos \theta$ is compressive and
- Bending stress due to $P \sin \theta$ is tensile

$$\text{eq (7)} \Rightarrow 0 = \frac{-M \cos \theta y_A}{I_z} + \frac{M \sin \theta z_A}{I_y}$$

$$\Rightarrow 0 = \frac{-M \cos \theta y_A}{I_z} + \frac{M \sin \theta z_A}{I_y}$$

$$\Rightarrow \frac{M \cos \theta y_A}{I_z} + \frac{M \sin \theta z_A}{I_y}$$

$$\frac{y_A}{z_A} = \frac{I_z}{I_y} \frac{\sin \theta}{\cos \theta} \Rightarrow \tan \alpha = \frac{I_z}{I_y} \tan \theta$$

(ii)

Now put values of I_z, I_y
and θ in eq (ii)

$$\tan \alpha = \frac{I_z}{I_y} \tan \theta$$

$$\Rightarrow \tan \alpha = \frac{2.8125 \times 10^{-5}}{1.25 \times 10^{-5}} \quad (\tan 30^\circ)$$

$$\tan \alpha = -14.4129$$

$$\alpha = \tan^{-1}(-14.4129)$$

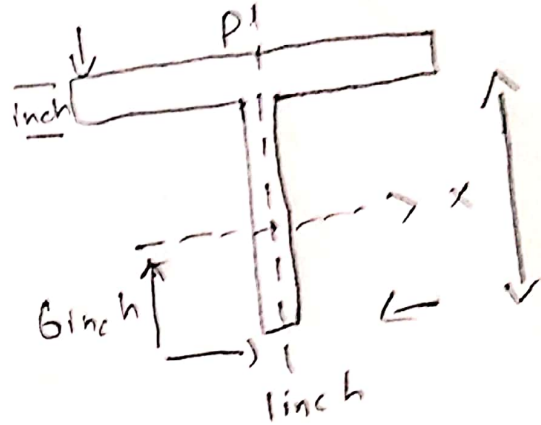
$$\alpha = 1.5^\circ$$

$$\alpha = 1^\circ 30' 5''$$

Q No 02

Part 'B'

Given:



$$L = 16 \text{ ft}$$

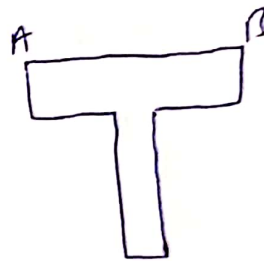
$$I_x = 112.6 \text{ inch}^4$$

$$I_y = 18.7 \text{ inch}^4$$

$$S_e = 12000 \text{ psi}$$

$$S_t = 5000 \text{ psi}$$

Solution:



By figure we can judge
that maximum compression
would occur on a axial
maximum tension at B
There will tension as well

a
 Compression which will
 reduce that effect of each
 other so we will
 calculate stress at A and
 C.

So

$$S_A = \frac{M_{xy}}{I_x} + \frac{M_{yx}}{I_y} \text{ (Compression)}$$

$$S_C = \frac{M_{xy}}{I_x} + \frac{M_{yx}}{I_y} \text{ (Tension)}$$

Now M_x and M_y

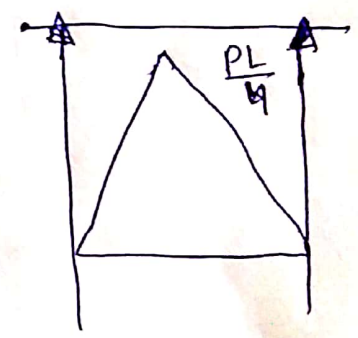
So

$$M_x = \frac{P \cos 60^\circ (16 \times 12)}{4}$$

$$M_x = 48 P \cos 60^\circ$$

$$M_y = \frac{P \sin 60^\circ (16 \times 12)}{4}$$

$$M_y = 48 P \sin 60^\circ$$



Now

$$S_A = \frac{M_x y}{I_x} + \frac{M_y x}{I_y}$$

$$\Rightarrow 1200 = \frac{48P \cos 60^\circ \times 3.07 +$$

$$\frac{48P \sin 60^\circ \times 3}{18.7}$$

Solving the equation

$$\Rightarrow \boxed{P = 1638.6 \text{ lb}}$$

Now

$$S_C = \frac{M_x y}{I_x} + \frac{M_y x}{I_y}$$

$$5000 = 48P \cos 60^\circ \times (5.93 \text{ ft}) +$$

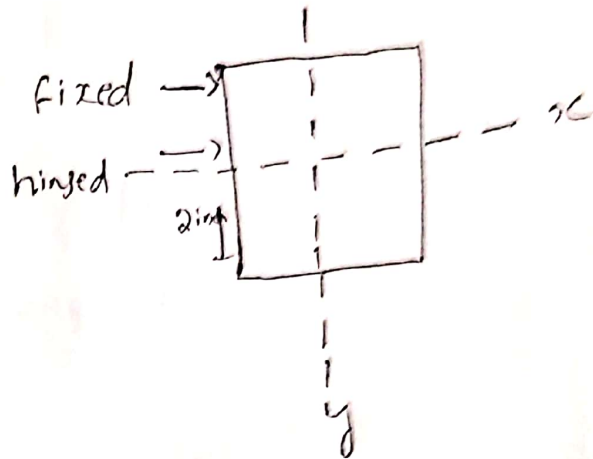
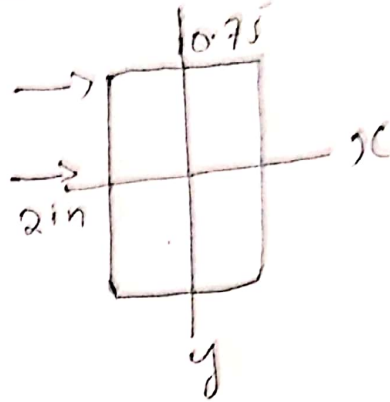
$$\frac{48P \sin 60^\circ \times 0.5}{18.7}$$

Solving the equation

$$\boxed{P = 2104.91 \text{ lb}}$$

So the maximum load P applied should 1638.6 lb

Question No (2)



Given Data:

Length = 10ft

Breadth = 0.75"

height = 2"

Factor of Safety = 2

$E = 10.3 \times 10^6$

Required Data:

Safe load = ?

Sol:

Case 1:

Struct Column act as a
hinged Column about an
axis perpendicular to the
2nd dimension then

$$I = I_x = \left(\frac{3}{4}\right) (2)^3 = 0.5 \text{ in}^4$$

$$L_e = L \text{ (for hinged ended column)}$$

$$P_{cr} = \frac{n^2 EI \pi^2}{L_e^2}$$

$$P_{cr} = \frac{(1)^2 (10.3 \times 10^6) (0.5) (\pi)^2}{(10 \times 12)^2}$$

$$\Rightarrow P_{cr} = 3526.17$$

$$P_{safe} = \frac{P_{cr}}{\text{factor of safety}}$$

$$P_{safe} = \frac{3526.17}{2}$$

$$\Rightarrow P_{safe} = 1767.08$$

Case II:

Column act as a fixed to end
about axis parallel to z in
i.e y axis

$$I = I_y = \frac{(2)(0.75)^3}{12}$$

$$\Rightarrow I_y = 0.07 \text{ in}^4$$

Now for fixed ended

$$L_e = L/2$$

$$P_{cr} = \frac{\pi^2 EI \pi^2}{L_e}$$

$$\Rightarrow P_{cr} = \frac{(1)^2 (10.3 \times 10^4) (0.07) (3.14)^2}{(12/2)}$$

$$\Rightarrow \boxed{P_{cr} = 1974.65 \text{ lb}}$$

~~For~~ ~~safe~~

For safe : Page No (14)

$$P_{safe} = \frac{P_{e1}}{\text{factor of safety}}$$

$$P_{safe} = \frac{1974.65}{2}$$

$$\Rightarrow P_{safe} = 987.32 \text{ lb}$$

In both cases we take
Smaller value of P_{safe}

$$P_{safe} = 987.32 < 1763.07$$