

SUBMITTED TO  
ENGR ADEED

SUBMITTED BY  
HAMAD

STUDENT ID  
7747

SECTION  
B

SUBJECT:

HYDRAULIC STRUCTURES

# QUESTION # 1 (A)

## GULVERT

## CAUSEWAY

→ Culvert is defined as tunnel structure constructed under roadway or railways to provide cross drainage or to take electrical or other cable from one side to other.

→ It is totally enclosed by soil or ground. Pipe culvert, box culvert and arch culvert are the common type used under roadways and railways.

→ Used for natural flow of water for controlling it.

→ A causeway is a raised road built as embankment.

→ It is supported mostly by earth or stone.

→ And it is not a bridge because it supports a roadway b/w piers.

→ It can be constructed of earth, masonry, wood or concrete for track road or railway.

### QUESTION # 01 (B)

#### CROSS DRAINAGE WORKS:

→ Cross drainage is structure which is constructed when there is crossing of channel and natural drain.

To prevent the drain water from mixing into channel water.

→ By mixing two or more stream into one and only one cross drainage work to be constructed.

#### NECESSITY OF CROSS DRAINAGE WORK

The water-shed channel do not cross the natural drainages but in actual orientation of the channel network this condition may not be available and obstacles like natural drainages may be present across the

channel. So, the cross drainage works must be provide for running the irrigation system.

→ When the water cross, the water of the channel and the drainage get intermixed for the smooth running of the water (channel) so design discharge the cross drainage works.

→ The cross drainage work required to dispose of the drainage water so that the channel supply water remain undisturbed.

## TYPES OF CROSS DRAINAGE WORK

follow are the types as under.

### TYPE - I

Irrigation channel passes over drainage.

# 1 AQUEDUCT : DEFINITION

The hydraulic structure in which the irrigation channel is taken over the drainage is called aqueducts.

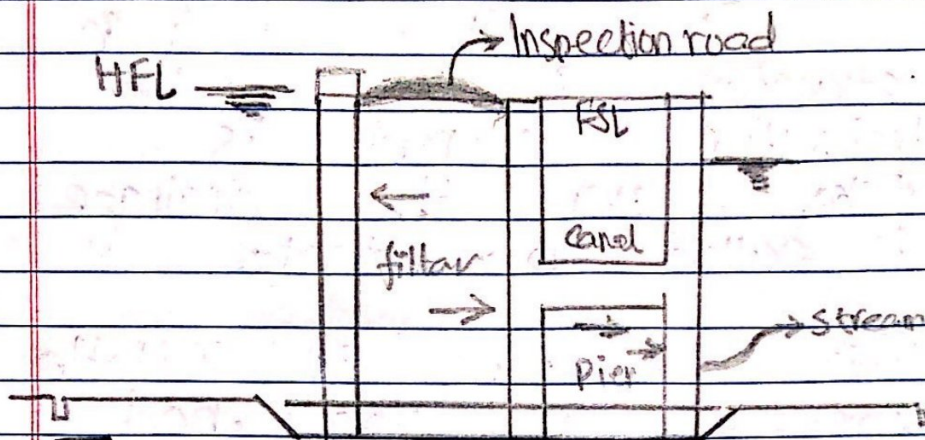
→ This structure is suitable for when the bed level is greater or above the highest flood level of drainage. In this case, the drainage water passes clearly below the channel.

# 2 SIPHON AQUEDUCT :

In a hydraulic structure where the channel is taken over the drainage but the drainage cannot pass clearly below the channel it flows under siphon action so it is called aqueduct.

→ Siphon aqueduct is suitable when the bed below

the highest flood level.



**TYPE - II**  
DRAINAGE PASSES OVER THE  
IRRIGATION CHANNEL

**SUPPER PASSAGE :**

The structure in which the drainage is taken over the irrigation channel is known as super passage.

→ It is suitable when the bed level of drainage is above the full supply level of the channel.

→ The water of the channel passes clearly below the drainage.

Babar Paper Product

Checked By: ..... Parents: ..... Excellent  Good  Need Improvement

### SIPHON SUPER PASSAGE :

The hydraulic structure which the drainage is taken over the irrigation etc canal, but the canal water passes below the canal (Drainage) under siphonic action is called siphon super passage.

It is suitable when the bed level of drainage is below the full supply level of the canal.

### TYPE - III DRAINAGE AND CANAL INTEREST EACH OTHER AT THE SAME LEVEL

### LEVEL CROSSING :

When the bed level of the canal and the stream are approx the same and quality of water in canal and stream is not much different the cross drainage

work constructed are known as level crossing.

- later of channel canal and stream allowed to mix with the help of regulator both in canal and stream,
- water is disposed through level and stream is required quantity
- Level crossing consist of crest wall stream regulator canal regulator.

### INLET & OUTLET :

When irrigation canal meets a small stream or drain at same level, drain is allowed to enter the canal as inlet.

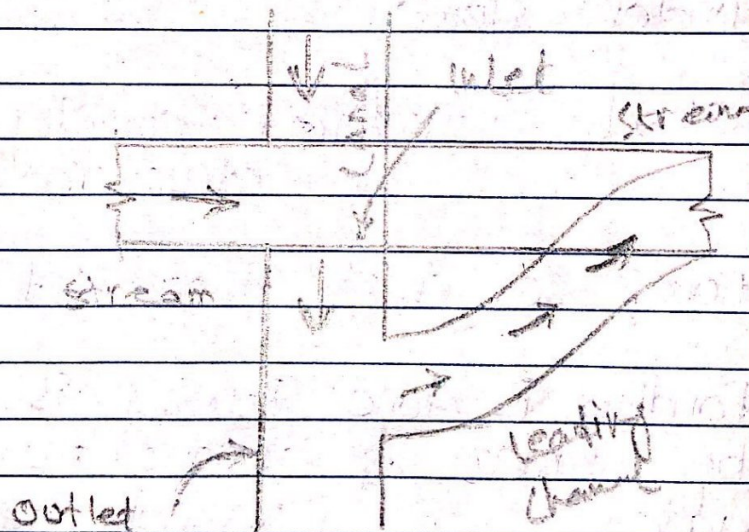
- At same distances from this inlet point, part of water is allowed.



to drainage as out let.

→ Eventually meets the original stream, stone pitching is required at the inlet and out let

→ The bed and banks b/w inlet and out let are also protected by stone pitching. This type of CPW is know as inlet and out let.



## QUESTION # 2 (a)

## DIFFERENCE B/W WEIR &amp; BARRAGE

## BARRAGE

High flood can be passed with minimum afflux.

Longer construction period.

Silt removal is done through under sluices.

Closely structure

Low set crest

Ponding is done by means of gates.

Gate over entire length.

Gate are at greater height.

## WEIR

Excessive afflux is high flood.

Shorter construction period.

No mean for silt disposal.

Relatively cheaper structure.

High set crest.

Ponding is done by means against the raised crest.

Shutter are part length.

Shutter are of lower height.

**QUESTION #2 (B)****REYNOLD NUMBER**

Reynold number  
 can be define as ;  
 "The ratio of an Inertial  
 forces to viscous forces."

→ Reynold number is also  
 use to predict if a  
 flow condition will be  
 laminar or turbulent or  
 transition.

**FORMULA :**

$$Re = \frac{\text{Inertial force}}{\text{viscous Forces}}$$

**LAMINAR FLOW :**

→ The flow is  
 which the Reynold number  
 is less than 2000

$$\rightarrow Re < 2000$$

**TURBUCENT FLOW :**

The flow  
 in which the Reynold  
 number is greater than 4000

Baber Paper Product

Checked By: ..... Parents: ..... Excellent  Good  Need Improvement

$$Re > 4000$$

### TRANSITION FLOW:

→ The flow at which the Reynold nbr is greater than 2000 and less than 4000

$$\rightarrow Re > 2000 \leq 4000$$

### LOWER CRITICAL VELOCITY

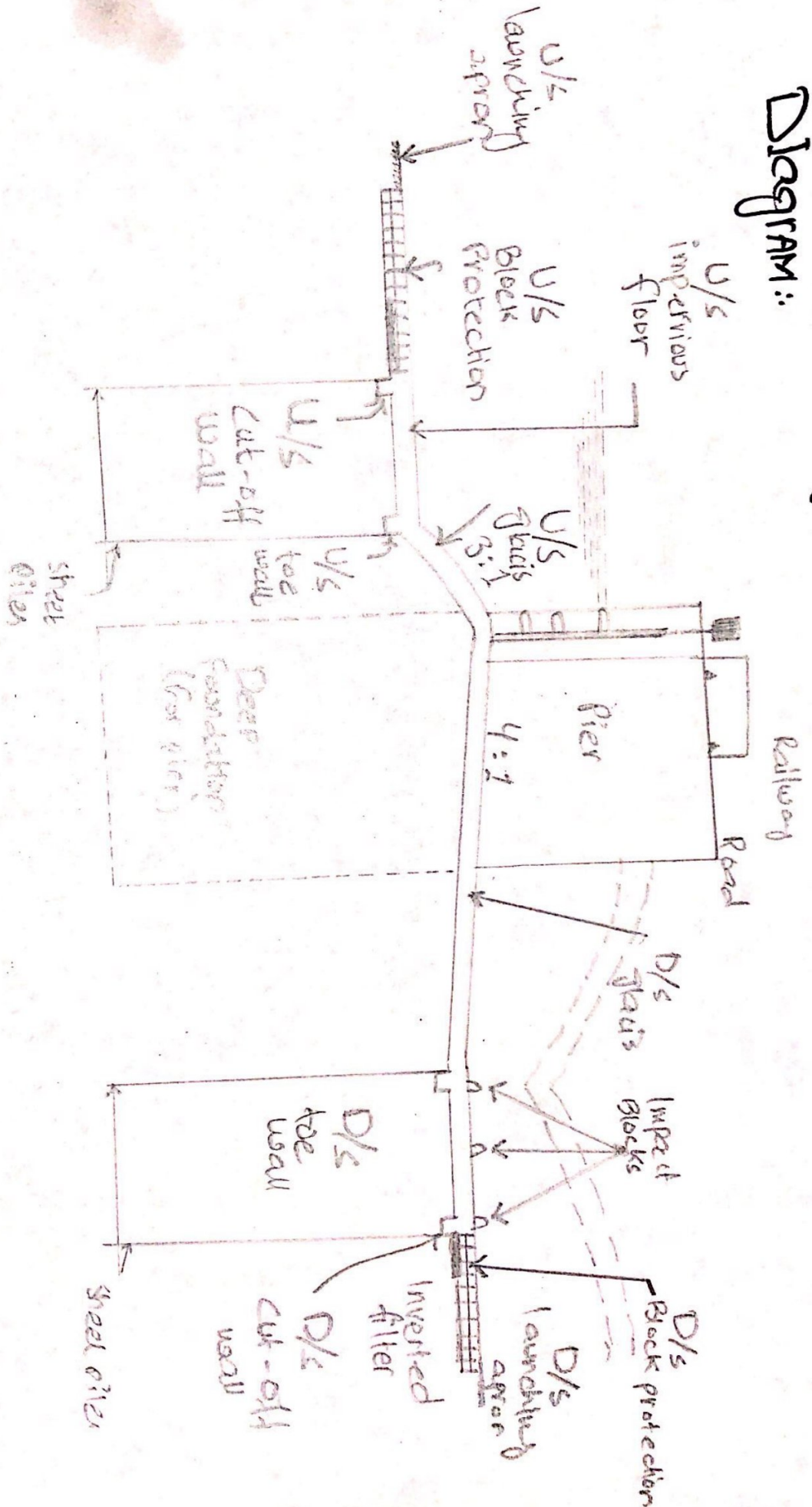
the flow which changes from laminar to transition state

### HIGHER CRITICAL VELOCITY

the flow which changes from transition to turbulent flow called higher critical velocity.

# QUESTION # 3 (A)

Diagram:



## QUESTION # 3 (B)

## ANSWER :

Several formula based on experiment result have been proposed to predict the maximum or equilibrium scour depth ( $Y_s$  below the ground bed level) around bridge piers, in general, these the assume relationship.

$$Y_s/b' = \phi (Y_0/b', Fr, d/b')$$

Where  $b'$  is the pier width,  $Y_0$  is the upstream flow depth,  $d$  is the sediment size and  $Fr$  is the Froude number.

Laurson's (1962) experimental result underestimate the scour depth, compared to many Indian experiment which suggest the formula.

$$Y_s/b' = 4.2 (Y_0/b')^{0.78} Fr^{0.33}$$

The scour should be taken as twice the regime  $\times$  scour depth.

→ In case of live bed (a stream with bedload transport) the formula.

$$y_s/y_0 = (B/b')^{5/7} - 1$$

Predicts. The Maximum Equilibrium scour depth.

In relatively deep flow a first order estimate of local scour may obtained by.

$$y_s = 2.3 k_a b'$$

Where  $k_a$  = angularity coefficient which is a function of the pier alignment.

i.e. angle of attack of approach flow.

→ The best estimate will be achieved with the appropriate coefficient for flow depth, alignment etc.

→ The live bed may contribute to an appreciable reduced local scour depth if the sediment bed is distinctly layer and cover layer of thickness less than the local scour depth the overall scouring phenomena is totally different.

→ The stepped scour depth in the lowering layer, it is given by

$$H = \eta (y_2 - y_1)$$

→  $y_2 - y_1$  are the uniform flow depth over a flat bed of grain roughness corresponding to the upstream surface particles ( $\Delta_1$ ) and the underlying



Surface and fine particles ( $d_s$ ) respectively.

Coefficient of silt and clay forming sediment  $\eta = 2.6$  for design purpose

→ The total scour depth may lead to a gross underestimate if the lower layer is of very fine material.

## QUESTION # 4

### Given data:

Inside dimension = 15' x 15'

Live load = L.L. = 1.5 K/ft<sup>2</sup>

Dead load = D.L. = 300 lb/ft<sup>2</sup>

Unit weight of Soil = 100 lb/ft<sup>3</sup>

Angle of repose = 30°

Use concrete of 1:2:4 ratio

$f_y = 60$  ksi

Thickness = 0.92m = 3ft

### Required ::

Design culvert = ?

### Solution:

#### Load calculation

Total load carry on top slab

= self wt of slab + L.L. + D.L.

self wt of slab = 3 x 150  
= 450 lb/ft<sup>2</sup>

$$W = 450 + 1800 + 300 = 2250 \text{ lb/ft}^2$$

Co-efficient of earth pressure :

$$K_a = \frac{1 - \sin \theta}{1 + \sin \theta}$$

$$K_a = \frac{1 - \sin 30^\circ}{1 + \sin 30^\circ}$$

$$K_a = 0.33$$

Lateral Pressure Due to  
(D.L + L.L)

Total vertical load (L.L + D.L)

$$\begin{aligned} & \times K_a \\ & (1800 + 300) \times 0.33 \end{aligned}$$

$$= 594 \text{ lb/ft}^2$$

Lateral pressure due to soil

$$\begin{aligned} & = K_a \times V_h \\ & = 0.33 \times 100 \times 18 \\ & = 594 \text{ lb/ft}^2 \end{aligned}$$

Lateral Pressure :

Top

Lateral pressure due to (D.L + LL)  
= 874 lb/ft<sup>2</sup>

Bottom

Lateral pressure due to  
(D.L + L.L) + lateral pressure  
due to Soil

$$= 874 + 874$$

$$= 1188 \text{ lb/ft}^2$$

