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* Subject:-

* MOS-II

* Submitted to:-

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Q 1- Part "a"

Given Data:-

Upper web = 6mm and 20mm

Flange = 2mm

thickness = 2mm

Req. Data:-

Location of shear center = ?

Solution:-

$$e = \frac{t_f h^2 b^2}{4I}$$

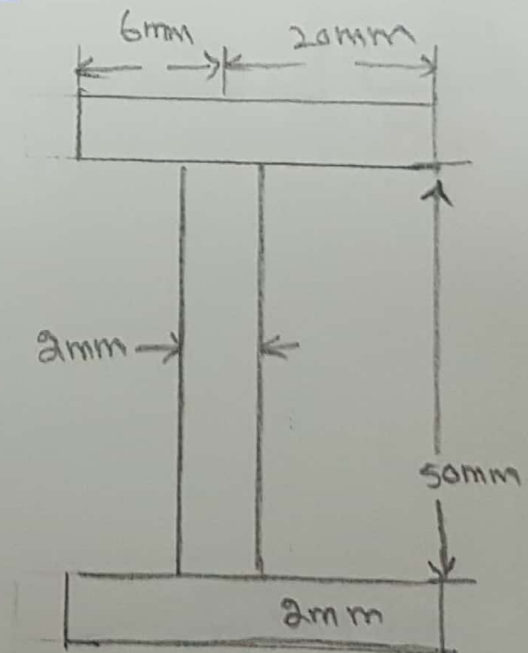
$$I = 2 \left(\frac{26(2)^3}{12} + (26 \times 2)(24)^2 \right) + \left(\frac{2(46)^3}{12} \right)$$

$$I = 29969.33 + 16222.66$$

$$I = 76161.33 \text{ mm}^4$$

$$e = \frac{2(50)^2(26)^2}{4 \times 76161.33}$$

$$e = 11.099 \text{ mm}$$



Q: No. 1 - Part "b"

o Given Data:-

height upto which tank is filled, $h = 26 \text{ ft}$

water tank diameter, $d = 22 \text{ ft}$

Circumferential stress, $\sigma_t = 6000 \text{ psi}$

Specific weight of water, $\gamma_w = 62.4 \text{ lb/ft}^3$

o Required Data:-

Thickness of wall of water tank, $t = ?$

o Solution:-

pressure developed

$$P = \gamma h$$

Circumferential stress,

$$\sigma_t = \frac{PD}{2t}$$

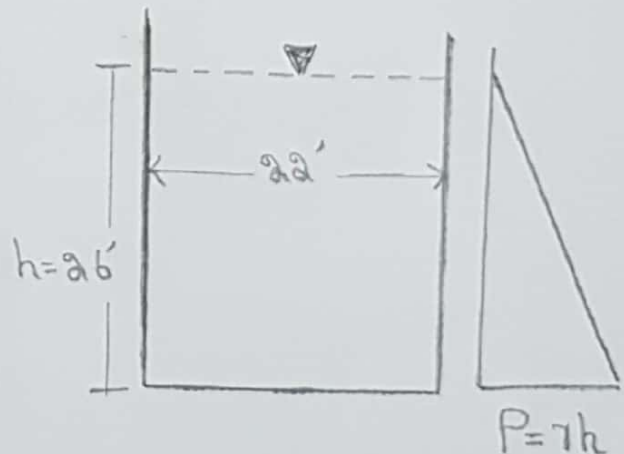
$$\sigma_t = \frac{\gamma h D}{2t}$$

So thickness is

$$t = \frac{\gamma h D}{2 \times \sigma_t}$$

$$t = \frac{62.4}{12^3} * 26 * 12 * 22 * 12 \text{ in} = \boxed{0.247 \text{ inch}}$$

(P-T-O)



Q No:2 part: A

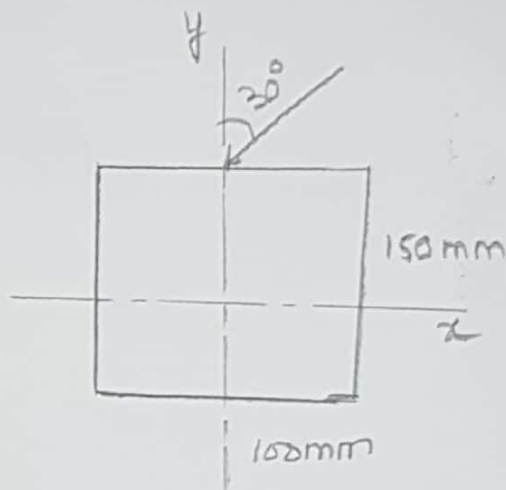
Given Data:-

beam section = 100x150 mm

Uniform load = 4 kN

Span = 3m

angle with vertical = 30°



o Req. Data:-

maximum bending stress at mid span
and for same section locate neutral axis.

o Solution:-

Moment of inertia

$$I_z = \frac{bh^3}{12} = \frac{0.1m (0.15)^3}{12} = 2.8125 \times 10^{-5} m^4$$

$$I_y = \frac{bh^3}{12} = \frac{0.15(0.1)^3}{12} = 1.25 \times 10^{-5} m^4$$

$$\sigma = \frac{M_z y}{I_z} + \frac{M_y z}{I_y}$$

$$\sigma = \frac{M \cos \alpha y}{I_z} + \frac{M \sin \alpha z}{I_y}$$

$$M_z = M \cos 30 = 12 \times \cos 30^\circ$$

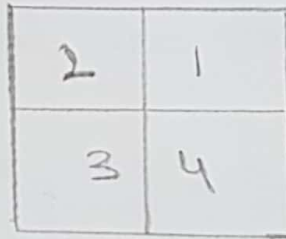
$$M_z = 10.39 \times 10^3 \text{ N}\cdot\text{m} = 10392.30 \text{ N}\cdot\text{m}$$

$$M_y = M \sin 30^\circ = 12 \times \sin 30 = 6000 \text{ N}\cdot\text{m}$$

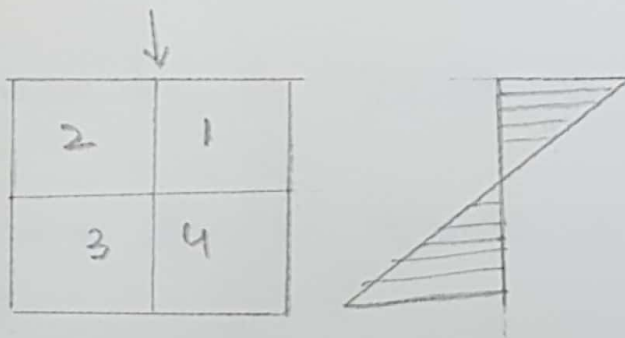
$$\sigma = \frac{10392.3 \times 0.075}{2.8125 \times 10^{-5}} + \frac{6000 \times 0.05}{1.25 \times 10^{-5}}$$

$$\sigma = 51712800 \text{ N/m}^2$$

Sign Convention:-

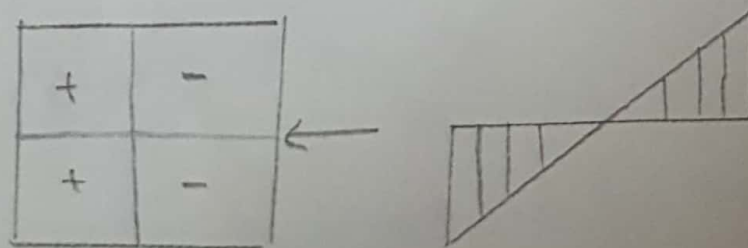


if we take compression as negative and tension as +ve and the



Quadrant 1,2 , -ve

Quadrant 3,4 , +ve



Quadrant 1,4, -ve
 Quadrant 2,3, +ve

* In case of unsymmetrical loading the neutral axis lies at an angle of " α " to the principal axis and the algebraic sum of stress at N.A is zero in this case neutral axis pass through 2,4

$$\sigma = \frac{M \cos \theta y}{I_z} + \frac{M \sin \theta z}{I_y}$$

- * let consider a point "A" on N.A lies in quadrant "2" where
- * Bending stress due to $P \cos \theta$ is compressive
- * Bending stress due to $P \sin \theta$ is tensile

$$0 = \frac{M \cos \theta y}{I_z} + \frac{M \sin \theta z}{I_y}$$

$$0 = -\frac{M \cos \theta y}{I_z} + \frac{M \sin \theta z}{I_y}$$

$$\Rightarrow \frac{M \cos \theta y}{I_z} = \frac{M \sin \theta z}{I_y}$$

$$\frac{y}{z} = \frac{I_z M \sin \theta}{M \cos \theta I_y} = \frac{I_z}{I_y} \tan \theta$$

$$\Rightarrow \tan \alpha = \frac{I_z}{I_y} \tan \theta$$

$$\Rightarrow \tan \alpha = \frac{I_z}{I_y} \tan 30^\circ$$

$$\tan \alpha = \frac{2.8125 \times 10^{-5}}{1.25 \times 10^{-5}} (\tan 30^\circ)$$

$$\tan \alpha = ~~1.44~~ 1.294$$

$$\tan \alpha = 1.294$$

$$\alpha = \tan^{-1}(1.294)$$

$$\boxed{\alpha = 52.2^\circ}$$

Q: 2: - Part 1b: -

Given Data:-

length = 16 ft

angle of load = 60 degree

$I_x = 112.6 \text{ in}^4$

$I_y = 18.7 \text{ in}^4$

Compressive stress = 12000 psi

tensile stress = 5000 psi

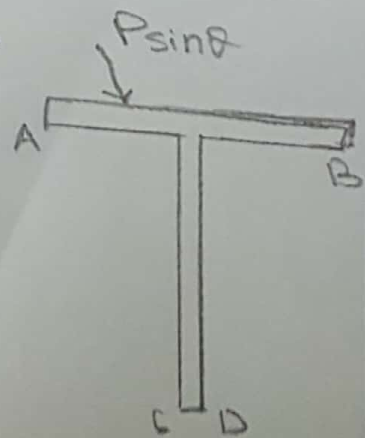
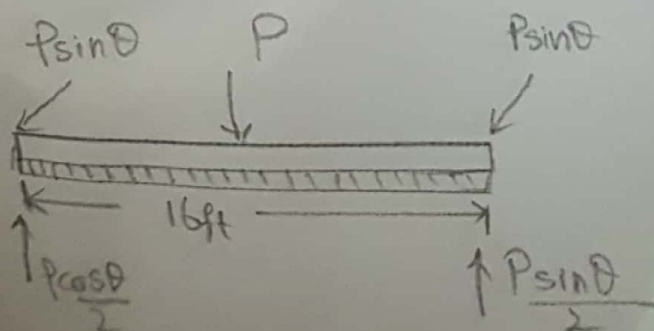
* Required Data:-

what is maximum load. that will not overstress load.

o Solution:-

The maximum bending stress occur at the mid section due to minimum bending moment so the critical section is the mid section. where the compressive and tensile stresses can exceed the limiting values

we also know that at any given section the maximum stress occur at the extreme fiber for example point A, B, C, D.



for point load max moment at mid section

$$M = \frac{PL}{4}$$

So,

$$M_x = \frac{(P \cos 60)(16 \times 12)}{4}$$

$$M_y = \frac{(P \sin 60)(16 \times 12)}{4}$$

Stress at A, B, C, D

at 'A'

$$\sigma = \frac{M_x y}{I_x} + \frac{M_y x}{I_y}$$

$$= - \frac{(48 P \cos 60)(3.07)}{112.6} + \frac{48(P \sin 60)(3)}{18.7}$$

$$= -0.654P + 6.6P$$

$$= 6.01P \text{ tension}$$

$$\text{tension} \leq 5000 \text{ psi}$$

$$5000 = 6.01P$$

$$P = \frac{5000}{6.01} = 831.94 \text{ lb}$$

A+B

$$\sigma = \frac{-48 P \cos 60 (3.07)}{112.6} - \frac{48 (P \sin 60) (3)}{18.7}$$

$$= -0.654p - 0.6p$$

$$= -7.2p \quad \text{compressive}$$

Compression ≤ 12000 psi

$$12000 = -7.25p$$

$$p = \frac{12000}{7.25} = 1655 \text{ lb}$$

$$1655 \text{ lb} > 831.9$$

Controlling value is 831.9 lb

A+C

$$\sigma = \frac{M_x y}{I_x} + \frac{M_y x}{I_y}$$

$$= \frac{48 P \cos 60 (5.93)}{122.6} + \frac{48 P \sin 60 (0.5)}{18.7}$$

$$= 1.26p + 1.11p$$

$$\sigma = 2.37p$$

$$\angle \text{ tension} \leq 5000 \text{ psi}$$

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$$5000 = 2.37 p$$

$$p = \frac{5000}{2.37} = 2109.71 \text{ lb}$$

at D:-

$$f = \frac{48p \cos 60 (5.93)}{112.6} - \frac{48p \sin 60 (0.5)}{18.7}$$

$$6 = 1.26p - 1.4p$$

$$6 = 0.15p$$

$$p = \frac{5000}{0.15} = 33333.33 \text{ lb}$$

So controlling value is 2109.71 lb

Q No: 3:-

① Given Data:-

Length of strut = 10 ft

Rectangular section = 0.75 in X 2 in

factor of safety = 2

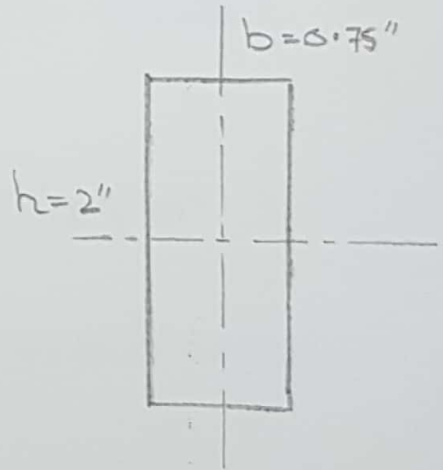
$E = 10.3 \times 10^6$ psi

① Required Data:-

Determine safe load, $P = ?$

① Solution:-

* Strut is a compression member and act as a column.



① Case: 1

strut act as on hinged column about an axis perpendicular to the 2 in dimension then.

$$I = I_x = \frac{(0.75 \text{ in})(2 \text{ in})^3}{12} = 0.5 \text{ in}^4$$

$$L_e = L \quad (\text{for hinged ended column})$$

$$P_{cr} = \frac{n^2 EI \pi^2}{L_e^2}$$

$$P_{cr} = \frac{(1)^2 (10.3 \times 10^6) (\pi^2) (0.5)}{(6 \times 12)^2}$$

$$P_{cr} = 9804.8 \text{ lb}$$

$$P_{\text{safe}} = \frac{P_{\text{cr}}}{\text{factor of safety}}$$

$$P_{\text{safe}} = \frac{9804.8 \text{ lb}}{2}$$

$$P_{\text{safe}} = 4902.43 \text{ lb}$$

* Case-2

strut or column act as a fixed ended column about an axis parallel to ~~of~~ - 2 in dimension

$$I = \bar{I}_y = \frac{2(0.75)^3}{12} = 0.07 \text{ in}^4$$

$$L_e = \frac{L}{2} \text{ (for fixed ended column)}$$

then

$$P_{\text{cr}} = \frac{n^2 EI \pi^2}{L_e^2}$$

$$= \frac{(1)^2 (10.3 \times 10^6) (0.07) (\pi^2)}{\left(\frac{6 \times 12}{2}\right)^2}$$

$$P_{\text{cr}} = 5490.7 \text{ lb}$$

$$P_{\text{safe}} = \frac{P_{\text{cr}}}{\text{factor of safety}}$$

$$P_{safe} = \frac{5490.7 \text{ lb}}{2}$$

$$P_{safe} = 2745.36 \text{ lb}$$

* In both cases take smaller value of P_{safe}

$$P_{safe} = 2745.36 < 4902.43 \text{ lb}$$

