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1.(a) What is re-tempering of concrete? In which case is re-tempering of concrete done?

(b) What is the normal RPM of the agitator of a transit mixer? What is the minimum limitation of total revolutions of agitator in a transit mixer set by ASTM before concrete placement?

Ans: Re-tempering of concrete:

When water is added to a stiffened concrete or partially set fresh concrete in order to bring it back to the desired consistency or workability, then it is called "retempering of concrete.

Retempering is done owing to loss of workability or

undue stiffness of concrete at actual side in case of long

tunnels, road construction etc. where batching plant is

few kilometres away.

(b):

Transit mixers have capacity ranging from 4-7m³. Speed of agitator varies

from 2 to 5rpm. A limit of 300 revolutions has been set by ASTM before

placing.

2.(a) What will be the expected loss in strength of 3000psi concrete if it curing has not been performed at all?

(b) What is the percentage efficiency of membrane curing as compared to water curing?

(c) What is meant by retrogression of strength in concrete? Which method of curing promotes retrogression in concrete strength?

Ans:(a).The expected loss in strength of 3000 psi concrete if curing has not been performed at all will be 60%.

(b). Membrane curing is 80% efficient as compared to water curing.

(c). Strength retrogression is defined as a change in the hydration products that are formed when **cement** is exposed to high temperatures (>110°C / 230°F). It can be described as a decline of **cement strength** at elevated temperatures where decreased **strength** is observed with increasing time.

Application of heat promotes retrogression in concrete strength.

- 3.(a) What do you mean by endurance level? What is the endurance level of concrete and steel?
- (b) What is the difference between attrition and erosion of concrete?

(c) What steps should be taken to improve bond strength of reinforcement in concrete?

Ans: (a)Endurance level:

A fluctuating stress i.e. with positive and negative phase is the root cause of fatigue failure. ... Thus, there is existence of hypothesis that there exists stress amplitude below which it can take infinite number of load cycles; this limit is defined as endurance level or fatigue endurance limit.

(b) Attrition of concrete:

It increases the shear strength of structural concrete by providing a more homogeneous mix.

It reduces the water (and hence the cement) content in ready-mixed concrete for a given structural strength, thereby reducing the cost per cubic metre.

It increases the skid-resistance of asphalt wearing courses by presenting more angular faces to the tyre than a series of very flat particles.

It helps to reduce aquaplaning in wet weather when used for asphalt wearing courses because it reduces the amount of flat surfaces in contact with the tyre.

Erosion in concrete:

Erosion is the deterioration of **concrete** surface as a result of particles in moving water scrubbing the surface.

(c) steps to improve bond strength in reinforcement concrete:

Strength of bond depends upon the friction between steel and concrete.

.The bond strength increase with increase in compressive

strength of concrete.

.Deformed (ribbed) bars should be used to increase friction

between reinforcement and concrete.

4. What is creep? What are the factors affecting creep? What difference is between creep and strain relaxation?

Ans. Creep:

_____The increase in strain of concrete with in passage of

time under sustained stress is known as creep.

Factors effecting creep:

. Stifer the aggregate lower the creep. More the content

of aggregate per unit volume of concrete, lower the

creep.

. Decrease in W/C causes decrease in creep. In other words strength and creep and inversely proportional.

. Creep is smaller when concrete is cured at high

temperature because strength is higher than when

cured and loaded at high temperature.

. Creep also depends upon the applied stress. The

relationship is directly proportional.

. Creep also depends on the type of cement. High

alumina cement experiences less creep as compared to

Ordinary Portland Cement.

Difference b/w creep & strain relaxation:

two terms are sometimes used interchangeably, although they are really **different**. **Creep** is an increase in plastic strain under constant **stress**. **Strain relaxation** is a decrease in **stress** under constant strain. **Creep** is an increased tendency toward more strain and plastic deformation with no change in **stress**.

5. What is the difference between drying shrinkage and plastic shrinkage? Is drying and plastic shrinkage reversible?

Ans. Drying shrinkage:

Drying shrinkage results from the loss of capillary water from the hardened cement mixture, leading to contraction and crack formation within concrete. According to the previous studies, the addition of plant fibbers, such as sisal, to cement mortar increases its **drying shrinkage**.

Plastic shrinkage:

Plastic shrinkage is caused by the loss of water by evaporation from the surface of newly laid concrete or by suction of dry concrete underneath. At the surface, **plastic shrinkage** occurs when the rate of evaporation exceeds the rate of bleeding.

Reversible or not:

A part of drying shrinkage is reversible

through moisture movement (40 to 70%).

While plastic shrinkage is not reversible at all.

6.(a) What are risks to concrete structure exposed to sea water? How do you increase resistance of concrete to sea water? (b) Concrete is required for the internal columns of a building. The specified

28 days strength is (x+y+15) MPa (where x and y are the last two digits of your Roll no.) The following equations may be used to find average compressive strength The slump required is 50mm and a maximum size of 25mm is required. The fine aggregate has a Fineness modulus of 2.60. Preliminary tests indicate that FA and CA have a specific gravity of 2.65 and 2.7, with 1 percent absorption in C.A and 2% free moisture in F.A. The Bulk density of C.A is 1600 kg/m³. Find the required quantities of ingredients.

Ans: (a)Risks :

<u>1.</u> The constituents of seawater reacts chemically with constituents of cement concrete which results damage to the concrete structure in several ways. The magnesium sulfate present in seawater reacts with calcium hydroxide of cement and forms calcium sulfate as well as magnesium hydroxide precipitation.

2.Magnesium sulfate also reacts with hydrated calcium alumnat and forms calcium sulpho aluminate. These final formations are the primary reasons for chemical attak on concrete structures.

3. The deterioration of concrete structures by seawater is more due to leaching rather than expansion of concrete. Leaching more effects the small concrete structures than expansion while large concrete structures are effected by leaching as well as expansion.

Resistance of concrete to seasewater:

- 1. Cement with low C₃A content should be preferable to make concrete.
- 2. Prepare rich concrete with low water cement ratio which makes the concrete impervious. Then the pores in concrete are very small and they cannot hold seawater results in the prevention of expansion by freezing of water and crystallization of salt in the pores.

- 3. The concrete is of low water cement ratio. To make it workable for construction, Water reducing admixtures can be added to the concrete which is recommended by ACI 318 and ACI 357.
- 4. The admixtures should not contain chloride in any form otherwise corrosion of reinforcement takes place.
- 5. Adequate cover should be provided for reinforcement in concrete structure to enhance durability.

<u>(b)</u>

0 Dig bulk volume of CA For Somm with 2.60 FM ≥ 0.76 Average compressive strength where my Roff number is A 1111100 07 \$ (3+2+15) 20 MRa Cuphere is Pm=20+1.64 x4 => 4 Por M20 Connect Pm = 26.56 Now according to formulae Standard dividuisn not given Now For 20 Mapa concrete Pm = Fmin 16.9 pm = 20 16.9 = 2.6.9 Mpg & where is we refis for 20mpa = 0.60 Now the Slump value is Somu and maximum Size aggregate is 25mm for non-air-entrained (our refe. maximum water ration = 180 given in table and manum entroped air=1:5 A So required concrete content-180=300 0 <

2) a marinum Size of \$ now aggregate is 25mm and Finanesi moduly 2.60 C/ bulle -the dry volume Course aggregate the 01S 0.76 per unit volume Concrete Ol. weight of C.A = 0-71x1000 & there por The =)1.218 kg/m2 Prom table 11.9 sise aggregate=2275 kg/ 25 mm a mak hor non civ. hor non air Concrete is @ lit is nest mentioned that concrete is criventraned are are non aly-entrancel Moxal:all KNOWN weisht of ingrediente Water = 180 IN (pm.eut) weight Content 01 = 300 weight 01 (A= 1216 Now 75-(180+300+1216 23 = 679 kg /m3 absolute 00 that (olune FA = (1000-697)×103 al = 303 × 103 wgnt F.A = 303×2-65 191 803 1 Kg/m3 Adopt F.A a 803 Kg/m3 2/4 ||| < \bigcirc

3 quantity per cubic Now estimate maderi al 1-0V matter Concrete Ol = 300 Kg/m3 emment F.A = 803 , Kg 1 m3 1m3 C.A = 12.16 Kg 180 Kg/m3 water = 2499 10g/m3 max size Where 2375 For 25mm aggregate. Now Coarge aggrefic Fine Waleement (oc 803 300 180/ 1216 300 1300 300 = 2.677 4.05 0.6 meetericel So weisit 06 bag mix in leg = SG 60r one Althe Finj agsiegte Coment (orsse aggreste Water 50 Sox 2.677 Sox4.05 50. 10.6 202.5 133.85kg Jolifre 109 3/4 ||| \bigcirc <

· 🖉 Y EA do have 2% moisture 2/ x803 = 16.06 Kg/m3 \$ 12 7100 F.A Peild Contain = 803+16005819 Weight 01 GA Cubsorption 03 Present A 2.1 where is x 1210 = 12.16 = [100 feild Condition C.A weight 01 1216-12.16 = 1203 . Ky Kg /m3 regard to with now F.A by 16.06 water C.A by and 12.16 pore 16.06 - 12.16 There = 3. 9 19 quantity 3 So 5.91 kg be deducter will water water 10 600m to water 180 - 3.91 = 176. (1 kg/m3 176 ·11= 176 Kg/m3 of material to be Quandity Peole Conditution ind 1.red Converst = 300 BIG F.A = C.A = 1203 - 841 Weller= 176 Concrete \$ 2468. DY of each Feild density 4/4 < \bigcirc