

NAME :- MALIK Afnan

ID :- 7839

Section :- "B"

Semester :- "Six"

Department :- Civil Engineering

Subject :- Hydraulic Engineering

Q No 1
(A)Given data

$$\text{Discharge (Q)} = 7839 \text{ lit/sec}$$

$$Q = \frac{7839}{1000} = 7.839 \text{ m}^3/\text{sec}$$

$$\text{Breadth (b)} = 8 \text{ m}$$

$$\text{Mean velocity (v)} = 7839 - 220$$

$$v = 7619 \text{ ft/sec}$$

$$v_1 = 2322.86 \text{ m/sec}$$

1) AS we know that

$$Q = q \cdot b$$

$$q = \frac{Q}{b} \text{ Putting value.}$$

$$q = \frac{7.839}{8}$$

$$q = 0.9798 \text{ m}^2/\text{Sec}$$

$$\rightarrow y_c = \left(\frac{q^2}{g} \right)^{1/3}$$

Putting value.

$$y_c = \left[\frac{(0.9798)^2}{9.81} \right]^{1/3}$$

$$y_c = \left[\frac{0.96000}{9.81} \right]^{1/3}$$

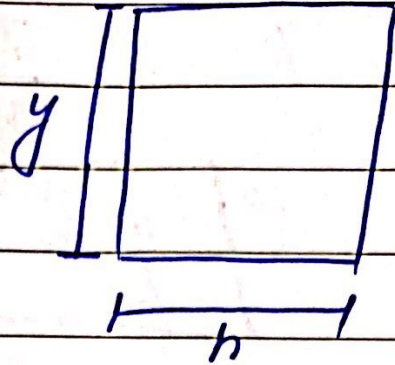
$$y_c = \left[0.09785 \right]^{1/3}$$

$$y_c = 0.46084$$

As it is Rectangular Section

$$Q = qb \text{ --- (1)}$$

$$Q = Av \text{ --- (2)}$$



Equation (1) And (2)

$$qb = Av$$

$$q/b = y/v$$

$$q = yv$$

$$V_c = \frac{q}{y_c} = \frac{0.9798}{0.46084} = 2.1261$$

$\therefore V > V_c$ (Super Critical flow)
Height of hydraulic jump on the upstream side

As $Q = Av$

$$Q = byv$$

$$y_1 = \frac{Q}{v_1 b}$$

Putting value.

$$y_1 = \frac{7.839}{2322.86 \times 8} = \frac{7.839}{18582.88}$$

$$y_1 = 0.000421 \text{ m}$$

$$y_2 = \frac{-y_1}{2} + \sqrt{\frac{y_1^2}{4} + \frac{2y_1(v_1)^2}{g}}$$

$$y_2 = \frac{-0.000421}{2} + \sqrt{\frac{(0.000421)^2}{4} + \frac{2(0.000421)(2322.86)}{9.81}}$$

$$y_2 = 0.446 \text{ m}$$

$$\Delta y = y_2 - y_1$$

$$\Delta y = 0.446 - 0.000421 =$$

$$\Delta y = 0.441 \text{ m}$$

$$ii) :- \Delta E = E_1 - E_2$$

As we know that

$$Q_1 = Q_2$$

$$A_1 V_1 = A_2 V_2$$

$$\cancel{y_1} V_1 = \cancel{y_2} V_2 \quad : \quad b_1 - b_2 = b$$

$$V_2 = y_1 V_1 / y_2$$

$$V_2 = \frac{0.000421 \times 2322.86}{0.446} =$$

$$V_2 = 2.7926 \text{ m/sec}$$

$$\Delta E = E_1 - E_2 = \left(y_1 + \frac{V_1^2}{2g} \right) - \left(y_2 + \frac{V_2^2}{2g} \right)$$

$$\Delta E = 0.000421 + \left(\frac{(2322.86)^2}{2g} \right) - \left(0.446 + \frac{(2.7926)^2}{2g} \right)$$

$$\Delta E = 0.000421 + 275009.0 - 0.446 + 27500.90$$

$$\Delta E = 0.000421 + 550017.5$$

$$\Delta E = 550017.5$$

→ Power absorbed

$$\Delta P = \rho g Q (E_1 - E_2)$$

putting value

$$\Delta P = 1000 \times 9.8 \times 7839 (550017.5)$$

$$\Delta P = 44253.5 \text{ m}$$

Qno1
(B)

Given data

$$b = 4 \text{ m}$$

$$Q = 7839 \text{ ft}^3/\text{sec} = \frac{7839}{(3.28)^2} =$$

$$\frac{7839}{35.2875} = 222.14 \text{ m}^3/\text{sec}$$

$$y_1 = 2.9 \text{ m}$$

$$y_2 = 1.1 \text{ m}$$

Let specific energy at upstream
and down stream side

$$E_1 = E_2$$

$$y_1 + \frac{v_1^2}{2g} = y_2 + \frac{v_2^2}{2g} \quad \text{--- (1)}$$

As we know that

$$Q = A_1 v_1 = A_2 v_2$$

$$b y_1 v_1 = b y_2 v_2 \quad \therefore b_2 = b_1 = b$$

$$v_2 = \frac{y_1 v_1}{y_2}$$

putting value.

$$v_2 = \frac{2.9}{1.1} v_1$$

$$\boxed{v_2 = 2.634 v_1} \quad \text{--- (2)}$$

Putt the value of Eq (2) in Eq (1)

$$2.9 + \frac{v_1^2}{2 \times 9.81} = \frac{1.1 + (2.634 v_1)^2}{2 \times 9.81}$$

$$2.9 - 1.1 = \frac{6.938 v_1^2}{19.62} - \frac{v_1^2}{19.62}$$

$$1.8 = \frac{6.938v_1^2 - v_1^2}{19.62}$$

$$1.8 \times 19.62 = 6.938v_1^2$$

$$v_1^2 = \sqrt{\frac{1.8 \times 19.62}{5.938}}$$

$$v_1^2 = \sqrt{\frac{35.316}{5.938}}$$

$$\sqrt{v_1^2} = \sqrt{5.9474}$$

$$v_1 = 2.44 \text{ m/sec}$$

Now we put the value of "v₁" in Eq (1).

$$y_1 + \frac{v_1^2}{2g} = y_2 + \frac{v_2^2}{2g}$$

$$2.9 + \frac{(2.44)^2}{19.62} = 1.1 + \frac{v_2^2}{19.62}$$

Putting value

$$2.9 - 1.1 = \frac{v_2^2}{2g} - \frac{5.95}{2g}$$

$$1.8 = \frac{v_2^2 - 5.95}{2g}$$

$$1.8 \times 2g = v_2^2 - 5.95$$

$$1.8 \times 2 \times 9.81 = v_2^2 - 5.95$$

$$\sqrt{v_2^2} = \sqrt{41.266}$$

$$v_2 = 6.42 \text{ m/sec}$$

Using Froude No to determine type of flow

UP Stream Side:-

$$Fr_1 = \frac{v_1}{\sqrt{gy_1}} = \frac{2.44}{\sqrt{9.81 \times 2.9}}$$

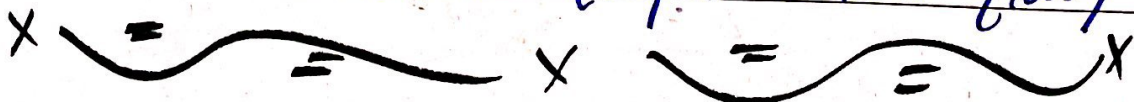
$$= 0.457 < 1$$

(sub critical flow)

Down Stream:-

$$Fr_2 = \frac{v_2}{\sqrt{gy_2}} = \frac{6.42}{\sqrt{9.81 \times 1.1}}$$

$$= 1.95 > 1 \text{ (super critical flow)}$$



Q No 2
(A)Given data:-

$$y = 1.8 \text{ m}$$

$$b = 66' = \frac{66}{3.28} = 20.12 \text{ m}$$

$$Q = \frac{7839}{(3.28)^3} = 222.14 \text{ m}^3/\text{sec}$$

Required data:-

Minimum height (P) of weir

$$Q = Av$$

$$v = \frac{Q}{A} = \frac{Q}{by} = \frac{222.14}{20.12 \times 1.8}$$

$$v = \frac{222.14}{36.216} = 6.14 \text{ m/Sec}$$

As we know that

$$y_c = \left(\frac{q^2}{g} \right)^{1/3} = \left(\frac{(11.04)^2}{9.81} \right)^{1/3}$$

$$\therefore q = \frac{Q}{b} = \frac{222.14}{20.12}$$

$$q = 11.04$$

$$y_c = \left(\frac{(11.04)^2}{9.81} \right)^{1/3}$$

$$y_c = \left(\frac{121.88}{9.81} \right)^{1/3}$$

$$y_c = (12.424)^{1/3}$$

$$y_c = 2.31 \text{ m}$$

Also $v = \sqrt{gy}$

$$v_c = \sqrt{9.81 \times 2.31}$$

$$v_c = 4.77 \text{ m/sec}$$

Now according to Specific energy.

$$E_1 = E_2$$

$$y_1 + \frac{v_1^2}{2g} = \frac{v_c^2}{2g} + y_c + P$$

$$1.8 + \frac{(6.14)^2}{2g} = \frac{(4.77)^2}{2g} + (2.31) + P$$

$$P = 3.720 - 3.46$$

$$P = 0.26 \text{ m}$$

Q No 2
(B)

Given data:-

$$b = 2.8 \text{ m}$$

$$d = 1.5 \text{ m}$$

$$H_1 = 5 \text{ m}$$

$$H_2 = 5 + 1.5 = 6.5 \text{ m}$$

$$H = 5 + 0.6 = 5.6 \text{ m}$$

$$C_d = 0.7839$$

Required data:-

$$Q = ?$$

Discharge through submerged portion

$$Q_1 = C_d \times b \times (H_2 - H) \times \sqrt{2gH}$$

Putting value.

$$Q_1 = 0.7839 \times 2.8 \times (6.5 - 5.6) \times \sqrt{2 \times 9.81 \times 5.6}$$

$$Q_1 = 2.1949 \times (0.9) \times \sqrt{109.872}$$

$$Q_1 = 1.97541 \times 10.481$$

$$Q_1 = 20.70 \text{ m}^3/\text{sec}$$

$$Q_1 = 20.70 \text{ m}^3/\text{sec}$$

Discharge of free portion:-

$$Q_2 = \frac{2}{3} cd \times b \sqrt{2g} [H^{3/2} - H_1^{3/2}]$$

$$Q_2 = \frac{2}{3} (0.7839) \times 2.8 \sqrt{2(9.81)} [(5.6)^{3/2} - 5^{3/2}]$$

$$Q_2 = 0.666 (2.1949) \sqrt{19.62} [5.6^{3/2} - 5^{3/2}]$$

$$Q_2 = 1.4618 \sqrt{19.62} [5.6^{3/2} - 5^{3/2}]$$

$$Q_2 = 1.4618 \times 4.4294 [5.6^{3/2} - 5^{3/2}]$$

$$Q_2 = 6.4748 [5.6^{3/2} - 5^{3/2}]$$

$$Q_2 = 6.4748 [5.6^{1.5} - 5^{1.5}]$$

$$Q_2 = 6.4748 [13.25 - 11.18]$$

$$Q_2 = (6.4748) (2.07)$$

$$Q_2 = Q_2 = 13.402$$

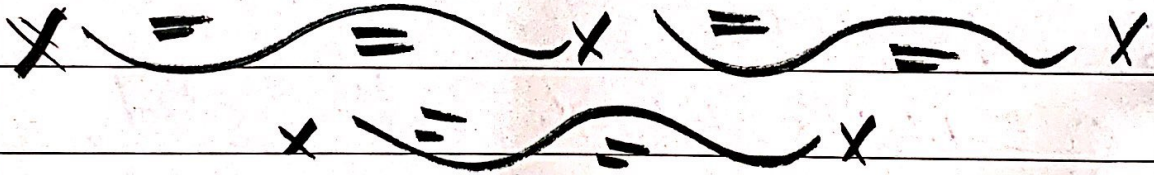
Total Discharge

$$Q = Q_1 + Q_2$$

Putting value.

$$Q = 20.70 + 13.402$$

$$Q = 34.102$$



Q No 3
(A)

Given data:-

$$P_1 = R + 800 = 7839 + 800 = 8639 \text{ N/m}^2$$

$$d_1 = R - 200 = 7839 - 200 = 7639 \text{ mm} = 7.639 \text{ m}$$

$$A_1 = \frac{\pi}{4} d_1^2 = \frac{\pi}{4} (7.639)^2 = 45.80 \text{ m}^2$$

$$d_2 = R + 3000 = 7839 + 3000 = 10839 \text{ mm}$$

$$d_2 = 10.839 \text{ m}$$

$$A_2 = \frac{\pi}{4} d_2^2 = \frac{\pi}{4} (10.839)^2 = 92.221$$

$$A_1 = 92.221$$

$$Q = 0.95 \text{ m}^3/\text{sec}$$

$$Q = AV$$

$$V = \frac{Q}{A_1}$$

$$v_1 = \frac{Q_1}{A_1} = \frac{0.95}{45.80} = \boxed{0.020 \text{ m/sec}}$$

$$v_2 = \frac{Q_2}{A_2} = \frac{0.95}{92.221} = \boxed{0.010 \text{ m/sec}}$$

1) Head loss due to Sudden enlargement.

$$h_e = \left(1 + \frac{A_1}{A_2}\right)^2 \frac{[v_1 - v_2]^2}{2g} = \left(\frac{1 - \frac{45.80}{92.221}}{1}\right)^2 \times$$

$$\frac{[0.020 - 0.010]^2}{2 \times 9.81}$$

$$h_e = \frac{(92.221 - 45.80)^2}{92.221} \times \frac{(0.01)^2}{19.62}$$

$$h_e = (0.5033)^2 \times \frac{0.0001}{19.62}$$

$$h_e = 0.2533 \times 5 \times 10^{-6}$$

$$h_e = 1.26 \times 10^{-6}$$

$$\boxed{h_e = 0.00000126 \text{ m}}$$

2) Power lost due to Sudden enlargement.

$$P = \rho g Q h_e$$

$$P = 1.00 \times 9.81 \times 0.95 \times 1.26 \times 10^{-6}$$

$$P = 9.319 \times 1.26 \times 10^{-6}$$

$$P = 0.0011 \text{ W}$$

(3) Pressure is the smallest pipe.

Apply Bernoulli's equation

$$\frac{P_1}{\rho g} + \frac{v_1^2}{2g} = \frac{P_2}{\rho g} + \frac{v_2^2}{2g} + h_e$$

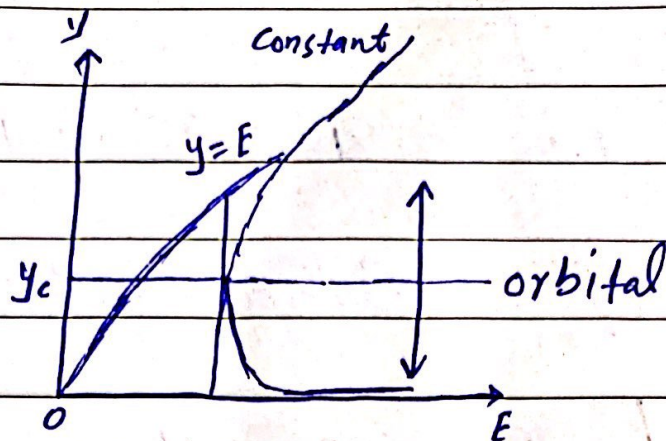
$$\frac{8639}{1000 \times 9.81} + \frac{(0.020)^2}{2(9.81)} = \frac{P_2}{(1000 \times 9.81)} + \frac{(0.010)^2}{2(9.81)}$$

$$+ 1.26 \times 10^{-6}$$

$$P_2 = 8580.78 \text{ N/m}^2$$

Q No 3

(B)

Ans. -

The above graph is plot between depth flow (y) and specific energy (E). It is made from three degree polynomial equation which shown us the different specific energy for the depth flow which may be either:

- i) Sub critical
- ii) Critical
- iii) Super critical

Specific energy is used to clarify the meaning the above terms in an open channel. How is this achieved?

Total energy = Potential energy + Kinetic energy

$$T.E = mgh + \frac{1}{2}mv^2$$

$$\therefore w = mg$$

$$m = \frac{w}{g}$$

$$= wh + \frac{1}{2} \frac{w}{g} v^2$$

ignoring "IN" weight of water

$$T.E = y + \frac{v^2}{2g} \quad \text{--- (1)}$$

As we know that

$$Q = VA$$

$$v = \frac{Q}{A}$$

$$v^2 = \frac{Q^2}{A^2}$$

Squaring on both sides

Put v^2 in Eq (1)

$$E = y + \frac{Q^2}{A^2 g}$$

Let suppose the Channel is Rectangular

$$A = y \times b \quad \text{--- (2)}$$

$$Q = q \times b \quad \text{--- (3)}$$

Putting value of "x" and "y" in Eq (2)

$$E = y + \frac{Q^2}{y^3 b^2 2g} \rightarrow \text{Putting (2)}$$

$$E = y + \frac{q^2}{y^3 2g} \rightarrow \text{Putting (4)}$$

$$E - y = \frac{q^2}{y^3 2g}$$

$$(E - y)y^3 = \frac{q^2}{2g}$$

$$(E - y)y^3 = \text{Constant}$$

As "q" and "g" are constants

* Critical depth is the flow depth corresponding to minimum specific energy.

$y > y_c \Rightarrow$ Sub critical flow.

$y = y_c \Rightarrow$ Critical flow.

$y < y_c \Rightarrow$ Super critical flow.

