

SUBJECT: STRUCTURAL ANALYSIS I

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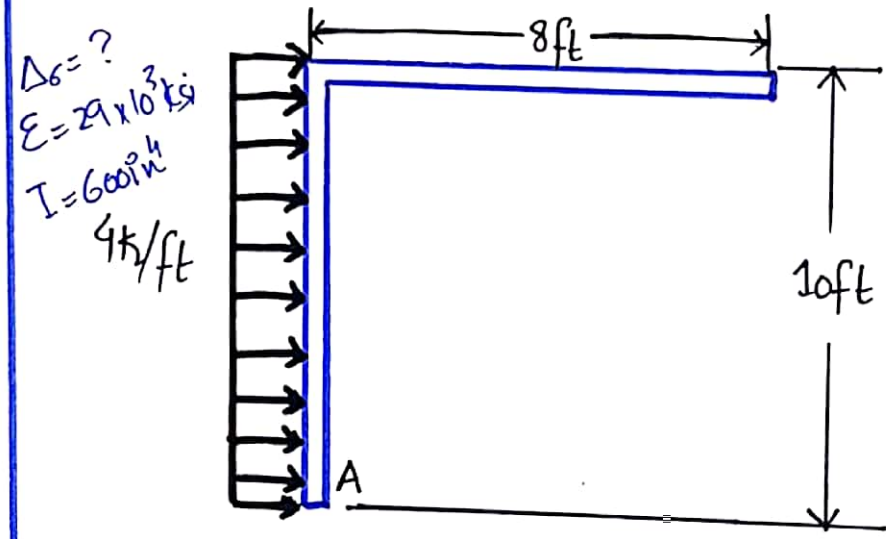
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SECTION:B

QUESTION No-1

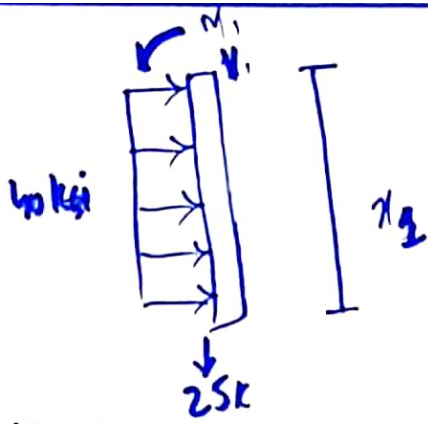
Determine the vertical displacement of free end point C on the frame shown in figure. Take $E = 29(10^3)$ ksi and $I = 600 \text{ in}^4$ for both members. Use method of Virtual Work.



Solution:

Finding reactions

$$\begin{aligned}
\sum M_A &= 0 \\
-4(10)(5) + A_y(8) &= 0 \\
A_y &= 25 \text{ k} \\
\sum F_y &= 0 \uparrow \\
25 + A_y &= 0 \\
A_y &= -25 \text{ k} \\
\sum F_x &= 0 \rightarrow \\
40 - A_x &= 0 \Rightarrow A_x = 40 \text{ k}
\end{aligned}$$

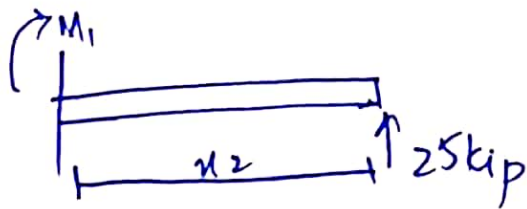


⇒ **Rect.** moments

$$\sum^+ M_1 = 0$$

$$-40(x_1) + 4(10)\left(\frac{x_1}{2}\right) + M_1 = 0$$

$$M_1 = 40x_1 - 20x_1^2$$

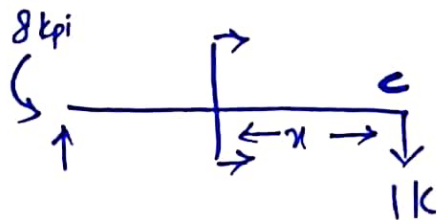
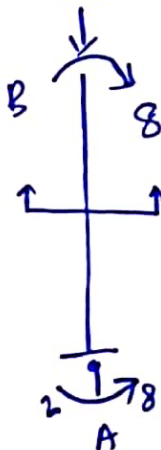


$$-25(x_2) + M_2 = 0$$

$$M_2 = 25x_2 \text{ kip}$$

Now

Vertical Moments



Members	BA	CB
Origin	B	C
limit	0-10	0-8
M	$2x^2$	0
m	8	x

By virtual working

$$1. \Delta_c = \int_0^{10} \frac{(2x^2)(8)}{EI} dx + \int_0^8 \frac{(0)(x)}{EI}$$

$$= \frac{16x^3}{3} \Big|_0^{10} + 0$$

$$= \frac{16 \times 1000}{3} / EI$$

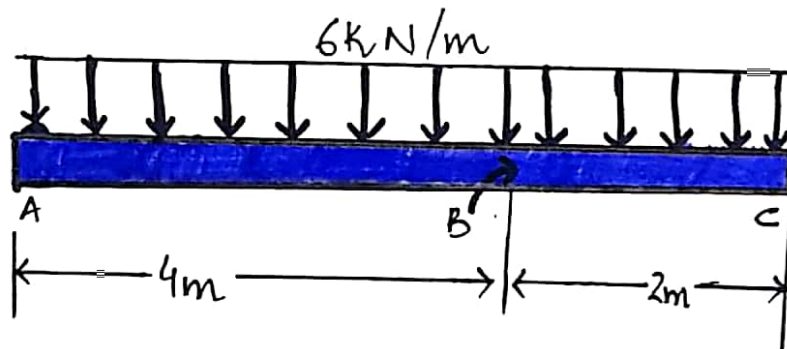
$$= \frac{5333.33}{EI} = \frac{533.33}{29 \times 10^3 \times 600}$$

$$= 3.06 \times 10^{-4} \text{ in}$$

QUESTION No.2

Determine the slope and displacement at point B. Assume the support at A is a pin and C is a roller. Take $E = 200 \text{ GPa}$, $I = 60(10^6) \text{ mm}^4$. Use Castigliano's Theorem.

Figure:



SOLUTION:

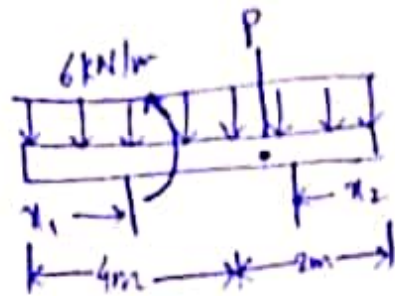
Given Data

$$E = 200 \text{ GPa}$$

$$I = 60(10^6) \text{ mm}^4$$

Required:

Slope and displacement = ?



$$R_1 + R_2 = 0 \quad \text{--- (1)}$$

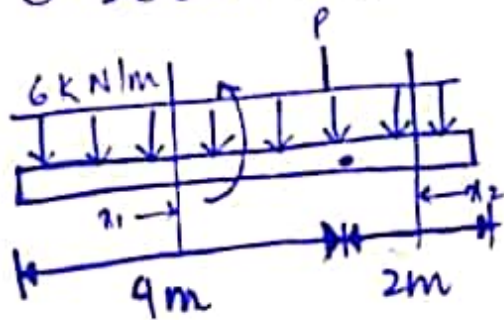
$$\sum M_A = 0 \quad \curvearrow +$$

$$1 + R_2(6) = 0$$

$$\Rightarrow -0.16667 \quad \text{put in (1)}$$

$$R_1 + (-0.16667) = 0$$

$$R_1 = 0.16667 \text{ KN}$$



$$R_1 + R_2 = 1$$

$$\curvearrow + \sum M_A = 0$$

$$- (1)(4) + R_2(6) = 0$$

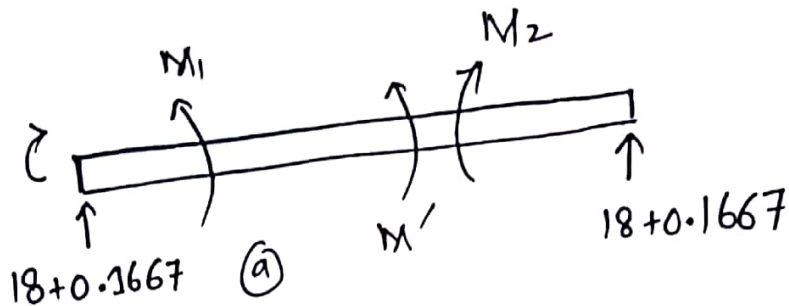
$$R_1 = 0.6667 \text{ KN}$$

$$R_2 = 1 - 0.6667 \text{ KN}$$

$$R_2 = 0.333 \text{ KN}$$

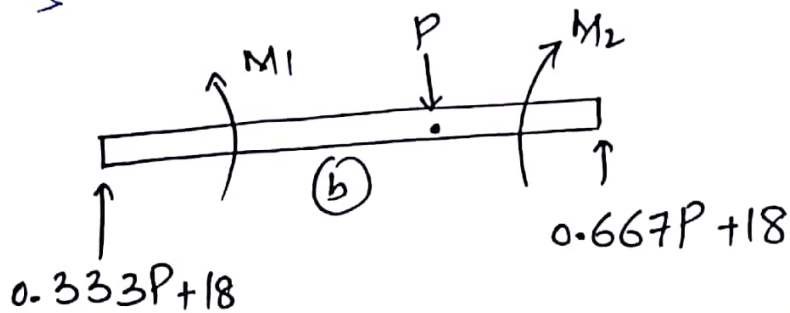
$$M_1 = (18 + 0.1667 M') x_1 - 2x_1^2$$

$$M_2 = (18 - 0.1667 M') x_2 - 2x_2^2$$



$$M_1 = (0.333 P + 18) x_1 - 2x_1^2$$

$$M_2 = (0.667 P + 18) x_2 - 2x_2^2$$



The displacement functions shown in the fig. a above

$$\frac{\partial M_1}{\partial M'} = 0.1667 x_1 \quad \text{and} \quad \frac{\partial M_2}{\partial M'} = 0.1667 x_2, \quad \text{set } M' = 0 \text{ then}$$

$$M_1 = [18 + 0.1667(0)] x_1 - 2x_1^2$$

$$\Rightarrow M_1 = (18x_1 - 2x_1^2)$$

$$\Rightarrow M_2 = (18x_2 - 2x_2^2)$$

$$\phi_B = \int_0^L M \left(\frac{\partial M}{\partial M'} \right) \frac{dx}{Ei} = \int_0^L \frac{(18x_1 - 2x_1^2)(0.1667x_1)}{Ei} dx_1 + \int_0^L \frac{(18x_2 - 2x_2^2)(0.1667x_2)}{Ei} dx_2$$

$$\phi_B = \frac{42.65}{Ei} + \frac{6.66}{Ei}$$

$$\phi_B = \frac{49.31}{Ei}$$

$$\phi_B = \frac{49.31}{(200 \times 10^6 \text{ kPa})(0.0006)} = 0$$

$$\boxed{\phi_B = 0.4411 \text{ rad}}$$

→ For the displacement functions are shown in figure "6"

$$\frac{\partial M_1}{\partial P} = 0.333 \pi_1 \quad \text{and} \quad \frac{\partial M_2}{\partial P} = 0.6667 \pi_2 \quad \text{also}$$

$$\text{Set } P = 0$$

$$\text{then } M_1 = (18 \pi_1 - 2 \pi_1^2) \text{ KN}\cdot\text{m}$$

$$M_2 = (18 \pi_2 - 2 \pi_2^2) \text{ KN}\cdot\text{m}$$

thus

$$\Delta_B = \int_0^L M \left(\frac{\partial M}{\partial P} \right) \frac{dx}{Ei}$$

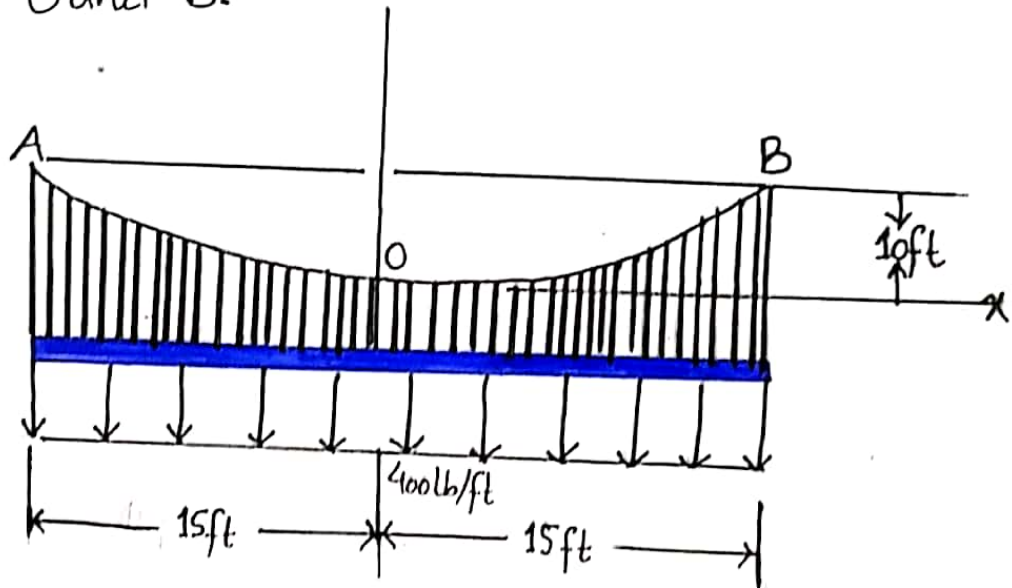
$$\Delta_B = \int_0^1 \frac{(30 \pi_1 - 2 \pi_1^2)(0.333 \pi_1)}{Ei} dx + \int_0^2 \frac{(30 \pi_2 - 2 \pi_2^2)(0.6667 \pi_2)}{Ei} dx$$

$$\Delta_B = \frac{218.5}{Ei} = \frac{218.5}{(200 \times 10^6)(0.0006)} = \boxed{0.018 \text{ m} \Rightarrow 18 \text{ mm} \downarrow}$$

QUESTION No.3

The cable is subjected to the uniform loading. If slope of the cable at point O is zero, determine the equation of the curve and the force in the cable at O and B.

Figure



Solution:

From Eq. 5-9

$$y = \frac{w}{L^2} x^2 = \frac{400}{15^2} x^2$$

$$y = 0.0444 x^2$$

From Eq. 5-8

$$T_0 = F_u = \frac{w_0 L^2}{2h} = \frac{400(15^2)}{2(10)}$$

$$T_0 = 4500 \text{ lb} \quad \div \text{ing by } 1000$$

$$T_0 = 4.5 \text{ K}$$

From Eq. 5-10

$$T_B = T_{\max} = \sqrt{F_u^2 + (w_0 L)^2}$$
$$= \sqrt{(4500)^2 + (400)^2 (15)^2}$$

$$T_B = T_{max} = \sqrt{20250000 + (400 \times 15)^2}$$

$$T_B = T_{max} = 7500 \text{ lb} \quad \div \text{ing by } 1000$$

$$T_B = T_{max} = 7.5 \text{ k}$$

Also From Eq. 5-11

$$T_B = T_{max} = w_0 L \sqrt{1 + \left(\frac{L}{2h}\right)^2}$$

$$T_{max} = (400)(15) \sqrt{1 + \left(\frac{15}{20}\right)^2}$$

$$T_{max} = 6000 \sqrt{1 + \frac{225}{400}}$$

$$T_{max} = 6000 (1.25)$$

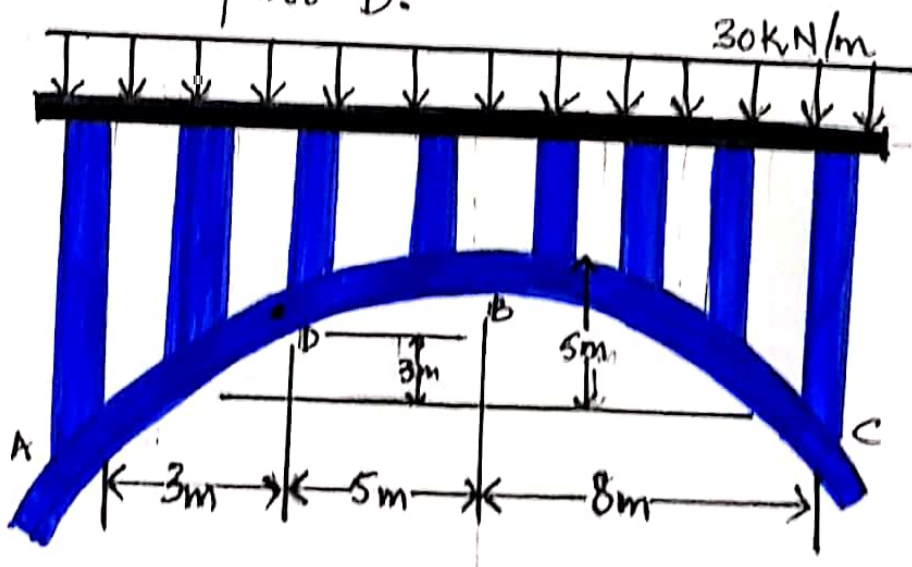
$$T_{max} = 7500 \text{ lb} \quad \div \text{ing by } 1000$$

$$T_B = T_{max} = 7.5 \text{ k}$$

QUESTION No.4

The three hinged spandrel arch is subjected to the uniform load of 30 kN/m . Determine internal moment arch at point D.

Figure:



SOLUTION

Member AB;

$$\curvearrowright + \sum M_A = 0$$

$$\Rightarrow B_x(5) + B_y(8) - 240(4) = 0$$

Member BC;

$$\curvearrowright + \sum M_C = 0$$

$$\Rightarrow -B_x(5) + B_y(8) + 240(4) = 0$$

Solving

$$B_x = 192 \text{ kN}, B_y = 0$$

Segment BD:

$$\curvearrowright + \sum M_D = 0$$

$$\Rightarrow 192(2) - 150(2.5) - M_D = 0$$

$$M_D = 9 \text{ kN}\cdot\text{m}$$