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ID: 7395 Sec: A

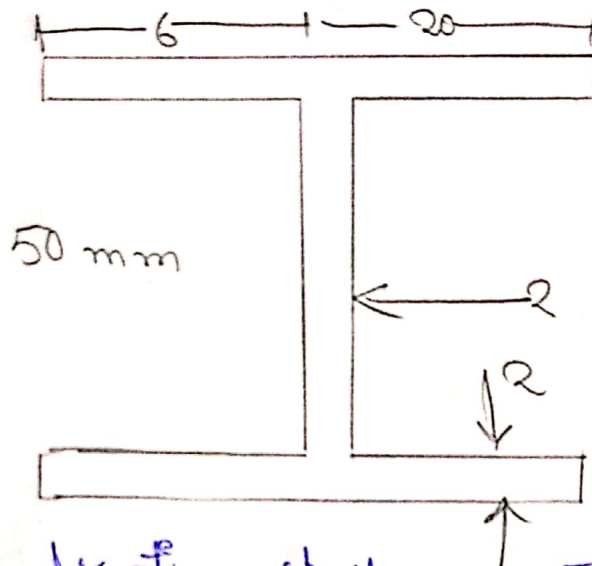
SUBJECT: M0S-II

INSTRUCTOR:

Engr M. Saqib.

DATE: 23rd June 2020.

Question No (1)(a) :



Required location of shear centre :

Sol: As we know .

$$e = \frac{t_f h^2 b^2}{4I}$$

and

$$I = 2 \left(\frac{bh^3}{12} + Ay^2 \right) + \left[\frac{bh^3}{12} + Ay^2 \right]$$

$$= 2 \left[\frac{26(2)^3}{12} + (20 \times 2)(25)^2 \right] + \left[\frac{2(50)^3}{12} + 0 \right]$$

$$I = 50034.66 + 20833$$

$$I = 70867.99 \text{ mm}^4$$

$$e = \frac{2(50)^2(25)^2}{4(70867.99)} = 11.02 \text{ mm}$$

So shear centre = $e = 11.02 \text{ mm}$

Question No 1(b):

DATA:

$$H = 26 \text{ ft}$$

⇒ I assume diameter

$$D = 22 \text{ ft}$$

$$\Rightarrow \text{Tangential stress} = 6000 \text{ lb/ft}^3$$

$$\Rightarrow \text{Specific weight of water tank} = 62.4 \text{ lb/ft}^3$$

We have to find the thickness = ?

Solution:

The pressure develops by water = $P = \gamma h$

$$S_t = \frac{PD}{2t}$$

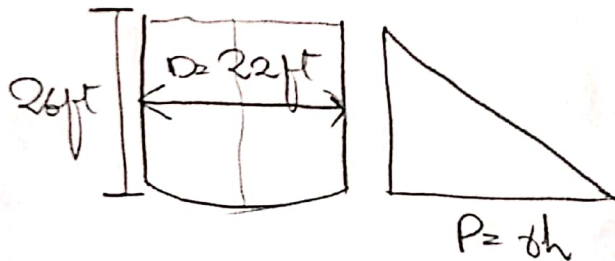
$$S_t = \frac{PD}{2t} = \frac{\gamma h D}{2t}$$

$$2t \times S_t = \gamma h D$$

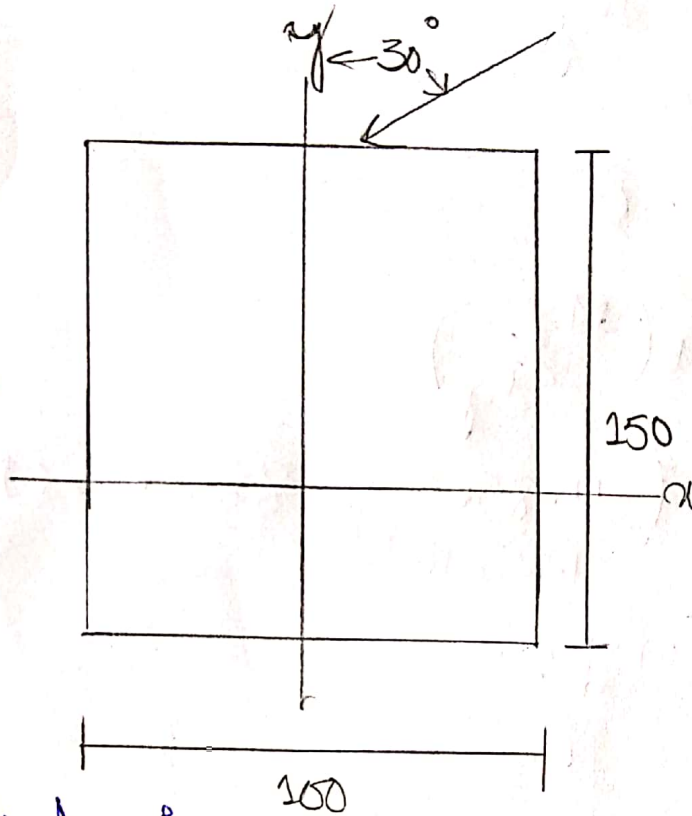
$$2t = \frac{\gamma h D}{S_t}$$

$$t = \frac{\gamma h D}{S_t \times 2}$$

$$t = \frac{(62.4) \times (26 \times 12) \times (22 \times 12)}{(6000 \times 2)} = t = 0.24 \text{ in.}$$



Question No 2(a):



Moment of Inertia:

$$I_z = \frac{bh^3}{12} = \frac{0.1(0.15)^3}{12} = I_z = 2.8125 \times 10^{-5}$$

Now

$$I_y = \frac{hb^3}{12} = \frac{0.15(0.1)^3}{12}$$

$$I_y = 1.25 \times 10^{-5}$$

$$I = Mz \frac{y}{I_z} + My \frac{z}{I_y}$$

$$I = M \cos \theta \frac{z}{I_z} + M \sin \theta \frac{y}{I_y}$$

$$\text{where } My = M \cos \theta = P \cos \theta = Mz$$

$$= 12 \cos 30^\circ Mz$$

$$M_z = 1.8510$$

$$M \sin \theta = P \sin \theta = M_y$$

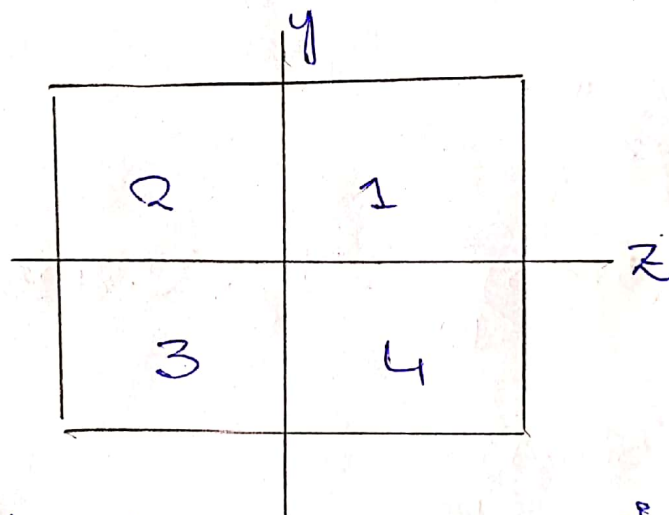
$$M_y = 12 \sin 30$$

$$M_y = -11.8563$$

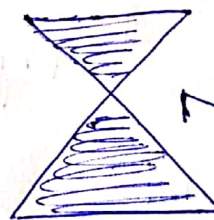
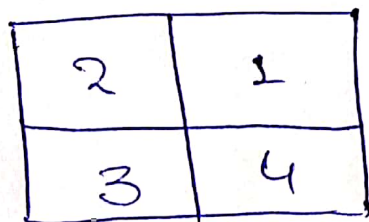
$$\sigma = \left(\frac{M_z}{I_z} \right) + \left(\frac{M_y}{I_y} \right)$$

$$\sigma = \frac{1.851}{2.812 \times 10^{-5}} + \left(\frac{-11.8563}{1.25 \times 10^{-5}} \right) = 882628 \text{ Nm}^{-2}$$

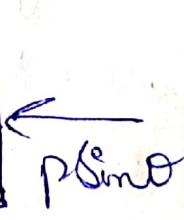
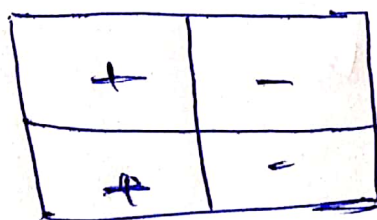
Sign Conventions:



If we take compression as negative and tension as positive and the beam is a simply supported



Quadrant 1, 2 -ve
Quadrant 3, 4 +ve



Quadr 1, 4 -ve
Quadr 2, 3 +ve

In case of unsymmetrical loading. The neutral axis lies at an angle of ' α '. The principal axis and the algebraic sum of stress at N.A. is zero.

$$f = \frac{M \cos \theta}{I_z} y + \frac{M \sin \theta}{I_y} z \quad \text{--- (1)}$$

in this case N.A. passes through 2, 4, 130.

$$f = \frac{M \cos \theta}{I_z} y + \frac{M \sin \theta}{I_y} z$$

Let consider a point 'A' on N.A. lies in Quadrant, where
 a Bending stress due to $P \cos \theta$ is compressive
 c Bending and stress due to $P \sin \theta$ is tensile.

$$\text{eq (1)} \quad 0 = -\frac{M \cos \theta}{I_z} y_A + \frac{M \sin \theta}{I_y} z_A$$

$$\Rightarrow 0 = \frac{M \cos \theta}{I_z} y_A + \frac{M \sin \theta}{I_y} z_A$$

$$= \frac{M \cos \theta}{I_z} y_A + \frac{M \sin \theta}{I_y} z_A$$

$$\frac{y_A}{z_A} = \frac{I_z}{I_y} \frac{\sin \theta}{\cos \theta} \Rightarrow \tan \alpha = \frac{I_z}{I_y} \tan \theta \quad \text{--- (2)}$$

Now put values of I_z , I_y and θ in eq (2)

$$\tan \alpha = \frac{I_z}{I_y} \tan 30$$

$$\tan \alpha = \frac{2.08125 \times 10^5}{1.25 \times 10^5} (\tan 30^\circ)$$

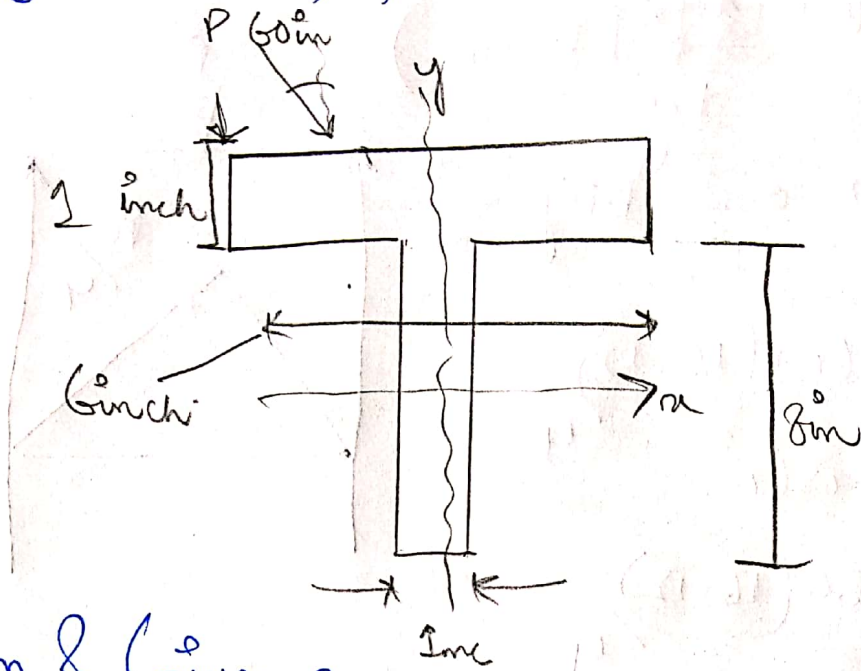
$$\tan \alpha = -14.4124$$

$$\alpha = \tan^{-1}(-14.4124)$$

$$\alpha = 1.05^\circ$$

$$\alpha = 1^\circ 30' 5''$$

Question No (2)(b):



Solution & Given:

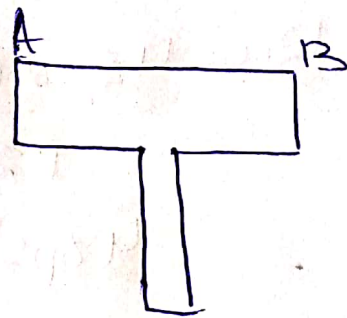
$$L = 16 \text{ ft}$$

$$I_x = 112.6 \text{ in}^4$$

$$I_y = 18.7 \text{ in}^4$$

$$E_c = 12000 \text{ psi}$$

$$E_f = 5000 \text{ psi}$$



By looking figure we can judge that maximum compression would occur on A and maximum tension C at B. There will be tension as well as a compression which will reduce that effect of each other so we will calculate stress at A and C

so

$$\sigma_A = \frac{M x_y}{I_x} + \frac{M y_x}{I_y} \text{ comp.}$$

$$f_c = \frac{M_{xy}}{I_x} + \frac{M_{yx}}{I_y} \text{ (tension)}$$

Now M_x and M_y

So

$$M_x = P \cos 60^\circ (16 \times 12) / 4$$

$$M_x = 48 P \cos 60^\circ$$

$$M_y = P \sin 60^\circ (16 \times 12) / 4$$

$$M_y = 48 P \sin 60^\circ$$

$$\text{Now } \sigma_A = \frac{M_{xy}}{I_x} + \frac{M_{yx}}{I_y}$$

$$1200 = \frac{48 P \cos 60^\circ \times 3.07}{112.6}$$

$$= 48 P \sin 60^\circ \times 30 / 18.7$$

$$= 48 P \sin 60^\circ \times 30 / 18.7$$

Solving the equation

$$\Rightarrow P = 1638.6 \text{ lb}$$

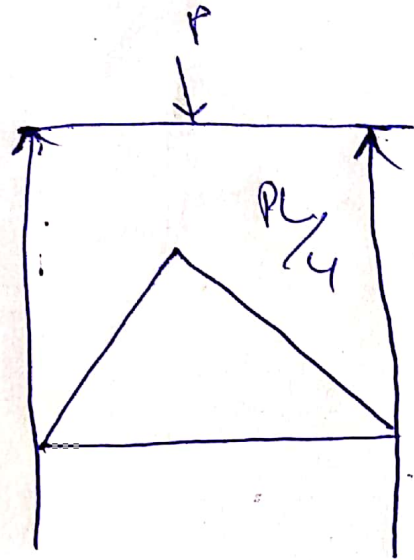
$$\text{Now } \sigma_c = \frac{M_{xy}}{I_x} + \frac{M_{yx}}{I_y}$$

$$5000 = 48 P \cos 60^\circ \times (5.93) + \frac{48 P \sin 60^\circ \times 0.5}{18.7}$$

$$18.7$$

Solving the equation

$$P = 2204.9 \text{ lb}$$



So the maximum load P applied should
1638.6 lb.



Question No (3):

Given Data:

$$\text{length } (L) = 10 \text{ ft}$$

As both sides are hinged

$$\text{So } l_e = L$$

$$E = 10.3 \times 10^6$$

$$\text{Factor of safety} = 2$$

$$b = 0.75 \text{ inch}$$

$$h = 2 \text{ inch}$$

Required Data:

Determine safe load = ?

Solution: As

$$P_{cr} = \pi^2 \frac{EI}{l_e^2}$$

As we know that

$$I = A r^2$$

$$r = \sqrt{\frac{I}{A}}$$

$$r = \sqrt{\frac{\frac{hb^3}{12}}{bh}} \Rightarrow \sqrt{\frac{b^2}{12}}$$

$$r = \frac{b}{2\sqrt{3}} = \frac{0.75}{2\sqrt{3}} = r = 0.216 \text{ inch}$$

$$P_{cr} = \frac{\pi^2 EI}{(L_e/r)^2}$$

$$\Rightarrow (3.14)^2 (10.3 \times 10^6) (15) / \left(\frac{10}{0.216} \right)^2$$

$$P_{cr} = 853.843$$

Safe load = crippling load / factor of safety.

$$\Rightarrow 853.8343 / 2$$

$$\text{Safe load} = 426.917$$

* for fixed extended column

$$L_e = L/2 = 10/2$$

$$L_e = 5 \text{ ft}$$

$$P_{cr} = \frac{\pi^2 EI}{(L_e/r)^2} = \frac{(3.14)^2 \times (10.3 \times 10^6) (1.5)}{\left(\frac{60}{0.216} \right)^2}$$

$$P_{cr} = 1974.207$$

Safe load = P_{cr} / factor of safety.

$$= 1974.207 / 2$$

$$= 987.103$$

Ans.