

Mechanics Of Solid



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Q# 1 Part (A)

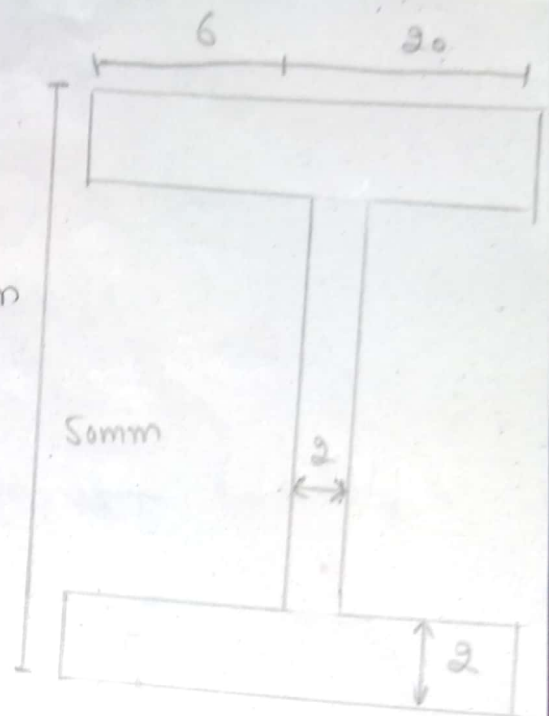
Given that

Height of section $h = 50 \text{ mm}$

Thickness $\Rightarrow b \Rightarrow 20 + 6$

$$b = 26 \text{ mm}$$

$$T_f = 2 \text{ mm}$$



Required :-

Shear Center = ?

As we know that

For Unsymmetrical member, the Shear Center is some distance away from the Centre

\Rightarrow This distance is called eccentricity which is given as

$$e = \frac{T_e h^2 b^2}{4I}$$

Here I = moment of Inertia and
is given as

$$I = 2 \left(\frac{bh^3}{12} + Ay^2 \right) + \left(\frac{bh^3}{12} + Ay^2 \right)$$

$$\Rightarrow I = 2 \left(\frac{2b(2)^3}{12} + (2 \times 2)(25)^2 \right) + \left(\frac{2(50)^3}{12} + 0 \right)$$

$$I = 50034.66 + 20833$$

$$I = 70867.99 \text{ mm}^4$$

Now equation ① \Rightarrow

$$e = \frac{Te h^2 b^2}{4I}$$

$$e = \frac{2 \times (50)^2 \times (25)^2}{4(7086.799)}$$

$$e = 11.0234 \text{ mm}$$

So, shear center is 11.0234 mm away
from geometrical center.

Q#1:- Part B:-

Given data:-

$$\text{Height} = h = 26 \text{ ft}$$

$$\text{Tangential stress} = 6000 \text{ Psi}$$

$$\text{Specific weight of water} = 62.4 \text{ lb/ft}^3$$

Required Data:-

Thickness of wall of water tank

$$= t = ?$$

Solution:-

As we know that

Pressure is equal to

$$P = \gamma h$$

$$Gt = \frac{PD}{2t} = \frac{\gamma h \times D}{2t}$$

$$t = \frac{\gamma h D}{2Gt}$$

Putting the values, we get

$$t = \frac{62.4 \times 26 \times D}{(12)^3}$$
$$\frac{\quad}{2(6000)}$$

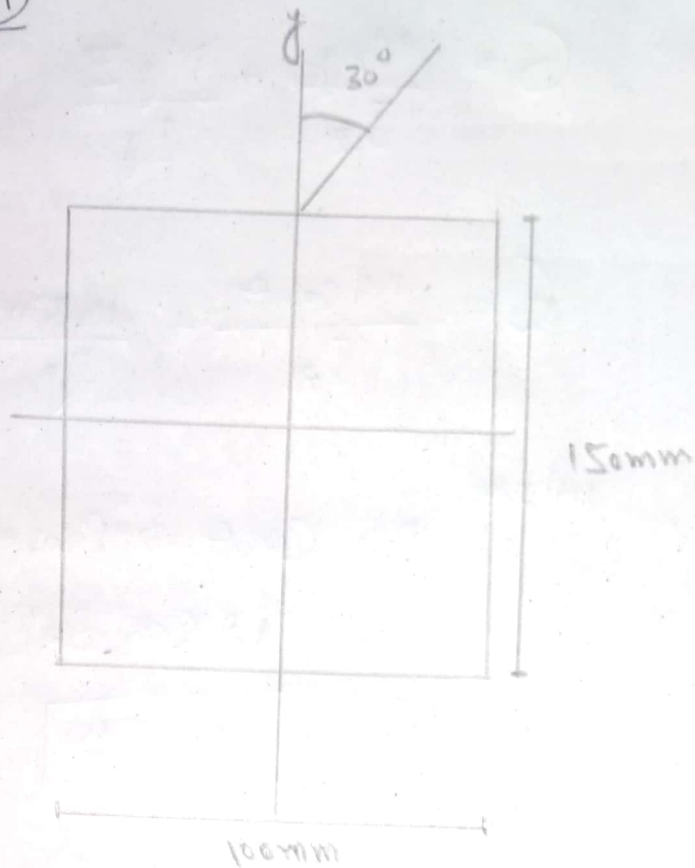
$$t = \frac{62.4 (26 \times 12) (22 \times 12)}{(12)^3}$$
$$\frac{\quad}{2(6000)}$$

$$t = 0.24 \text{ inch}$$

Ans

Q#2 :- Part (A)

Ans :-



Moment of Inertia

$$I_z = \frac{bh^3}{12} \Rightarrow \frac{0.1 (0.15)^3}{12}$$

$$I_z = 2.8125 \times 10^{-5}$$

Now

$$I_y = \frac{bh^3}{12} \Rightarrow \frac{(0.15)(0.1)^3}{12}$$

$$I_y = 1.25 \times 10^{-5}$$

$$\sigma = \frac{M_z y}{I_z} + \frac{M_y z}{I_y}$$

$$\sigma = \frac{M \cos \theta}{I_z} + \frac{M \sin \theta}{I_y}$$

where

$$M = P \cos \theta \Rightarrow P \cos \theta = M_z \\ = 12 \cos 30^\circ$$

$$\boxed{M_z = 1.8510}$$

$$M \sin \theta = P \sin \theta = M_y$$

$$12 \sin 30^\circ$$

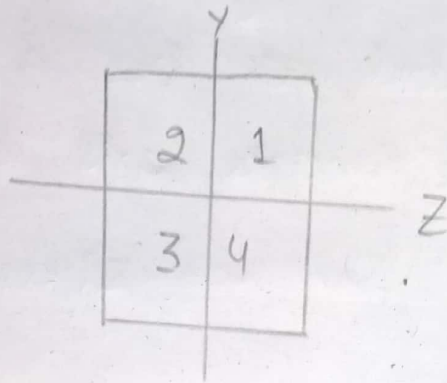
$$\boxed{M_y = -11.8563}$$

$$\sigma = \left(\frac{M_z}{I_z} \right) + \left(\frac{M_y}{I_y} \right)$$

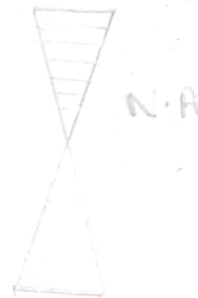
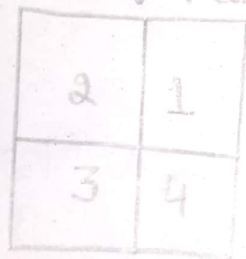
$$\sigma = \frac{1.851}{2.812 \times 10^{-5}} + \left(\frac{-11.8563}{1.25 \times 10^{-5}} \right)$$

$$\boxed{\sigma = 882678 \text{ N/m}^2}$$

Sign Convention

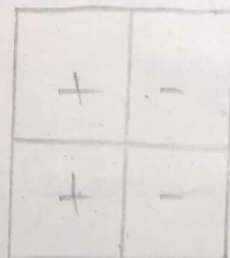


* If we take compression as negative and tension as positive and ~~tension~~ the beam is simply supported.

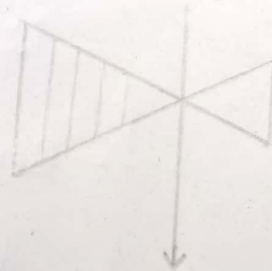


Quadrant 1, 2 -ve

Quadrant 3, 4 +ve



← $f \sin \theta$



Quadrant 1, 2 -ve

Quadrant 3, 4 +ve

In case of unsymmetrical loading the Neutral axis lies at an angle of " θ " to the ~~the~~ Principle axis and the algebraic sum of stress at N.A is Zero-

$$\sigma = \frac{M \cos \theta}{I_z} y + \frac{M \sin \theta}{I_y} z \rightarrow \textcircled{1}$$

In this case N.A passes through 2, 4

$$\sigma = \frac{M \cos \theta \cdot y}{I_z} + \frac{M \sin \theta \cdot z}{I_y}$$

Let consider a point "A" on NA lies in Quadrant 2, where

- Bending stress due to $P \cos \theta$ is compressive
- Bending stress due to $P \sin \theta$ is tensile-

eq ①

$$0 = \frac{-M \cos \theta y_A}{I_z} + \frac{M \sin \theta z_A}{I_y}$$

$$\frac{M \cos \theta y_A}{I_z} = \frac{M \sin \theta z_A}{I_y}$$

$$\frac{y_A}{z_A} = \frac{I_z \sin \theta}{I_y \cos \theta}$$

$$\tan \alpha = \frac{I_z}{I_y} \cdot \tan \theta \rightarrow \textcircled{2}$$

Now Put values of I_z , I_y and θ
in eqn ②

$$\tan \alpha = \frac{I_z}{I_y} \tan 30$$

$$\Rightarrow \frac{2.8125 \times 10^{-5}}{1.25 \times 10^{-5}} \tan 30^\circ$$

$$\tan \alpha = -14.4129$$

$$\alpha = \tan^{-1}(-14.4129)$$

$$\alpha = 1.5^\circ$$

$$\alpha = 1^\circ 30' 5'' \text{ Ans}$$

Q# 2 B Part:-Given Data:-

$$L = 187t$$

$$I_x = 112.6 \text{ in}^4$$

$$I_y = 18.7 \text{ in}^4$$

$$\sigma_c = 12000 \text{ Psi}$$

$$\sigma_t = 5000 \text{ Psi}$$

Solution:-

By seeing to the figure, we can judge that maximum compression would occur on A and maximum tension at C - At B there will be taken in well as compression which will reduce the effects of each other -

So, we will calculate stresses at A and C

$$\text{So, } \sigma_A = \frac{M_x y}{I_x} + \frac{M_y x}{I_y} \text{ Compression}$$

$$\sigma_c = \frac{M_x y}{I_x} + \frac{M_y x}{I_y} \text{ Tension}$$

Now M_x and M_y

$$\text{So } M_x = \frac{P \cos 60^\circ \times 16 \times 12}{4}$$

$$M_x = 48 P \cos 60^\circ$$

$$M_y = \frac{48 P \sin 60^\circ (16 \times 12)}{4}$$

$$M_y = 48 P \sin 60^\circ$$

Now

$$\sigma_A = \frac{M_x y}{I_x} + \frac{M_y x}{I_y}$$

$$12000 = \frac{48 P \cos 60^\circ \times 3.07}{1126} + \frac{48 P \sin 60^\circ \times 3}{18.7}$$

By solving the equation

$$P = 1638.6 \text{ lb}$$

Now

$$\sigma_c = \frac{M_x y}{I_x} + \frac{M_y x}{I_y}$$

$$\sum \tau_{2000} = \frac{48P \cos 60 \times (5.93)}{112.6} + \frac{48 \sin 60 \times 0.5}{18.7}$$

Solving the equation

$$P = 2104.9 \text{ lb}$$

So the maximum load of P applied should be 1638.6 lb



Q# 3:-Given Data:-

$$\text{Length of Column} = L = 10 \text{ ft}$$

$$E = 10.3 \times 10^6$$

$$\text{Breadth} = B = 0.75$$

$$\text{Height} = h = 2$$

$$\text{Factory of Safety} = 2$$

Required:-

Safe load = ?

When

Both end hinged

Both end fixed

So

⇒ For hinged column

Effective length $l_e = L$

$$I = I_x = \frac{bh^3}{12}$$

$$= \frac{0.75(2)^3}{12}$$

$$I_x = 0.65 \text{ inch}^2$$

$$P_{\text{critical}} = \frac{n^2 EI \pi^2}{L_e^2}$$

$$= \frac{(1)^2 (10.3 \times 10^6) (1.5) \pi^2}{(10 \times 12)^2}$$

$$P_{\text{cr}} = \frac{50776940}{14400}$$

$$= \boxed{3526.176 \text{ lb}}$$

$$\text{Safe load} = P_{\text{safe}} = \frac{P_{\text{cr}}}{\text{Factor of safety}}$$

$$= \frac{3526.176}{2}$$

$$\Rightarrow \boxed{P_{\text{safe}} = 1763.088 \text{ lb}}$$

When Both ends fixed in this

$$\text{Case } L_e = \frac{1}{2}$$

$$L_e = 586$$

$$I = I_y = \frac{2\pi (0.75)^3}{12} = 0.07 \text{ inch}^4$$

$$P_{\text{cr}} = \frac{n^2 EI \pi^2}{L_e^2}$$

$$= \frac{(1)^2 (10.3 \times 10^6) (0.07) (3.14)^2}{(5 \times 12)^2}$$

$$P_{cr} = \frac{7108771.6}{60^2}$$

$$P_{cr} = \frac{1974.658}{2}$$

So, Safe load

$$P_{\text{safe load}} = \frac{1974.658}{2}$$

$$P_{\text{safe}} = 987.3293 \text{ kN}$$

Ans

