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I-D = 7671

Section = (B)

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Exam = Final Term

Date = 23 - 06 - 020

IQRA NATIONAL

UNIVERSITY

Q NO# 01

Part (B)

Data :-

$$\Rightarrow H = 26 \text{ ft.}$$

$$\Rightarrow \text{assume diameter } D = 22 \text{ ft.}$$

$$\Rightarrow \text{Tangential stress} = 600 \text{ lb/ft.}$$

$$\Rightarrow \text{Specific weight of water}$$

$$\text{Tank} = 62.4 \text{ lb/ft}^3.$$

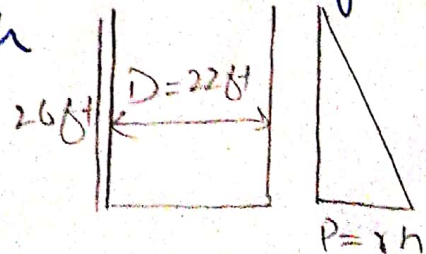
we have to find the thickness = ?

Solution :-

The pressure developed by

$$\text{water} = P = \gamma h$$

$$\sigma_t = \frac{PD}{2t}$$



$$\sigma_t = \frac{PD}{2t} \Rightarrow \frac{\gamma h D}{2t}$$

$$2t \times \sigma_t = \gamma h D$$

P.T.O

$$2t = \frac{rhd}{\delta_t}$$

$$t = \frac{rhd}{\delta_t \times 2}$$

~~$$t = \frac{(62.4) \times (26 \times 12) \times (22 \times 12)}{(112)^3}$$~~

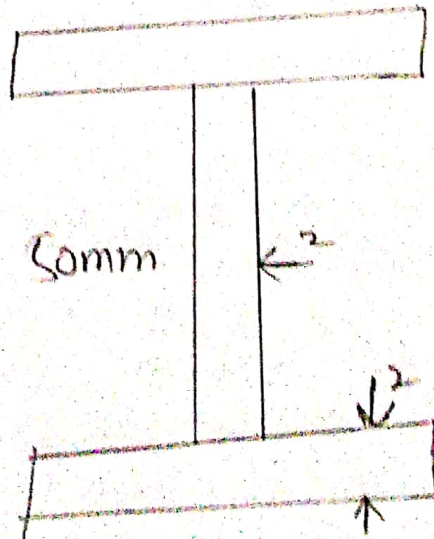
$$t = \frac{(62.4) \times (26 \times 12) \times (22 \times 12)}{(112)^3}$$

$$6000 \times 2$$

$$t = 0.24''$$

Q NO # 01

Part (a) . 1 - 6 - 1 - 20 - 1



Required:-

Location of Shear Centre:-

Sol:- As we know.

$$e = \frac{6 + bh^2 + b^2}{4I}$$

And:-

$$I = 2 \left(\frac{bh^3}{12} + Ay^2 \right) + \left[\frac{bh^3}{12} + Ay^2 \right]$$

$$= 2 \left[\frac{26(2)^3}{12} + (20 \times 2)(25)^2 \right] +$$

$$\left[\frac{2(50)^3}{12} + 0 \right]$$

$$I = 50034.66 + 20833$$

$$I = 70867.99 \text{ mm}^4$$

$$e = \frac{2(50)^2(25)^2}{4(70867.99)} = 11.02 \text{ mm.}$$

So Shear Centre.

$$e = 11.02 \text{ mm. Ans.}$$

Q NO# 03:

Ans:

Given Data:

$$\text{length} = 10 \text{ ft.}$$

$$E = 10.3 \times 10^6$$

$$b = 0.75$$

$$n = 2$$

Factor of Safety = 7.

Required:

- Safe load at hinged = ?
- Safe load at fixed = ?

Sol:-

a) For hinged column.

$$L_e = L.$$

$$I = I_x = \frac{(0.75)(2)^3}{12} = 0.5 \text{ in}^4$$

$$P_{cr} = \frac{n^2 E I \pi^2}{L_e^2} = \frac{(1)^2 (10.3 \times 10^6) (0.5) \pi^2}{(10 \times 12)^2}$$

$$P_{cr} = 0$$

$$P_{cr} = \frac{50776940}{14400} = 3526.176 \text{ lb}$$

$$P_{\text{Safe load}} = \frac{P_{cr}}{\text{Factor of Safety}}$$

$$= \frac{3526.176}{2} = 1763.088 \text{ lb}$$

(B)

Strut act column.

$$L_e = L/2 \text{ (For Fixed ended)}$$

$$L_e = \frac{10}{2} = 5 \text{ ft.}$$

$$I = I_y = \frac{2 \times (0.75)^3}{12} = 0.07 \text{ in}^4 \quad (3.14^4)$$

$$P_{cr} = \frac{\pi^2 EI \pi^2}{L_e^3} = \frac{(1)^2 (10.3 \times 10^6) (0.07)}{(5 \times 12)^2}$$

$$P_{cr} = \frac{7108771.6}{(60)^2}$$

P.T.O

$$P_{cr} = 1974.658 \text{ lb}$$

$$P_{\text{safe load}} = \frac{1974.658}{2}$$

$$= 987.3293 \text{ lb}$$

occur on a y maximum tension
 at B there will tension
 as well a compression which
 will reduce that effect of each
 other so we will calculate
 stress at A by C.

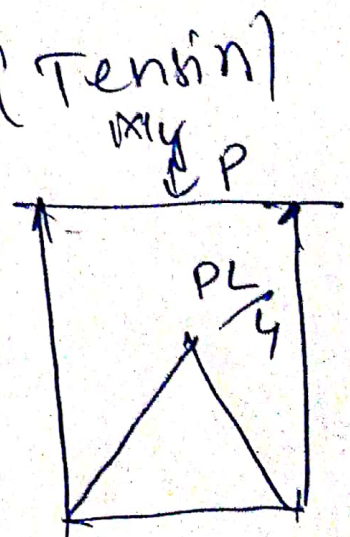
$$\text{So } \sigma_A = \frac{M_x y}{I_x} + \frac{M_y x}{I_y} \text{ (Comp)}$$

$$\text{So } = \frac{M_x y}{I_x} + \frac{M_y x}{I_y} \text{ (Tension)}$$

Now M_x by

$$\text{So } M_x = \frac{P_{\text{safe load}} \times (10 \times 12)}{4}$$

$$M_x = 48 P_{\text{safe load}}$$

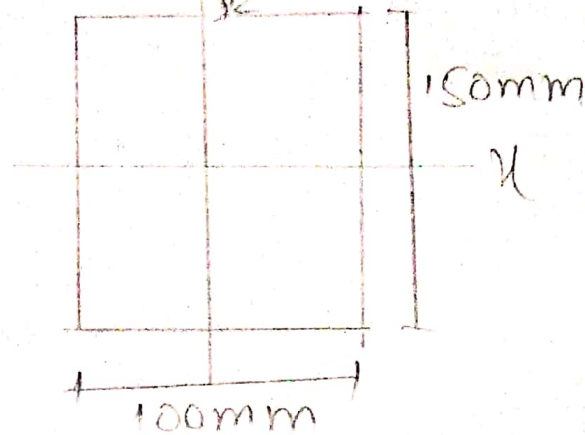


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$$My = \frac{P \text{ ringo } (16 \times 12)}{4}$$

$$My = 50 \text{ pcs } 60.$$

Question NO # 02
Part (a).



Moment of inertia:-

$$I_z = \frac{bh^3}{12} = \frac{0.1 (0.15)^3}{12}$$

$$I_z = 2.8125 \times 10^{-5}$$

Now;

$$I_y = \frac{bh^3}{12} = \frac{0.15 (0.1)^3}{12}$$

$$I_y = 1.25 \times 10^{-5}$$

$$\sigma = \frac{M_z y}{I_z} + \frac{M_y z}{I_y}$$

$$\sigma = \frac{M \cos \theta}{I_z} + \frac{M \sin \theta}{I_y}$$

P.T.O

Pages No# ~~08~~ 09

$$M_x = C \cos \alpha = P \cos \alpha = M_z \\ = 18 \cos 30^\circ = M_z$$

$$M_z = 1.8510.$$

$$M_y \sin \alpha = P \sin \alpha = M_y.$$

$$M_y = 12 \sin 30^\circ$$

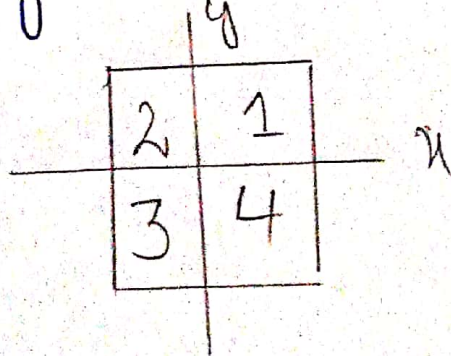
$$M_y = -11.8563$$

$$\sigma = \left(\frac{M_z}{I_z} \right) + \left(\frac{M_y}{I_y} \right)$$

$$\sigma = \frac{1.851}{2.812 \times 10^{-5}} + \left(\frac{-11.8563}{1.25 \times 10^{-5}} \right)$$

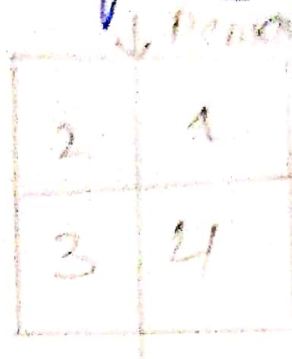
$$= 882628 \text{ Nm}^2$$

Sign Convention.

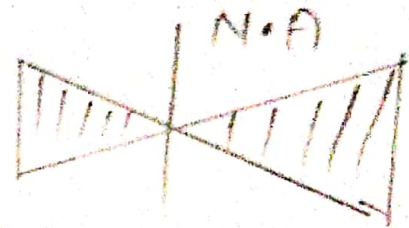


P.T.O

if we take Compression as a Negative
 by Tension as positive by the beam.
 is a simply supported -



Quardant 1, 2 -ive.
 Quardant 3, 4 +ive.



Quardant 1, 4 -ive
 Quardant 2, 3 +ive

incase of unsymmetrical loading
 the Neutral axis lies of an angle
 of " α " the principal axis by
 the algebraic sum of stresses
 at N.A is zero.

P.T-O

Pages NO# 10.

$$\sigma = \frac{m \cos \alpha}{I_z} y + \frac{m \sin \alpha}{I_y} z \rightarrow \textcircled{1}$$

in this case, N.A passes through
2, 4, 150.

$$\sigma = \frac{M \cos \alpha}{I_z} y + \frac{m \sin \alpha}{I_y} z$$

Let consider a point 'A' on N.A lies
in quadrant 3, where.

- Bending stress due to $P \sin \alpha$ is
Tensile.
- Bending stress due to $P \cos \alpha$ is
Tensile.

eqn ①

$$\Rightarrow \sigma = - \frac{M \cos \alpha}{I_z} y_A + \frac{m \sin \alpha}{I_y} z_A$$

$$\Rightarrow \frac{M \cos \alpha}{I_z} y_A + m \frac{\sin \alpha}{I_y} z_A$$

$$\frac{y_A}{z_A} = \frac{I_z}{I_y} \frac{\sin \alpha}{\cos \alpha} \Rightarrow \tan \alpha = \frac{I_z}{I_y} \tan \alpha$$

eqn ②

P.T.O

Pages No # 12
Now put values of I_z, I_y & α
in eqn (II).

$$\tan \alpha = \frac{I_z}{I_y} \tan 30^\circ$$

$$\Rightarrow \tan \alpha = \frac{3.8125 \times 10^{-5}}{1.25 \times 10^{-5}} (\tan 30^\circ)$$

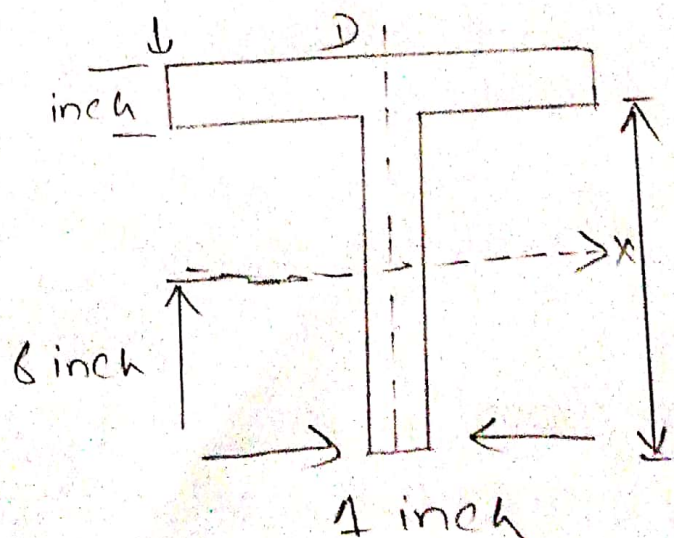
$$\tan \alpha = -14.4129$$

$$\alpha = \tan^{-1}(-14.4129)$$

$$\alpha = 1.5^\circ$$

$$\alpha = 1^\circ 30' 5'' \quad \text{Ans.}$$

Q NO # 02
part (B).



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$$L = 16 \text{ ft.}$$

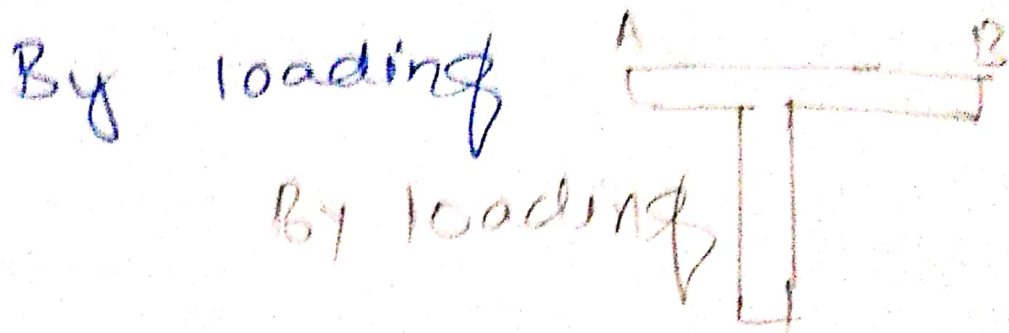
$$I_x = 112.6 \text{ inch}^4$$

$$I_y = 18.7 \text{ in}^4$$

$$\sigma_c = 12000 \text{ psi}$$

$$\sigma_t = 5000 \text{ psi.}$$

Sol:-



After we can judge that maximum compression would.

$$M_y = 48P \sin 60$$

Now

$$\sigma_A = \frac{M_{xy}}{I_x} + \frac{M_{yx}}{I_y}$$

$$\Rightarrow 12000 = \frac{48P \cos 60^\circ \times 3.074}{112.6}$$
$$P = 70$$

Page # 14

$$\frac{48 p \sin 60 \times 30}{18.7}$$

Solving the equation.

$$\Rightarrow p = 1638.6 \text{ lb}$$

Now;

$$I_c = \frac{M_x y}{I_x} + \frac{M_y x}{I_y}$$

$$5000 = 48 p \cos 60 \times (593) +$$

$$\frac{48 p \sin 60 \times 0.5}{18.7}$$

Solving the equation

$$p = 2104.9 \text{ lb}$$

So the maximum load p applied should be 1638.6 lb .