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Section A

Paper Advance Fluid Mechanics

Q.No.1): (a)

Ans: velocity profile For laminar flow:

As we have

$$h_L = \frac{\tau \cdot 2L}{\rho g}$$

From viscosity $\Rightarrow \tau = \mu \frac{du}{dy}$ — (1)

Where "u" is velocity at distance "y" from the boundary

Thus $y = r_0 - r$

$$dy = d r_0 - dr$$

$$dy = -dr$$

$\therefore dr \cdot u$ is constant value.

Putting value in (1)

$$\tau = -\mu \frac{du}{dr}$$

Now $h_L = \frac{\tau \cdot 2 \cdot L}{\rho g} \cdot r dr$

Integration on both side

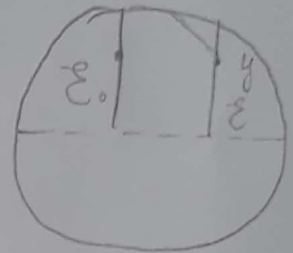
$$\int du = \int \frac{-h_L r}{2\mu L} \cdot r \cdot dr$$

$$u = \frac{-h_L r}{2\mu L} \cdot \frac{r^2}{2} + C$$

Now for $r = 0$, $u = u_{max}$

Putting values

$$u = \frac{-h_L r}{2\mu L} \cdot \frac{r^2}{2} + C$$



$$u = u_{\max}, \quad u_{\max} = 0 + C$$

$$C = u_{\max}$$

Thus
$$u = u_{\max} - \frac{hL\gamma}{4\mu L} \cdot \frac{\Sigma^2}{2}$$

(velocity at any point)

Assume
$$K = \frac{hL\gamma}{4\mu L} \cdot \frac{h\gamma D^2}{16\mu L} \therefore u = u_{\max} - K\Sigma^2$$

As
$$\Sigma = \Sigma_0, \quad u = 0$$

$$0 = u_{\max} - K\Sigma_0^2 \quad \text{or}$$

$$u_{\max} = K\Sigma_0^2 = \frac{hL\gamma}{4\mu L} \cdot \Sigma_0^2 = \frac{hc\gamma \cdot D^2}{16\mu L}$$

(It is also known as critical velocity.)

$$v_{av} = \frac{v_c \Sigma + 0}{2} = 0.5 v_c \Sigma$$

↓
average velocity

$$= \frac{hL\gamma \cdot D^2}{32\mu L} \quad \text{AS } \gamma = \frac{g}{L}, \quad \mu/\gamma = \nu$$

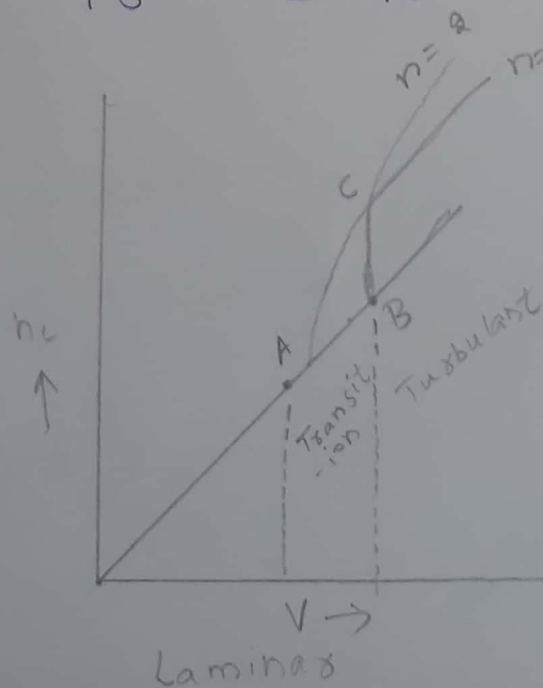
$$\Rightarrow \frac{32\mu L \nu}{\rho g \cdot D^2} = \frac{32\nu L}{g D^2} \nu$$

Q No. 1):

(b):

Ans): Critical Reynold Number: If head loss in given length of uniform pipe is measured at different values of velocity, it will found that as long as velocity is low enough to secure laminar flow, the head loss due to friction will be directly proportional to velocity, but increase in velocity change flow from laminar to turbulent cause change in head loss. Thus its values are plotted, lines obtained with slope ranging about 1.75 to 2.

Thus for laminar, drop of energy varies as V and for turbulent, friction varies as V^n where "n" is 1.75 to 2.



The upper critical Reynolds number is indeterminate and depend upon case taken to prevent initial disturbance. Its value is 4000. But normally, its impossible for flow to be in straight after R is at 2000. Thus lower value is than higher one more definite point. Thus lowest value is dividing true critical Reynold number.

$$R = \frac{D V_{cr}}{\nu}$$

QNO.2):

Solution:

Specific Gravity (s) = 0.7

Kinematic viscosity = $1.8 \times 10^{-5} \text{ m}^2/\text{sec}$

Dia of pipe = 150 mm = 0.15 m

Flow = 0.5 L/sec = $0.0005 \text{ m}^3/\text{sec}$

First we check flow is laminar or turbulent

$$R = \frac{DV}{\nu} \quad \text{--- (1)}$$

$$V = \frac{Q}{A} = \frac{Q}{\frac{\pi}{4} d^2} = \frac{0.0005}{\frac{\pi}{4} (0.15)^2}$$

$$V = 0.028 \text{ m/sec}$$

$$R = \frac{(0.15)(0.028)}{1.8 \times 10^{-5}}$$

$$R = 233.33 < 2000 \text{ (Laminar)}$$

$$V_{\text{cr}} = 2V = 2 \times 0.028$$

$$V_{\text{cr}} = 0.056 \text{ m/sec}$$

AS

$$u = u_{\text{max}} - K r^2$$

at $r = r_0 = 0.075 \text{ m}$, $u = 0$

Thus
$$u = u_{\text{max}} - K r^2$$

$$U_{max} = K \delta^2$$

$$K = \frac{U_{max}}{\delta^2} = \frac{0.056}{(0.075)^2}$$

$$K = 9.96$$

We get an equation

$$u = 0.056 - 9.96(\delta^2) \rightarrow \star$$

velocity at 10mm from edge

$$\delta = 0.065 \text{ m}$$

$$v = 0.056 - 9.96(0.065)^2$$

$$v = 0.014 \text{ m/sec}$$

velocity at edge;

$$\delta = 0.075 \text{ m}$$

$$v = 0.056 - 9.96(0.075)^2$$

$$v = -0.0002 \text{ m/sec} \quad \text{Say } v = 0$$

Similarly

$$f = \frac{64}{R} = \frac{64}{233.33}$$

$$f = 0.27$$

Shear stress at wall;

$$\tau = \frac{f}{4} \rho \frac{v^2}{2}$$

$$= \frac{0.27}{4} \times (0.7 \times 1000) \times \frac{(0.056)^2}{2}$$

$$\tau = 0.074 \text{ N/m}^2$$

