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Subject

Hydraulic Structure

Assignment

~~QSD~~

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Ans: "Stage Discharge relationship
for a concrete rectangular Box

= Culvert;

Given data:

$$\text{Width} = 1.4 \text{ m}$$

$$\text{Height} = 0.9 \text{ m}$$

$$\text{Length} = 26 \text{ m}$$

$$\text{Slope} = 1:1000$$

$$\text{Manning's } n = 0.013$$

Square edged entrance, $K_e = 0.5$

$$\text{Range} = 0-3 \text{ m}$$

Solution:

$$H/D \leq 1.4 \text{ m}$$

$$H < 0.9 \text{ m}$$

Discharge is given by:

$$Q = 2.92 y_0 \left[\frac{1.2 y_0}{1.2 + 2 y_0} \right]^{2/3} \text{ --- } \text{A}^2$$

y_0 (m)	Q (m^3s^{-1})	y_c (m)
0.3	0.299	0.166
0.6	0.785	0.317
0.9	1.330	0.451

* By putting values of " y_0 " we will get the corresponding discharge.

$$\begin{aligned} Q_1 &= 2.92 (0.3) \left[\frac{1.2(0.3)}{1.2 + 2(0.3)} \right]^{2/3} \\ &= 0.299 \text{ m}^3/\text{s} \end{aligned}$$

$$\begin{aligned} Q_2 &= 2.92 (0.6) \left[\frac{1.2(0.6)}{1.2 + 2(0.6)} \right]^{2/3} \\ &= 0.785 \text{ m}^3/\text{s} \end{aligned}$$

$$\bullet Q_3 = 2.92(0.9) \left[\frac{1.2(0.9)}{1.2 + 2(0.9)} \right]^{2/3}$$

Critical Depth:

$$Y_c = \left(\frac{q^2}{g} \right)^{1/3} \longrightarrow \text{"A"}$$

$$q = Q/B \longrightarrow \text{"B"}$$

By putting values in eq "B"

$$q_1 = Q_1/B = \frac{0.299}{1.4} = 0.213$$

$$q_2 = Q_2/B = \frac{0.785}{1.4} = 0.561$$

$$q_3 = Q_3/B = \frac{1.330}{1.4} = 0.95$$

Now by putting values in eq "A":

$$Y_{c1} = \left(\frac{q_1^2}{g} \right)^{1/3} = \left(\frac{(0.213)^2}{9.81} \right)^{1/3} = 0.166 \text{ m}$$

$$y_{c2} = \left(\frac{q v_2^2}{g} \right)^{1/3} = \left(\frac{0.561^2}{9.81} \right)^{1/3} = 0.317 \text{ m}$$

$$y_{c3} = \left(\frac{q v_3^2}{g} \right)^{1/3} = \left(\frac{0.95^2}{9.81} \right)^{1/3} = 0.451$$

At the Inlet over a short reach;

$$" H = y_0 + \frac{v^2}{2g} + k_e \cdot \frac{v^2}{2g} "$$

$$V_1 = 1.142 \text{ m/s}$$

So,

$$H_1 = y_{01} + \frac{v^2}{2g} + k_e \cdot \frac{v^2}{2g}$$

$$= 0.3 + \frac{(1.142)^2}{2(9.81)} + 0.5 \left(\frac{(1.142)^2}{2(9.81)} \right)$$

$$H_1 = 0.399 \text{ m}$$

$$H_2 = 0.6 + \frac{(1.142)^2}{2(9.81)} + 0.5 \left(\frac{(1.142)^2}{2(9.81)} \right)$$

$$H_2 = 0.699 \text{ m}$$

$$H_3 = 0.9 + \frac{(1.142)^2}{2(9.81)} + 0.5 \left(\frac{(1.142)^2}{2(9.81)} \right)$$

$$H_3 = 0.999 \text{ m}$$

y_0 (m)	H (m)	Q ($\text{m}^3 \text{s}^{-1}$)
0.3	0.399	0.299
0.6	0.699	0.785
0.9	0.999	1.330
Orifice > 0.9 "1.2" ▽	1.08	→ 1.477 By Interpolation

$$\boxed{2''} \quad H/D \geq 1.4$$

"a" ; For orifice flow ;

$$Q = C_d (1.4 \times 0.9) [2g(H - D/2)]^{1/2}$$

$$Q = 0.62 (1.4 \times 0.9) [2(9.81)(1.08 - \frac{0.9}{2})]^{1/2}$$

$$\boxed{Q = 2.746 \text{ m}^3/\text{s}}$$

The following Results are obtained

H (m)	Q (m ³ s ⁻¹)	y ₀ (m)
1.08	2.746	70.9,

↪ no orifice flow exists.

\boxed{b} For Pipe Flow the energy Equation gives ;

$$H + S_{OL} = D + h_L$$

Where ;

$$h_L = K_e \frac{v^2}{2g} + (K_n)^2 \frac{L}{R^{4/3}} + \frac{v^2}{2g}$$

Thus

$$Q = 2.08(H - 0.57)^{1/2}$$

During rising stages the barrel flows fall from $H = 1.08\text{m}$ and during falling stages the flow becomes free-surface flow when

$$H = 0.999\text{m}.$$

The following table Summarizes the result;

H (m)	Q (m ³ /s)	Type of flow
Rising Stages;		
0.399	0.299	open channel
0.699	0.785	open channel
0.999	1.330	open channel
1.080	1.477	Pipe Flow
2.000	2.487	Pipe flow
3.000	3.042	Pipe flow
Falling Stages		
2.000	2.487	Pipe flow
1.080	1.477	Pipe flow
0.999	1.330	Pipe flow
0.699	0.785	Open channel
0.399	0.299	Open channel

Scour:-

It is an erosional process that can occur in rivers due to the interaction between any of structure located underwater and the river flow.

OR,

Scour is an erosional that can occur in rivers due to natural or made man events.

Natural erosional process take place in rivers because they act as conduits for movement of water sediment. Man made Scour can be caused for instance by legal or illegal sediment extraction dam operation and the influence in general of any structure placed onto the river system.

Different Types of Scour:-

Natural Scour:-

Natural Scour occurs due to the natural variability of river stream flow and sediment regime, considering the influence from the catchment to the river scale. Gradation of the river bed, lateral channel migration, bend and ~~confluence~~ confluence Scour are part of natural Scour.

Local Scour:

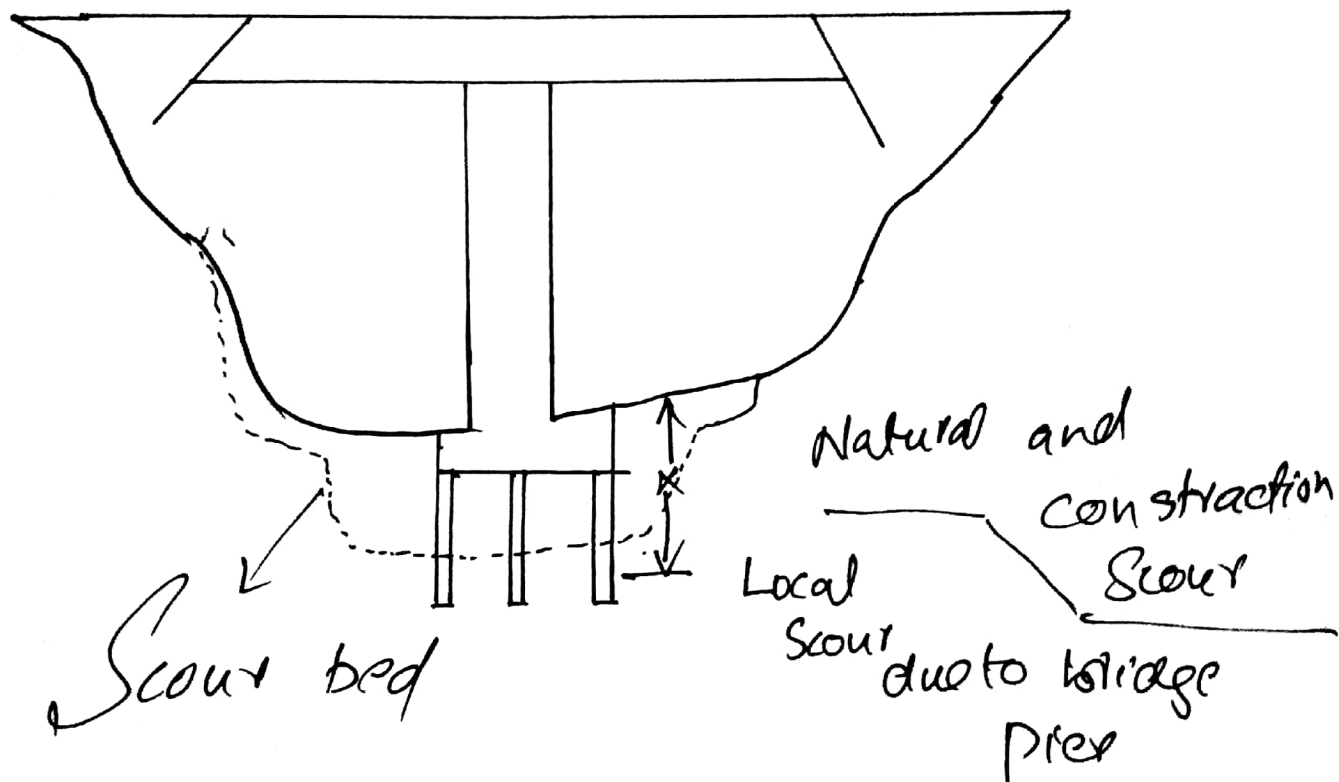
Local Scour emerges due to a local concentration of turbulence generated by structures that obstruct and split the flow. Local Scour occurs around these structures because of the limited influence range they have on the river flow.

Contraction Scour :-

Occurs due to flow contraction when flow velocity and thus shear stresses, increase, for instance between bridge abutments. Contraction Scour normally take place within the complete river stream width.

Total Scour:

Total Scour is defined as "The sum of effects of all the Scour process that take place at a given location"



Mechanics of Scour on bridge

Initial Phase:

The Scour process starts showing erosional patterns on the lateral side of the cylinder pier.

Progressing phase:

The erosional pattern progress from the lateral side to the front of pier. From the moment the two scour patterns coincide at the front of the pier, the deepest scour depth is achieved.

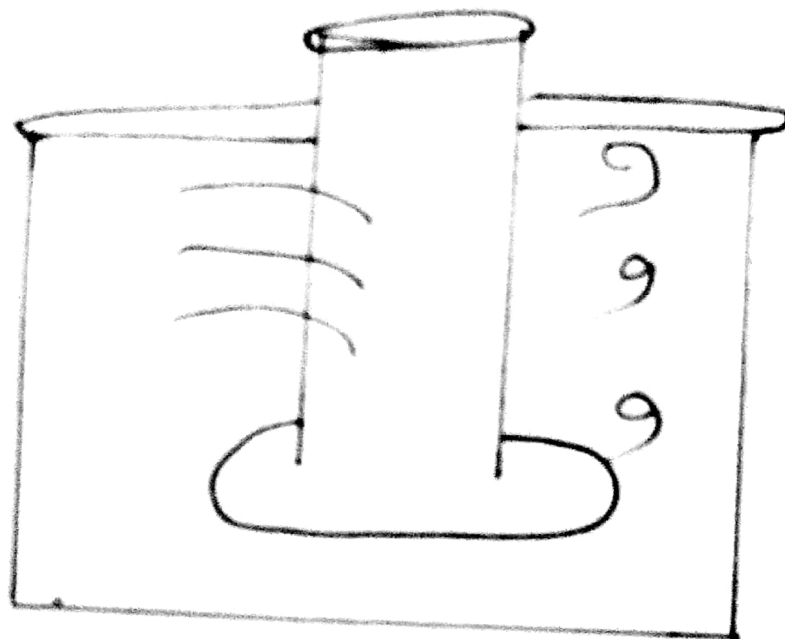
Developing phase:

The Scour process develops and the scour rate slows down.

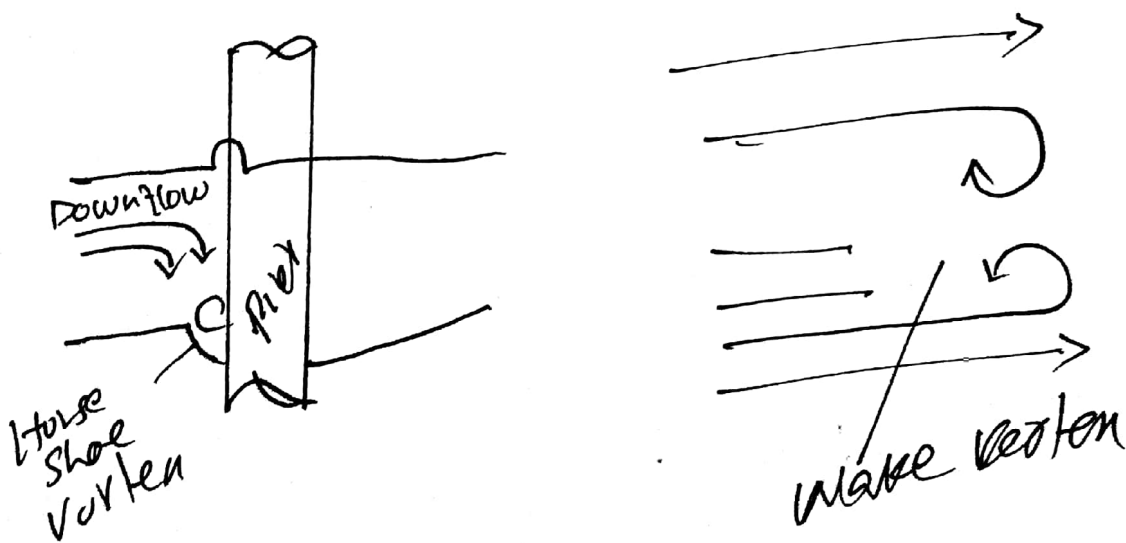
Equilibrium phase:

Erosion inside the scour hole is negligible

At the obstruction in form of pier or abutment the unidirectional flow changes into three dimensional as the water pile - up in front face of the obstruction and the flow accelerates around the nose. This phenomenon results. The pileup of water due to ~~the~~ obstruction because of decelerating of flow due to stagnation pressure of water ~~causes~~ a downward flow results in horse vortex. The vertical component of the downward flow causes erosion around the base of the pier.



Information of vortices at the base of the pier known as horse shoe vortices and the vortices from in the vertical direction downstream of the pier known as wake vortices as shown in figure.



Presentation of vortices around
a circular path

Due to rolling of unstable layer at the surface of the pier wake vortex are generated at the separation & moves forward with flow downstream of the pier. It can be shown.

In the practical case the river bed is generally composed of mixture of different size of material due to washing out scour finer material and armor layer is formed of coarser materials which protect the underlying finer particles from further scour.

