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Section	#	A.
Subject	#	Hydraulic Engineering
Assignment NO	#	01, 02, 03
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# Assignment # 1 page #1

Q1) What is venturi flume? Explain with detail?

Ans:- venturi flume is a critical-flow open flume with a constricted flow which caused a drop in the hydraulic grade line creating a critical depth. It is used in flow measured of very large flow rates. usually give a millions of cubic units. A venturi meter would normally measure in millimeter where as a venturi flume measure in meter.

Flumes require two measurement one upstream and one at the throat (narrowest cross section). If the flow passes in a subcritical state through the flume. If the flume are designed so as to pass the flow from sub to super critical state while passing through the flume a single measurement at the throat is sufficient for computation of discharge. To ensure the occurrence of critical depth at the throat the way as to form a hydraulic jump on downstream side of structure.

Qr A 3m wide channel carries a total discharge of  $12 \text{ m}^3/\text{sec}$  calculates

- (a) The critical depth
- (b) The minimum specific energy
- (c) The alternate depth when  $h = 4 \text{ m}$

Given data.

$$b = 3 \text{ m} \quad Q = 12 \text{ m}^3/\text{sec}$$

- (a) Discharge per unit width.

$$q = \frac{Q}{b} = \frac{12}{3} = 4 \text{ m}^2/\text{sec}$$

the rectangular channel

$$h_c = \left( \frac{q^2}{g} \right)^{1/3}$$

$$h_c = \left( \frac{4^2}{9.81} \right)^{1/3} = 1.177 \text{ m}$$

critical depth = 1.18 m

- (b) for rectangular channel:

$$E_c = \frac{3}{2} h_c = \frac{2}{3} \times 1.17$$

$$E_c = 1.766 \text{ m}$$

mini specific energy = 1.77



(c) As  $E > E_c$  there are two possible depths for a given specific energy

$$E = h + \frac{v^2}{2g} \quad \text{where} \quad v = \frac{Q}{A} = \frac{Q}{bh}$$

$$E = h + \frac{Q^2}{2g b^2 h^3}$$

Substituting values in meter-second unit

$$4 = h + \frac{0.8155}{h^2}$$

for the subcritical (slow-deep) solution the first term associated with potential energy

$$h = 4 - \frac{0.8155}{h^2}$$

e.g.  $h = 4$  gives  $h = 3.9518 \text{ m}$

for the supercritical solution

$$h = \sqrt{\frac{0.8155}{4-h}}$$

$$h = 0.4814 \text{ m}$$

alternate depths are 3.95 - and 0.481 m

Assignment # 2  
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Q(1) water flows at a depth of 10cm with a velocity of 6m/s in a rectangular channel is the flow subcritical or supercritical what is the alternate depth.

Given data:-

$$d = 10\text{cm}$$

$$v = 6\text{m/s}$$

Required:-

$$y_{alt} = ?$$

Solution:-

By checking Froude number.

$$Fr = \frac{v}{\sqrt{gy}} \Rightarrow \frac{6}{\sqrt{9.81 \times 0.1}} = 6.06$$

$Fr = 6.06 > 1$   
Flow is supercritical

$$E = y + \frac{v^2}{2g} = 0.1 + \frac{6}{2 \times 9.81}$$

$$E = 1.935\text{m}$$

For alternate depth  $E = 1.935\text{m}$

$$y_{alt} = 1.93$$

Q2:- water flow with a velocity of 2m/s and at a depth of 3m in a rectangular channel what is the change in depth and in water surface elevation produced by a gradual upward changing in bottom elevation up step of 60cm. what would be the depth and elevation changes if there were a gradual down step of 15cm what is max size of up step that could exist before upstream depth changes would result neglect head losses.

Given data:-

$$v_1 = 2 \text{ m/s}$$

$$y_1 = 3 \text{ m}$$

$$\Delta z = 60 \text{ cm} = 0.6 \text{ m}$$

$$\text{down step} = 15 \text{ cm} = 0.15 \text{ m}$$

Solution:-

$$E_1 = y_1 + \frac{v_1^2}{2g}$$

$$= 3 + \frac{2^2}{2 \times 9.81}$$

$$E_1 = 3.20 \text{ m}$$

now

$$E_1 = E_2 + \Delta z$$

$$= 3.2 - 0.6$$

$$E_2 = 2.60 \text{ m}$$



Also

$$E_2 = y_2 + \frac{v^2}{2gy_2^2}$$

$$2.60 = y_2 + \frac{6^2}{2 \times 9.81 y_2^2}$$

$$y_2 = 2.24 \text{ m}$$

$$\Delta y = y_2 - y_1$$

$$= 2.24 - 3$$

$$\Delta y = -0.76 \text{ m}$$

So water surface drop = 0.16 m

→ for a downward stop of 15 cm or 0.15 m we have.

$$E_2 = E_1 - \Delta z$$

$$E_2 = 3.20 - (-0.15)$$

$$E_2 = 3.35 \text{ m}$$

$$\text{Now } y_c = 3.17 \text{ m}$$

$$\Delta y = y_2 - y_1$$

$$\Delta y = 3.17 - 3$$

$$\Delta y = 0.17 \text{ m}$$

So water surface rises 0.02 m

$$E_2 = 3.35 \text{ m}$$

$$\text{now } \Delta y_2 = 3.17.$$

$$\Delta y = y_2 - y_1 = 3.7 - 3$$

$$\Delta y = 0.17$$

So water surface rises 0.02 m

The max upstep possible before effecting upstream water surface level is for  $y_2 = y_c$ .

$$y_c = 3 \sqrt{\frac{qv^2}{g}}$$

$$y_c = 3 \sqrt{\frac{62}{9.8}}$$

$$y_c = 1.54 \text{ m}$$



## Assignment # 3

Q. - A water passing from the slice gate in Dam having a depth of water of upstream side is 3.6m after passing through slice gate the back water curve shows that depth of water ~~curve~~ at down stream is 0.9m. The width of slice gate is 3.9m

Determine

(a) Discharge

(b) Froude no upstream and downstream

Given data:-

$$y_1 = 3.6 \text{ m}$$

$$y_2 = 0.9 \text{ m}$$

$$b = 3.9 \text{ m}$$

Solution:-

$$E_1 = E_2$$

$$y_1 + \frac{v_1^2}{2g} = y_2 + \frac{v_2^2}{2g} \quad \text{--- (1)}$$

Now

$$Q = A_1 v_1 = A_2 v_2$$

$$b y_1 \cdot v_1 = b y_2 \cdot v_2$$

$$y_1 v_1 = y_2 v_2$$

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$$v_2 = \frac{y_1}{y_2} \times v_1$$

$$v_2 = \frac{3.6}{0.9} \times v_1$$

$$v_2 = 4v_1 \rightarrow (2)$$

Put in eq (1)

$$y_1 + \frac{v_1^2}{2g} = y_2 + \frac{v_2^2}{2g}$$

$$3.6 + \frac{(v_1)^2}{2g} = 0.9 + \frac{(4v_1)^2}{2g}$$

$$\frac{(v_1)^2}{2g} - \frac{16v_1^2}{2g} = 0.9 - 3.6$$

$$\frac{v_1^2 - 16v_1^2}{2g} = -2.7$$

$$\frac{-15v_1^2}{2g} = -2.7$$

$$\sqrt{v_1^2} = \sqrt{\frac{2.7 \times (2 \times 9.81)}{15}}$$

$$v_1 = 1.879 \text{ m/sec.}$$

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Put in eqn we will get

$$v_2 = 4v_1$$

$$v_2 = 4(1.879)$$

$$v_2 = 7.516 \text{ m/sec}$$

$$\text{As } Q_1 = A_1 v_1 \quad \text{by } 1 \quad v_1$$

$$= 3.9 \times 3.6 \times 1.879$$

$$Q_1 = 26.38 \text{ m}^3/\text{sec}$$

$$Q_2 = A_2 v_2 = \text{by } 2 \quad v_2$$

$$3.9 \times 0.9 \times 7.516$$

$$Q_2 = 26.38 \text{ m}^3/\text{sec}$$

$$Q = Q_1 = Q_2 = 26.38 \text{ m}^3/\text{sec}$$

Froude number at upstream side

$$F_{s1} = \frac{v_1}{\sqrt{g y_1}} = \frac{1.879}{\sqrt{9.81 \times 3.6}}$$

$$F_{s1} = 0.31 \quad \text{Sub critical flow}$$



Froude number  $\rightarrow$  at downstream side

$$F_{r2} = \frac{v_2}{\sqrt{g y_2}} = \frac{7.516}{\sqrt{9.81 \times 0.9}}$$

$$F_{r2} = 2.52$$

Supercritical flow.