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Section:

B

Dept:

BE(C)

Subject

MOS-II

Submitted to:

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①

Question No 01 (a):

Determine the location of the Shear Center for the beams having the cross sectional dimensions shown in the figures 1. All members are to be considered thin walled and calculation should be based on the centerline dimension

Given Data:

Upper Section = 6mm and 20mm

Flange = 2mm

thickness = 2mm

Required:

Location of Shear Center?

Solution 2

$$e = \frac{I_f h^2 b^2}{4I}$$

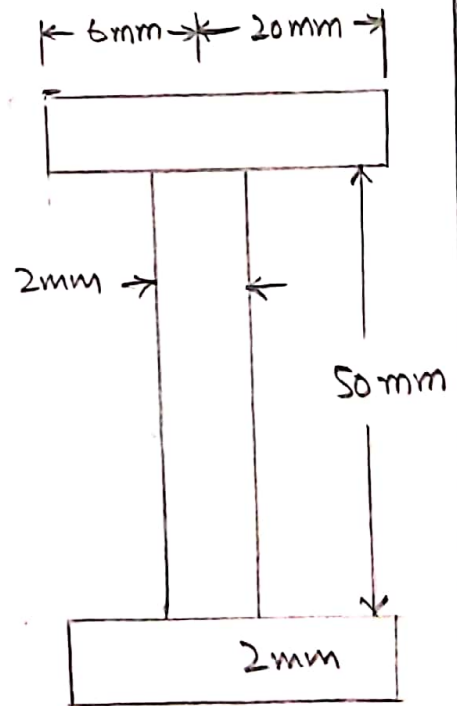
$$I = 2 \left(\frac{26(2)^3}{12} + (26 \times 2)(24)^2 \right) + \left(\frac{2(46)^3}{12} \right)$$

$$I = 29969.33 + 16222.66$$

$$I = 76161.33 \text{ mm}^4$$

$$e = \frac{2(50)^2(26)^2}{4 \times 76161.33}$$

$$e = 11.099 \text{ mm}$$



③

Question No of (b)

Determine the thickness of the wall of a water tank constructed from steel plates filled to a height of 26ft, the circumferential stress is limited to 6000psi the specific weight of water is 62.4 lb/ft^3 .

Given Data

- height of filled tank, $h = 26 \text{ ft}$
- Water tank diameter, $d = 22 \text{ ft}$
- Circumferential stress, $\sigma_t = 6000 \text{ psi}$
- Specific weight of water, $\gamma_w = 62.4 \text{ lb/ft}^3$

Required Data

Thickness, $t = ?$

4

Solution 2

Pressure developed by

$$P = \gamma h$$

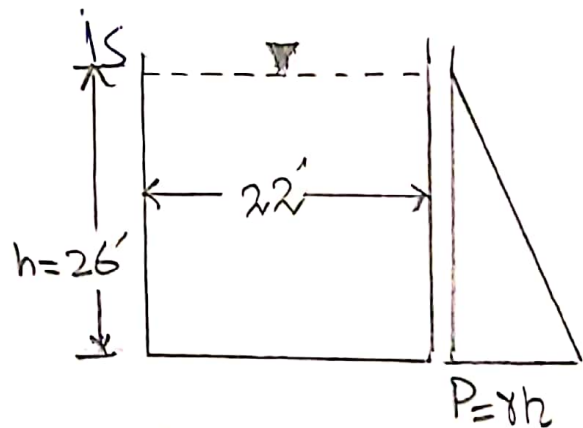
Circumferential stress,

$$\sigma_t = \frac{PD}{2t}$$

$$\sigma_t = \frac{\gamma h D}{2t} = \left\{ P = \gamma h \right\}$$

So thickness

$$t = \frac{\gamma h D}{2 \times \sigma_t}$$



$$t = \frac{62.4}{12^3} \times 26 \times 12 \times 22 \times 12$$

$$2 \times 6000$$

$$t = 0.247 \text{ inch}$$

(5)

Question no 02(a)

The 100×150 mm wooden beam shown in figure 2 is used to support a uniformly distributed load of 4 kN on a simply span of 3 m . The applied load acts in a plane making an angle of 30° with vertical. Calculate the maximum bending stress at mid span and for the same section locate the neutral axis. Neglect the weight of the beam.

Given Data:

Beam Section = $100 \times 150 \text{ mm}$

Uniform load = 4 kN

Span = 3 m

angle with vertical = 30°

③

Required Data:

Maximum bending stress at mid span and for same section locate neutral axis

Solution:

Moment of inertia

$$I_z = \frac{bh^3}{12} = \frac{0.1 \text{ m} (0.15 \text{ m})^3}{12} = 2.8125 \times 10^{-5} \text{ m}^4$$

$$I_y = \frac{bh^3}{12} = \frac{(0.15 \text{ m}) (0.1 \text{ m})^3}{12} = 1.25 \times 10^{-5} \text{ m}^4$$

$$\sigma = \frac{M_z y}{I_z} + \frac{M_y z}{I_y}$$

$$\sigma = \frac{M \cos 30^\circ y}{I_z} + \frac{M \sin 30^\circ z}{I_y}$$

$$M_z = M \cos 30^\circ = 12 \times \cos 30^\circ$$

$$M_z = 10.39 \times 10^3 \text{ N}\cdot\text{m} = 10392.30 \text{ N}\cdot\text{m}$$

$$M_y = M \sin 30^\circ = 12 \times \sin 30^\circ = 6000 \text{ N}\cdot\text{m}$$

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$$G = \frac{10392.3 \times 0.075}{2.8125 \times 10^{-5}} + \frac{6000 \times 0.05}{1.25 \times 10^{-5}}$$

$$G = 51712800 \text{ N/m}^2$$

Sign Convention

2	1
3	4

if we take Compression as negative
tension as +ve and the

↓

2	1
3	4



Quadrant 1, 2, -ve

Quadrant 3, 4, +ve

+	-
+	-

←

8

Quadrant 1, 4, -ve

Quadrant 2, 3, +ve

In Case of Symmetrical loading the neutral axis lie of an angle of " α " the principal axis and the algebraic sum of stress at N.A is zero in this case neutral axis pass through 2, 4

$$\sigma = \frac{M \cos \alpha y}{I_z} + \frac{M \sin \alpha z}{I_y}$$

Let Consider a point "A" on N.A lies in Quadrant "2"
Where

Bending Stress due to $P \cos \alpha$ is compressive

Bending Stress due to $P \sin \alpha$ is tensile

$$0 = \frac{M \cos \alpha y}{I_z} + \frac{M \sin \alpha z}{I_y}$$

(9)

$$0 = -\frac{M \cos \theta_2}{I_2} + \frac{M \sin \theta_2}{I_1}$$

$$\Rightarrow \frac{M \cos \theta_2}{I_2} = \frac{M \sin \theta_2}{I_1}$$

$$\frac{1}{2} = \frac{I_2 M \sin \theta_2}{M \cos \theta_2 I_1} = \frac{I_2 \tan \theta_2}{I_1}$$

$$\Rightarrow \tan \alpha = \frac{I_2 \tan \theta_2}{I_1}$$

$$\Rightarrow \tan \alpha = \frac{I_2 \tan 30^\circ}{I_1}$$

$$\tan \alpha = \frac{2.8125 \times 10^{-5} \tan(30^\circ)}{1.25 \times 10^{-5}}$$

$$\tan \alpha = 1.294$$

$$\alpha = \tan^{-1}(1.294)$$

$$\alpha = 52.2^\circ$$

(b)

Question No. 02 (b) 2

The T Section shown in Figure 3 is the cross section of a simply supported beam 16ft long that carries a central load inclined 60 degree left to the y axis. The centroid is 3.07 in below the top of the section. $I_x = 112.6 \text{ in}^4$ and $I_y = 18.7 \text{ in}^4$. If compressive stress is limited to 12000 psi and tensile stress to 5000 psi. What is the maximum load that will not overstress the beam?

Given Data:

Length = 16ft

angle of load = 60 degree

 $I_x = 112.6 \text{ in}^4$ $I_y = 18.7 \text{ in}^4$

(11)

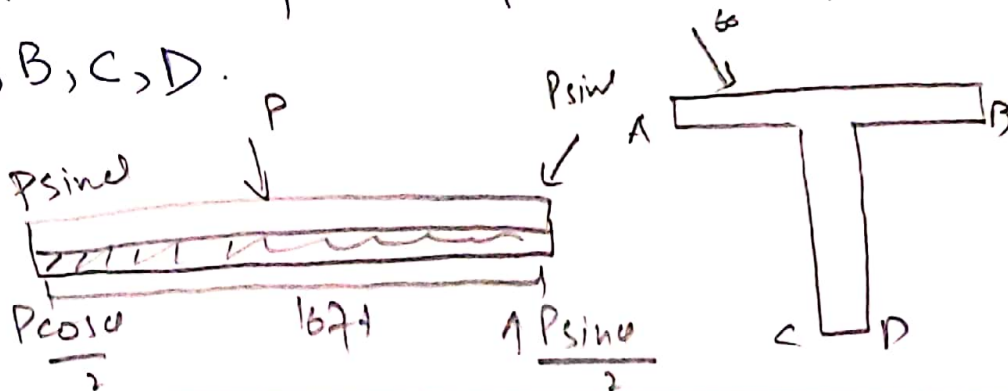
Compressive stress = 12000 psi
tensile stress = 5000 psi

Required Data:

What is maximum load that will not overstress load

Solution:

The maximum Bending stress occur at the mid section due to minimum bending moment so the critical section is the mid section where the compressive and tensile stresses can exceed the limiting values. We also know that at any given section the maximum stress accurate at the extreme fibre for example point A, B, C, D.



(12)

For point load max moment
at mid section

$$M_z = \frac{PL}{4}$$

So,

$$M_x = \frac{(P \cos 60) (16 \times 12)}{4}$$

$$M_y = \frac{(P \sin 60) (16 \times 12)}{4}$$

Stress at A, B, C, D

At A:

$$\sigma = \frac{M_x y}{I_x} + \frac{M_y x}{I_y}$$

$$= \frac{(48) P \cos(60) (3.07)}{112.6} +$$

$$\frac{(48) P (\sin 60) (3)}{18.7}$$

$$= -0.654P + 6.6P$$

$$= 6.01P \text{ Tension}$$

(13)

tension ≤ 5000 psi

$$5000 = 6.01P$$

$$P = \frac{5000}{6.01} = 831.94 \text{ lb}$$

At B

$$6 = - \frac{(48)P \cos(60)(3.07)}{112.6} -$$

$$\frac{(48)(P \sin(60))(3)}{18.7}$$

$$= -0.654P - 6.6$$

$$= -7.2P \text{ Compression}$$

$$12000 = -7.25P$$

$$P = \frac{12000}{7.25} = 1655 \text{ lb}$$

$$1655 \text{ lb} > 831$$

Controlling value is 831.9 lb

(14)

At C₂

$$\sigma = \frac{M_{xy}}{I_x} + \frac{M_{yx}}{I_y}$$

$$= \frac{(48)P \cos(60)(5.93)}{112.6} + \frac{(48)P \sin(60)(0.5)}{18.7}$$

$$= 1.26P + 1.11P$$

$$\sigma = 2.37P$$

tension \leq 5000 psi

$$5000 = 2.37P$$

$$P = \frac{5000}{2.37} = 2109.7 \text{ lb}$$

At D₂

$$\sigma = \frac{(48)P \cos(60)(5.93)}{112.6} -$$

$$\frac{48P \sin(60)(0.5)}{18.7}$$

(15)

$$G = 1.26P - 1.11P$$

$$G = 0.15P \text{ Tensile}$$

$$P = \frac{5000}{0.15} = 33333.33 \text{ lb}$$

So Controlling value is
2109.70 lb

(6)

Question No 03:

A 10ft long strut braced in the middle has a rectangular section of 0.75 in by 2in. A bolt through each end secures the struts so that it acts as a hinged column about an axis perp to the 2in dimension and as a fixed ended column about an axis parallel to 2in dimension. Determine the safe load P about using a factor of safety of 2 and $E = 10.3 \times 10^6$

Given Data:

Length of strut = 10ft

rectangular section = 0.75in x 2in

factor of safety = 2

$E = 10.3 \times 10^6$ psi

(17)

Required Data:

Determine Safe load, $P = ?$

Solution:

Strut is a Compressive member and act as a Column

Case-1:

Strut act as an hinged Column about an axis perp to the 2in dimension

$$I = I_x = \frac{(0.75 \text{ in})(2 \text{ in})^3}{12} = 0.5 \text{ in}^4$$

$$L_e = L \text{ (for hinged ended Column)}$$

$$P_{cr} = \frac{n^2 EI \pi^2}{L_e^2}$$

$$P_{cr} = \frac{(1)^2 (10.3 \times 10^6) (\pi^2) (0.5)}{(10 \times 12)^2}$$

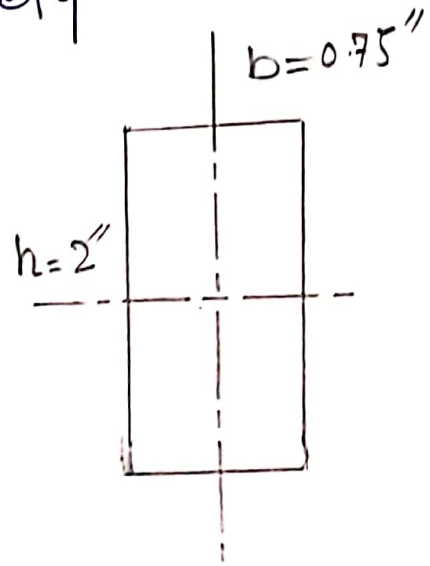
$$P_{cr} = 3529.75 \text{ lb}$$

(18)

$$P_{safe} = \frac{P_{cr}}{\text{factor of Safety}}$$

$$P_{safe} = \frac{3529.75}{2}$$

$$P_{safe} = 1764.87 \text{ lb}$$



Case-2

Strut or Column act as a fixed ended Column about an axis Parallel to 2in dimension

$$I = I_y = \frac{(2)(0.75)^3}{12} = 0.07 \text{ in}^4$$

$$L_e = L/2 \text{ (for fixed ended Column)}$$

then

$$P_{cr} = \frac{n^2 EI \pi^2}{L_e^2}$$

$$= \frac{(1)^2 (10.3 \times 10^6) (0.07) (\pi^2)}{(10 \times 12 / 2)^2}$$

(19)

$$P_{cr} = 1976.66 \text{ lb}$$

$$P_{safe} = \frac{1976.66}{2}$$

$$P_{safe} = 988.33 \text{ lb}$$

In both cases take smaller value
of P_{safe}

$$P_{safe} = (988.33 < 1764.87) \text{ lb}$$