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Section

A

Subject

Hydraulic Engineering

**Question #2**  
**Numerical**  
 Give  $\eta$ -

- $\Rightarrow$  Wide of channel = 3m
- $\Rightarrow$  Discharge =  $Q = 12 \text{ m}^3/\text{sec}$

Reqd-

- $\Rightarrow$  critical depth
- $\Rightarrow$  minimum specific energy
- $\Rightarrow$  alternate depth

Sol: As

**critical depth**

discharge per unit width

$$q = Q/b = 12/3$$

$$q = 4 \text{ m}^3/\text{sec}$$

For rectangular channel

$$h_c = \left(\frac{q^3}{g}\right)^{1/3} = \left(\frac{4^3}{9.81}\right)^{1/3}$$

$$h_c = 1.018 \text{ m}$$

**Minimum specific Energy**

For rectangular channel

$$E_c = \frac{3}{2} h_c = \frac{3}{2} \times 1.018$$

$$E_c = 1.527 \text{ m}$$

**The alternate depth**  $h = 4 \text{ m}$

$E > E_c$ , There are two possible depth for  $q$  given specific energy

$$E = h + \frac{u^2}{2g} \quad \text{where} \quad u = \frac{Q}{A} = \frac{q}{h}$$

(for rectangular channel)

$$E = \frac{h + q^2}{2gh^3}$$

$$4 = \frac{h + 0.8155}{h^3}$$

$$h = 4 - \frac{0.8155}{h^3}$$

For the 'section' subcritical solution the first term, associated with potential energy dominates.

Iteration (zoom e.g.  $h=4$ )

Gives  $h = 3.98\text{m}$ .

For the supercritical solution, the second term, associated with kinetic energy dominates

$$h = \sqrt{\frac{0.8155}{4-h}}$$

Iteration (zoom e.g.  $h=0$ )

Gives  $h = 0.4814\text{m}$

So alternate depths are  $3.85\text{m}$  and  $0.4814\text{m}$ .

## Assignment #02

QNO 2: Numerical

ANS :-

Sol: - AS

first we find the Froude number.

$$Fr = \frac{v}{\sqrt{gy}} = \frac{6 \text{ m/s}}{\sqrt{9.81 \times 0.1}}$$

$$Fr = 6.067 > 1$$

so the flow is supercritical

Alternate depth:-

AS we know that

$$E = y + \frac{v^2}{2g}$$

$$= 0.1 + \frac{0.9}{2 \times 9.81}$$

$$= \boxed{1.935 \text{ m}}$$

The alternate depth for  $E = 1.935 \text{ m}$   
yields  $\boxed{\text{alternate} = 1.93 \text{ m}}$

QNO 2:-  
Numerical

ANS #2:-

Given:-

$$\text{velocity} = v_1 = 2 \text{ m/s}$$

$$\text{depth} = y_1 = 3 \text{ m}$$

$$\text{Elevation } \Delta Z = 60 \text{ cm} = 0.6 \text{ m}$$

$$\text{down step} = 15 \text{ cm} = 0.15 \text{ m}$$

## Solution:-

As we know that

$$E_1 = y_1 + \frac{v_1^2}{2g}$$
$$= 3 + \frac{(2)^2}{2 \times 9.81}$$

$$E_1 = 3.20 \text{ m}$$

Now  $E_2 = E_1 - \Delta Z$

$$= 3.20 - 0.6$$

$$E_2 = 2.60 \text{ m}$$

Also

$$E_2 = y_2 + \frac{v_2^2}{2g y_2^2}$$
$$2.60 = y_2 + \frac{(6)^2}{2 \times 9.81 \cdot y_2^3}$$

$$y_2 = 2.24 \text{ m}$$

$$\Delta y = y_2 - y_1$$
$$= 2.24 - 3$$

$$\Delta y = -0.76 \text{ m}$$

So water surface drops 0.16 m

For a downward step of 15 cm or 0.15 m we have

$$E_2 = E_1 - \Delta Z$$
$$= 3.20 - (-0.15)$$

$$E_2 = 3.35 \text{ m}$$

Now  $y_2 = 3.17 \text{ m}$

$$\Delta y = y_2 - y_1 \\ = 3.17 - 3$$

$$\boxed{\Delta y = 0.17 \text{ m}}$$

So water surface rises = 0.17 m

The maximum upset possible before attaining upstream water surface level is for

$$y_2 = y_c$$

$$y_c = \sqrt[3]{\frac{q^2}{g}}$$

$$= \sqrt[3]{\frac{(0.7)^2}{9.81}}$$

$$\boxed{y_c = 1.54 \text{ m}}$$

# Assignment #03

Q No 1:-  
Given:-

$$y_1 = 3.6 \text{ m}$$

$$b = 3.9 \text{ m}$$

$$y_2 = 0.9 \text{ m}$$

Sol:-

As we know that

$$E_1 = E_2$$

$$y_1 + \frac{v_1^2}{2g} = y_2 + \frac{v_2^2}{2g} \rightarrow (1)$$

Also

$$Q = A_1 v_1 = A_2 v_2$$

$$b_1 y_1 \cdot v_1 = b_2 y_2 \cdot v_2$$

$$b y_1 \cdot v_1 = b y_2 \cdot v_2$$

$$y_1 v_1 = y_2 v_2$$

$$v_2 = \frac{y_1}{y_2} \times v_1$$

$$v_2 = \frac{3.6}{0.9} \times v_1$$

$$v_2 = 4 v_1 \rightarrow (2) \text{ put in eq (1)}$$

$$y_1 + \frac{v_1^2}{2g} = y_2 + \frac{v_2^2}{2g}$$

$$3.6 + \frac{v_1^2}{2(9.81)} = 0.9 + \frac{(4v_1)^2}{2(9.81)}$$

$$3.6 + \frac{v_1^2}{2g} = 0.9 + \frac{16v_1^2}{2g}$$

$$\frac{v_1^2}{2g} - \frac{16v_1^2}{2g} = 0.9 - 3.6$$

$$\frac{v_1^2 - 16v_1^2}{2g} = -2.7$$

$$-15 \frac{v_1^2}{2g} = -2.7$$

$$v_1 = \sqrt{\frac{2.7 \times 2(9.81)}{15}}$$

$$v_1 = 1.879 \text{ m/sec} \quad \text{put in eq (i)}$$

$$v_2 = 4v_1 = 4(1.879) \Rightarrow v_2 = 7.516 \text{ m/sec}$$

AS

$$Q_1 = A_1 v_1 = b y_1 v_1 = 3.9 \times 3.6 \times 1.879$$
$$Q_1 = 26.38 \text{ m}^3/\text{sec}$$

$$Q_2 = A_2 v_2 = b y_2 v_2 = 3.9 \times 0.9 \times 7.516$$
$$Q_2 = 26.38 \text{ m}^3/\text{sec}$$

$$Q = Q_1 = Q_2 = 26.38 \text{ m}^3/\text{sec}$$

2) Froude number  $\rightarrow$  At upstream side

$$F_{r1} = \frac{v_1}{\sqrt{g y_1}} = \frac{1.879}{\sqrt{9.81 \times 3.6}} = 0.31$$

$\downarrow$   
sub-critical flow

2) Froude number  $\rightarrow$  At downstream side

$$F_{r2} = \frac{v_2}{\sqrt{g y_2}} = \frac{7.516}{\sqrt{9.81 \times 0.9}} = 2.52$$

super critical flow