

Mechanics OF Solid

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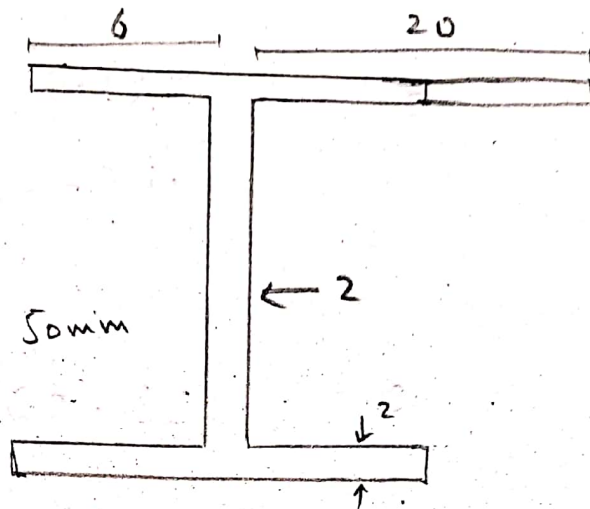
Sec : A

Date : 23/6/2020

Q No 1

(Part - a)

Ans:-



Required:-

location of shear centre.

Sol:-

As we know.

$$e = \frac{h^2 b^2}{4I}$$

and

$$I = 2 \left(\frac{bh^3}{12} + Ay^2 \right)$$

$$+ \left(\frac{bh^3}{12} + Ay^2 \right)$$

$$\Rightarrow 2 \left(\frac{25(2)^3}{12} + (20 \times 2)(25)^2 \right)$$

$$+ \left(2 \left(\frac{(50)^3}{12} + 0 \right) \right)$$

$$I = 50034.66 + 20833$$

$$I = 70867.99 \text{ mm}^4$$

$$e = \frac{2(50)^2(25)^2}{4(70867.99)}$$

$$e = 11.02 \text{ mm}$$

So shear centre

$$e = 11.02$$

$$e = 11.02 \text{ mm}$$

Ans.

Q No 1

(Part - b)

Ans:-

Given Data:-

$$\text{Height} = 26 \text{ ft}$$

$$\text{Tangential stress} = 6000 \text{ psi}$$

$$\text{Specific weight of water} = 62.4 \frac{\text{lb}}{\text{ft}^3}$$

Required:

Thickness of wall of water tank = $t = ?$

Sol:-

$$P = \gamma h$$

$$\sigma_t = \frac{PD}{2t} = \frac{\gamma h \cdot D}{2t}$$

$$t = \frac{\gamma h D}{2\sigma_t}$$

Putting the value

$$t = \frac{\frac{62.4}{(12)^3} (26 \times 10)}{2 (6000)}$$

$$t = \frac{\frac{62.4}{(12)^3} (26 \times 12) (22 \times 12)}{2 (6000)}$$

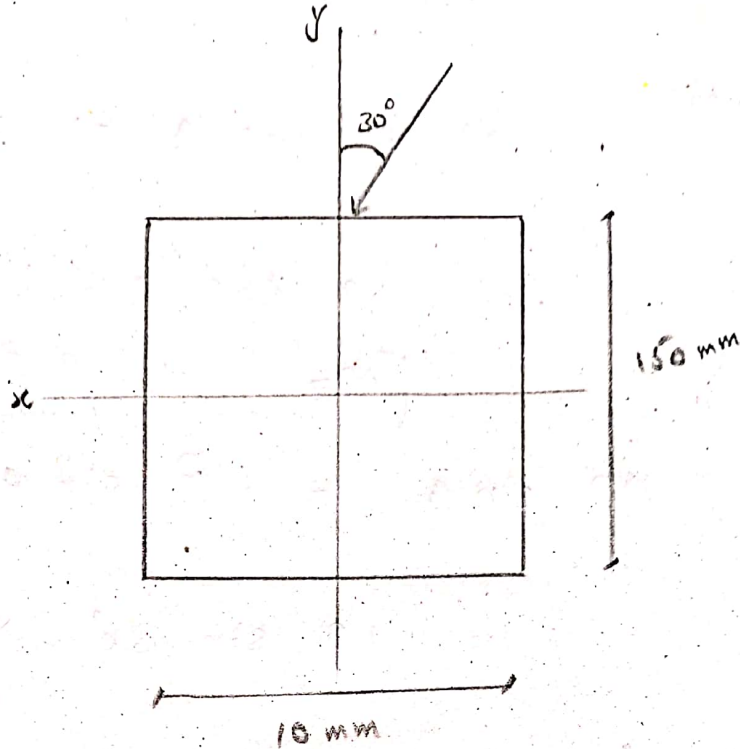
$$t = 0.24 \text{ inch}$$

Ans.

Q No = 2

(Part - 'a')

Ans:



Moment of Inertia

$$I_z = \frac{bh^3}{12} \Rightarrow \frac{0.1 (0.15)^3}{12}$$

$$I_z \Rightarrow 2.8125 \times 10^{-5}$$

Now

$$I_y = \frac{bh^3}{12} \Rightarrow \frac{(0.15) (0.1)^3}{12}$$

$$I_y = 1.25 \times 10^{-5}$$

$$G = \frac{M_z y}{I_z} + \frac{M_y z}{I_y}$$

$$\sigma = \frac{M \cos \theta}{I_z} + \frac{M \sin \theta}{I_y}$$

where

$$M = P \cos \theta \Rightarrow P \cos \theta = M_z$$

$$= 12 \cos 30^\circ$$

$$M_z = 1.8510$$

$$M \sin \theta = P \sin \theta = M_y$$

$$= 12 \sin 30^\circ$$

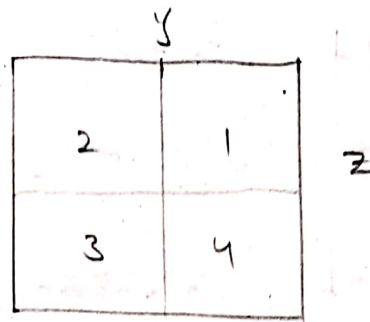
$$M_y = -11.8563$$

$$\sigma = \left(\frac{M_z}{I_z} \right) + \left(\frac{M_y}{I_y} \right)$$

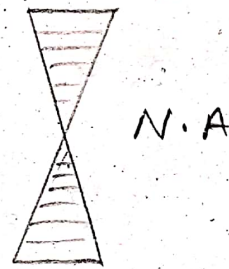
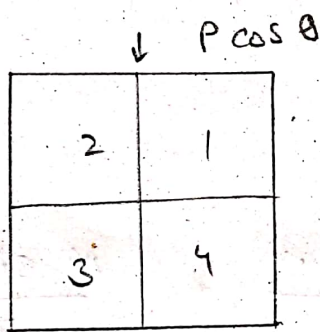
$$\sigma = \frac{1.851}{2.812 \times 10^{-5}} + \left(\frac{-11.8563}{1.25 \times 10^{-5}} \right)$$

$$\sigma = 882678 \text{ N/m}^2$$

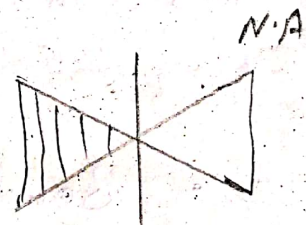
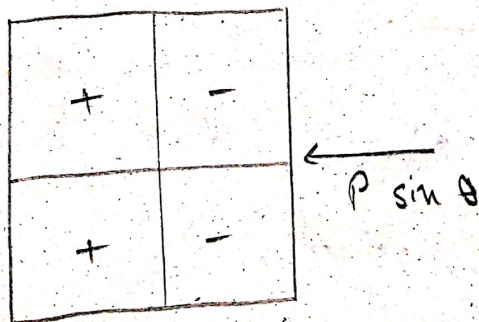
Sign Convention



★ If we take compression as negative and tension as positive and the beam is simply supported.



Quadrant 1, 2 -ive
 " 3, 4 +ive



Quadrant 1, 4 -ive
 " 2, 3 +ive

In case of Unsymmetrical loading the Neutral axis lies at an angle of " θ ". The principle axis and the algebraic sum of stress at N.A is zero.

$$\sigma = \frac{M \cos \theta \cdot y}{I_z} + \frac{M \sin \theta \cdot z}{I_y} \rightarrow \text{①}$$

In this case N.A passes through

z, y

$$\sigma = \frac{M \cos \theta \cdot y}{I_z} + \frac{M \sin \theta \cdot z}{I_y}$$

Let consider a point "A" on N.A lies in Quadrant z, where

- Bending stress due to $P \cos \theta$ is compressive.
- Bending stress due to $P \sin \theta$ is tensile.

eq \rightarrow (1)

$$0 = \frac{-M \cos \theta Y_A}{I_z} + \frac{M \sin \theta Z_A}{I_y}$$

$$\frac{M \cos \theta Y_A}{I_z} = \frac{M \sin \theta Z_A}{I_y}$$

$$\frac{Y_A}{Z_A} = \frac{I_z \sin \theta}{I_y \cos \theta}$$

$$\tan \alpha = \frac{I_z}{I_y} \tan \theta \rightarrow (2)$$

Now

Put values of I_z , I_y and θ in eq (2)

$$\tan \alpha = \frac{I_z}{I_y} \tan 30$$

$$\Rightarrow \frac{2.8125 \times 10^{-5}}{1.25 \times 10^{-5}} \tan 30^\circ$$

$$\tan \alpha = -14.1429$$

6

$$\alpha = \tan^{-1}(-14.4129)$$

$$\alpha = 1.5^\circ$$

$$\alpha = 1^\circ 30' 5''$$

Ans.

Q No 2

(Part - B)

Given Data :

Length of beam = 16 ft

Angle of inclination = 60°

$$I_x = 112.6 \text{ in}^4$$

$$I_y = 18.7 \text{ in}^4$$

$$S_t = 5000 \text{ PSI}$$

$$S_c = 12000 \text{ PSI}$$

Required :-

maximum load = ?

Soln -
?

There will be tension as well as compression which will reduce the effect of each other.

So, we will calculate stress at A and C So,

$$S_A = \frac{Mx}{I_x} + \frac{My}{I_y} \text{ (incompression)}$$

$$\sigma_c = \frac{M_x y}{I_x} + \frac{M_y}{I_y} \quad (\text{Tension}).$$

Now M_x and M_y

$$M_x = P \cos 60 (16 \times 12)$$

$$M_x = 48 P \cos 60.$$

$$M_y = \frac{P \sin 60 (16 \times 12)}{4}$$

$$M_y = 48 P \sin 60.$$

$$\sigma_A = \frac{48 P \cos 60 \times 3.07 + 48 P \sin 60 \times 3.07}{1126 + 18.7}$$

$$P = 1638.6 \text{ lb}$$

Now

$$\sigma_c = \frac{M_x y}{I_x} + \frac{M_y x}{I_y}$$

$$5000 = 48 P \cos 60 + 593 + \frac{48 P \sin 60 \times 0.5}{18.7}$$

$$P = 2104.9 \text{ lb}$$

maximum stress load = 1638.6 lb

Ans

Q No 3

Given Data

$$\text{Length} = 10 \text{ ft}$$

$$E = 10.3 \times 10^6$$

$$b = 0.75$$

$$h = 2$$

$$\text{Factor of safety} = 2$$

Required:-

- a) safe load at hinged = ?
- b) safe load at fixed = ?

Soln:-

a) For hinged column.

$$L_e = L$$

$$I = I_x = \frac{(0.75)(2)^3}{12} = 0.5 \text{ in}^4$$

$$P_{cr} = \frac{\pi^2 EI}{L_e^2} = \frac{(1)^2 (10.3 \times 10^6) (0.5) \pi^2}{(10 \times 12)^2}$$

$$P_{cr} = \frac{50776940}{14400} = 3526.176 \text{ lb}$$

$$P \text{ safe load} = \frac{P_{cr}}{\text{Factor of safety}}$$

$$\frac{3526176}{2} = \boxed{1763.088 \text{ lb}}$$

b, Strat act column.
(For fixed ended)

$$L_e = L/2$$

$$L_e = \frac{10}{2} = 5 \text{ ft.}$$

$$I = I_y = \frac{2 \times (0.75)^3}{12} = 0.07 \text{ in}^4$$

$$P_{cr} = \frac{\pi^2 EI \pi^2}{L_e^2} = \frac{(1)^2 (10.34 \times 10^6) (0.07) (3.14)^2}{(5 \times 12)^2}$$

$$P_{cr} = \frac{7108771.6}{(60)^2}$$

$$P_{cr} = 1974.658 \text{ lb}$$

$$P \text{ safe load} = \frac{1974.658}{2}$$

$$= \boxed{987.3293 \text{ lb}}$$

ANS