

Name = ShahKari ID = 7808

Section

A

Semester

6th

Program

B-Sc Civil Engineering

Assignment

Hydraulic Engineering

submitted to:

Engr. Fawad Ahmad

⌘ IA/SA National University Peshawar ⌘

Assignment # 01

Q 1: What is venturimeter? Explain with detail?

Ans:

A venturimeter is a critical flow open flume with a constricted flow which cause a drop in hydraulic ~~gage~~-grade line creating a critical depth. It is used in flow measurement of very large flow rates, usually give a millions of cubic units. A venturimeter would normally measure a millimeter, whereas a venturimeter flume measure in meters. Measurements of discharge with venturimeters required two measurement one upstream & one at the throat. If the flow passes in a subcritical stage through the flume. If the flume are designed so as to pass the flow from sub to super critical stage while passing through the flume. a single measurement at the throat is sufficient for computation of discharge. To ensure the occurrence of critical depth at the throat, the

flumes are usually designed in such way as to form a hydraulic jump on the downstream side of structure.

Question # 02

A 3-m wide channel carries a total discharge of $12 \text{ m}^3/\text{sec}$. Calculate.

(A) Critical depth

(B) The minimum specific energy.

(C) Alternate depth

where $E = 4 \text{ m}$.

Given data:

$$b = 3 \text{ m}, \quad Q = 12 \text{ m}^3/\text{sec}$$

1) Discharge per unit width:-

$$q = \frac{Q}{b} = \frac{12}{3} = 4 \text{ m}^3/\text{sec}$$

then rectangular channel

$$h_c = \left(\frac{q^2}{g} \right)^{1/2} = \left(\frac{4^2}{9.81} \right)^{1/2} = 1.77 \text{ m}$$

$$\boxed{\text{critical depth} = 1.18 \text{ m}}$$

2) For rectangular channel:

$$E_c = \frac{3}{2} hc \Rightarrow \frac{3}{2} \times 1.17$$

$$E_c = 1.766 \text{ m}$$

$$\boxed{\text{mini specific energy} = 1.77}$$

3) As $E > E_c$; there are two possible depth for a given specific energy:

$$E = h + \frac{v^2}{2g} \quad \text{where} \quad v = \frac{Q}{A} = \frac{q}{h}$$

$$E = h + \frac{q^2}{2gh^2}$$

substituting value in meter-second unit

$$4 = h + \frac{0.8155}{h^2}$$

Now for the subcritical (slow, deep) solution, the first term associated with potential energy

$$h = 4 - \frac{0.8155}{h^2}$$

$$\text{e.g. } h = 4 \quad \text{gives} \quad h = 3.948 \text{ m}$$

⇒ for the supercritical solution

$$h = \sqrt{\frac{0.8155}{4-h}}$$

$$h = 0.4814 \text{ m}$$

$$\boxed{\text{alternate depth are } 3.95 \text{ m \& } 0.481 \text{ m}}$$

Assignment # 02

Q10

water flow at a depth of 10cm with a velocity of 6m/s in a rectangular channel is the flow subcritical or supercritical? what is the alternate depth?

Given data

$$d = 10\text{cm}, \quad v = 6\text{m/s}$$

$$y_{alt} = ?$$

Solution:

By checking Froude number

$$Fr = \frac{v}{\sqrt{gy}} \Rightarrow \frac{6}{\sqrt{9.81 \times 0.1}} = 6.06$$

$$\boxed{Fr = 6.06 > 1} \quad \text{Flow is supercritical}$$

$$E = y + \frac{v^2}{2g} = 0.1 + \frac{6}{2 \times 9.81}$$

$$E = 1.935\text{m}$$

For alternate depth = $E = 1.935\text{m}$

$$\boxed{y_{alt} = 1.93}$$

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QNo2) water flow with a velocity of 2 m/s at a depth of 3 m in a rectangular channel. water is the change in depth y_2 in water surface elevation produced by a gradual upward change in bottom elevation (upstep) of 60 cm ? what would be the depth y_2 Elevation change if there were a gradual down step of 15 cm ? what is max size of up step that could exist before upstream depth changes would result? neglected head loss?

Given data:

$$v_1 = 2\text{ m/s} \Rightarrow y_1 = 3\text{ m}$$

$$\Delta z = 60\text{ cm} \Rightarrow 0.6\text{ m}$$

$$\text{down step} = 15\text{ cm} = 0.15\text{ m}$$

Sol =

$$\begin{aligned} E_1 &= y_1 + \frac{v_1^2}{2g} \\ &= 3 + \frac{2^2}{(2 \times 9.81)} \end{aligned}$$

$$E_1 = 3.20\text{ m}$$

Now

$$\begin{aligned} E_2 &= E_1 - \Delta z \\ &= 3.2 - 0.6 \end{aligned}$$

$$E_2 = 2.60\text{ m}$$

So

$$E_2 = y_2 + \frac{v^2}{2g y_2^2}$$

$$2.60 = y_2 + \frac{6^2}{2 \times 9.81 \times y_2^2}$$

$$\boxed{y_2 = 2.29 \text{ m}}$$

$$\Delta y = y_2 - y_1$$

$$= 2.29 - 3$$

$$\boxed{\Delta y = -0.76 \text{ m}}$$

So water surface drop = 0.76 m

⇒ For a downward step of 15 cm or 0.15 m we have

$$E_2 = E_1 - \Delta z = 3.20 - (-0.15)$$

$$\boxed{E_2 = 3.35 \text{ m}}$$

Now $y_2 = 3.17 \text{ m}$

$$\Delta y = y_2 - y_1 = 3.17 - 3$$

$$\boxed{\Delta y = 0.17 \text{ m}}$$

the water surface rises = 0.17 m

⇒ The max upstep possible before effecting upstream water surface level is for

$$y_2 = y_c$$

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$$y_c = 3 \sqrt{\frac{q^2}{g}}$$

$$y_c = 3 \sqrt{\frac{6^2}{9.18}}$$

$$y_c = 1.54 \text{ m}$$

Assignment # 03

Q1) A water passing from the sluice gate in dam having a depth of water at upstream side is 3.6m after passing through sluice gate the back water curve shows that depth of water at downstream side is 0.9m. The width of sluice gate is 3.9m.

Determine

a) Discharge = Q

b) broad no upstream & downstream.

Given data

$$y_1 = 3.6 \text{ m} = y_2 = 0.9 \text{ m}$$

$$b = 3.9 \text{ m}$$

Solution

$$E_1 = E_2$$

$$y_1 + \frac{v_1^2}{2g} = y_2 + \frac{v_2^2}{2g} \rightarrow (i)$$

NOW

$$Q = A_1 V_1 = A_2 V_2$$

~~$$3.6 \cdot V_1 = 0.9 \cdot V_2$$~~

$$y_1 V_1 = y_2 V_2$$

$$V_2 = \frac{y_1}{y_2} \times V_1$$

$$V_2 = \frac{3.6}{0.9} \times V_1$$

$$\boxed{V_2 = 4V_1} \quad \text{--- (2)}$$

put in eq 4 \rightarrow (1)

$$y_1 + \frac{V_1^2}{2g} = y_2 + \frac{V_2^2}{2g}$$

$$3.6 + \frac{(V_1)^2}{2g} = 0.9 + \frac{(4V_1)^2}{2g}$$

$$\left(\frac{V_1^2}{2g}\right)^2 - \frac{16V_1^2}{2g} = 0.9 - 3.6$$

$$\frac{V_1^2 - 16V_1^2}{2g} = -2.7$$

$$\frac{15V_1^2}{2g} = +2.7$$

$$\sqrt{V_1^2} = \sqrt{\frac{2.7 \times (2 \times 9.81)}{15}}$$

$$\boxed{V_1 = 1.879 \text{ m/sec}}$$

(9)

M = 2808

put in eq (2) we will get

$$v_2 = 4v_1$$

$$v_2 = 4(1.879) = \boxed{7.516 \text{ m/sec}}$$

As

$$Q_1 = A_1 v_1 = b y_1 v_1$$

$$= 3.9 \times 3.6 \times 1.879$$

$$\boxed{Q_1 = 26.38 \text{ m}^3/\text{sec}}$$

$$Q_2 = A_2 v_2 = b y_2 v_2$$

$$= 3.9 \times 0.9 \times 7.516$$

$$\boxed{Q_2 = 26.38 \text{ m}^3/\text{sec}}$$

$$Q_1 = Q_2 = 26.38 \text{ m}^3/\text{sec}$$

1) Froude number at upstream side :-

$$Fr_1 = \frac{v_1}{\sqrt{g y_1}} = \frac{1.879}{\sqrt{9.81 \times 3.6}}$$

$$\boxed{Fr_1 = 0.31}$$

subcritical flow

2) Froude number at downstream ^{side} :-

$$Fr_2 = \frac{v_2}{\sqrt{g y_2}} = \frac{7.516}{\sqrt{9.81 \times 0.9}} = 2.52$$

$$\boxed{Fr_2 = 2.52}$$

← super critical flow.