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MID TERM

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Section; "B"

Semester; 6th

Subject; Geotechnical Foundation

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2

Question # 01

Answer: 1: Plastic Equilibrium:

State of stress within a soil mass or a portion thereof that has been deformed to such an extent that its ultimate shearing resistance is mobilized.

2: Angular Distortion:

When two foundations support wall/columns settle unequally it means that structure is subjected to angular distortion.

3: Compressive Index:

The compressive curve is plotted for effective stress versus void ratio. The void ratio is plotted using normal scale and effective stress is plotted using logarithm scale.

3)

4: Ultimate Bearing capacity:

Ultimate bearing capacity is the theoretical maximum pressure which can be supported without failure; allowable bearing capacity is the ultimate bearing capacity divided by a factor of safety.

5: Poisson Ratio of soil:

Poisson ratio is the negative ratio of transverse to axial strain. The Poisson's ratio of a stable, isotropic, linear elastic material cannot be less than -1.0 nor greater than 0.5 with the latter being a value typically associated with a perfectly incompressible material.

4)

Part B

Answer:

Given data:

$$H = 6\text{m}$$

$$C = 0$$

$$\phi = 30$$

$$\gamma = 19.2 \text{ kN/m}^3$$

$$\text{Slope} = \begin{array}{l} \text{Horizontal} = 1 \\ \text{vertical} = 3 \end{array}$$

Required:

$$\frac{Pa}{n} = ?$$

$$\frac{Va}{b} = ?$$

Solution:

$$\frac{Pa}{b} = \frac{\gamma \times H^2 \times Ka}{2}$$

$$B = \tan B \left(\frac{1}{3} \right)$$

5

$$B = 180^\circ$$

$$k_a = \cos B \times \frac{\cos B - \sqrt{\cos^2 B - \cos^2 \theta}}{\cos B + \sqrt{\cos^2 B - \cos^2 \theta}}$$

$$k_a = \cos(18) \times \frac{\cos(18) - \sqrt{\cos^2(18) - \cos^2(30)}}{\cos(18) + \sqrt{\cos^2(18) - \cos^2(30)}}$$

$$k_a = 0.3948$$

$$|k_a| = 0.395$$

now

$$\frac{p_a}{b} = \frac{19.2 \times (b)^2 \times 0.395}{2} = 136.512 \text{ kN/m}$$

$$\frac{n_a}{b} = \frac{p_a}{b} \times \cos B$$

$$\frac{n_a}{b} = 136.512 \times \cos(18)$$

$$\boxed{\frac{n_a}{b} = 129.83 \text{ kN/m}}$$

$$\frac{v_a}{b} = \frac{p_a}{b} \sin B$$

$$\frac{v_a}{b} = 136.512 \times \cos(18)$$

$$\boxed{\frac{v_a}{b} = 42.18 \text{ kN/m}}$$

b)

Question # 02

Part (a)

Answer: Bearing capacity:

In Geotechnical Engineering bearing capacity is defined as the engineering property of the soil due to which it resists the applied ~~force~~ load.

It is denoted by q_u .

In other words the internal strength of the soil is called bearing capacity.

Factors Affecting Bearing capacity:

1: Relative density of the soil:

More the relative density of the soil more will be its angle of friction. More will be the ϕ_u, ϕ_c, ϕ_i with increase of this (ϕ_u, ϕ_c, ϕ_i) the bearing capacity will increase.

This will increase more for dense soil/sand as compared with medium and loose. ϕ_u, ϕ_c, ϕ_i Terzaghi Bearing capacity Factors.

7)

2: Depth of the Footing:

When the increase of depth (d_b) of the foundation the bearing capacity of soil will increase.

This increase will be more in case of dense sand/soil as compared with loose or medium sand/soil.

3: Breadth of the Foundation:

More the breadth of foundation more will be the bearing capacity of soil.

It will be more in case of dense soil/sand as compared with loose or medium soil/sand.

4: Unit weight of soil:

Bearing capacity of soil is directly proportional to unit weight of soil sand. The bearing capacity of soil increase with increase in its unit weight.

It will be more in case of dense soil/sand as compared with loose or medium soil/sand.

8)

5: Water Table:

As water table comes near to footing, the bearing capacity get decreases.

Part B

Answer:

Given data:

$$L = 3\text{m}$$

$$B = 2\text{m}$$

$$D_f = 1.6\text{m}$$

$$F.O.S = 3$$

$$\gamma = 18\text{kN/m}^3$$

$$C = 20\text{kN/m}^2$$

$$\phi = 20^\circ$$

Required

$$Q_s = ?$$

Solution:

$$Q_u = C \cdot N_c \cdot S_c \cdot d_c + \gamma \cdot N_q \cdot S_q \cdot d_q + \frac{1}{2} \cdot \gamma \cdot B \cdot N_\phi \cdot S_\phi \cdot d_\phi$$

9)

First bot the slope Factor.

$$\begin{aligned}\alpha &= \left[45 + \frac{0}{2} \right] \\ &= \left[45 + \frac{20}{2} \right] \\ &= 55^\circ\end{aligned}$$

$$\begin{aligned}S_c &= 1 + 0.2 \frac{B}{L} \tan^2 \alpha \\ &= 1 + 0.2 \left(\frac{2}{3} \right) \tan^2 55 \\ &= 1.27 \approx 1.3\end{aligned}$$

$$\begin{aligned}S_q = S_r &= 1 + 0.1 \frac{B}{L} \tan^2 \alpha \\ &= 1 + 0.1 \frac{2}{3} \tan^2 55 \\ &= 1.14\end{aligned}$$

Depth Factor

$$\begin{aligned}d_c &= 1 + 0.2 \frac{D}{B} \tan \alpha \\ &= 1 + 0.2 \frac{1.6}{2} \tan 55 \\ &= 1.23\end{aligned}$$

$$\begin{aligned}d_i = d_q &= 1 + 0.1 \frac{D}{B} \tan \alpha \\ &= 1 + 0.1 \left(\frac{1.6}{2} \right) \tan 55 \\ &= 1.11\end{aligned}$$

10

now putting values

$$q_{ju} = (n_c \cdot s_c \cdot d_c \cdot i_c + q_{ur} \cdot q_j \cdot d_{qj} \cdot s_{qj} \cdot i_{qj} + \frac{1}{2} \gamma_{nh} \cdot s_1 \cdot d_1 \cdot n_1) \\ = (20 \times 14.8 \times 1.3 \times 1.23 \times 1) \times (18 \times 1.6 \times 6.4 \times 1.11 \times 1.11 \times 1) \\ + (0.5 \times 20 \times 2 \times 2.9 \times 1.11 \times 1.14 \times 1)$$

$$q_{ju} = 762 \text{ kN/m}^2$$

$$q_{jn.u} = q_{ju} - \bar{j} \\ = 762 - (18 \times 1.6) \\ = 733.2 \text{ kN/m}^2$$

$$q_{jn.s} = \frac{q_{jn.u}}{F.O.S} = \frac{733.2}{3} = 244.4$$

$$q_{js} = q_{jn.s} + \bar{s} \\ = 244.4 + (18 \times 1.6) \\ = 273.2 \text{ kN/m}^2$$

Total safe load on rectangular footing

$$A \times q_{js} = (2 \times 3) \times 273.2$$

$$= 1639.2 \text{ kN}$$

11

Question # 03

Part "a"

Answer: SETTLEMENT:

When load is applied on the ground surface this will produce effective vertical stresses, due to these stresses the effective vertical strain will be produced as result of which the moment will occur in the downward direction. This downward moment is called settlement.

Types of Settlement:

on the basis of movement of the structure it is divided into two types.

1: Total Settlement.

2: Differential Settlement.

1: Total Settlement:

It is also called uniform settlement. In this type of settlement each part of ~~settlement~~ structure will settle equally.

12

In uniform settlement the behavior of the structure is not much as considered as with the differential settlement.

The total settlement mostly take place in the structure which are constructed in rigid bearing. In this type of settlement the utility services such as water supply, electricity, sewage line, telephone etc may be decreased and the structure will remain sound.

Limitation For Uniform / Total Settlement:

The soil layer to which the load is to be transfer should sufficient in bearing to ~~exist~~ resist the load which is to be applied on it.

To spread the coming load over a large area.

2: Differential Settlement:

Different settlement in different parts of the same structure is called differential settlement.

Differential settlement is more

13

Undesirable as compared with total/uniform settlement, because it causes more damage to a structure as compared to total/uniform settlement.

Types of Differential Settlement:

Differential settlement is of two types.

1: Tilt

2: Angular Distortion.

part "B"

Answer:

Given data:

$$C_c = 0.31$$

$$P_1 = 130 \text{ kN/m}^2$$

$$P_2 = 170 \text{ kN/m}^2$$

$$C_0 = 1.02$$

$$H = 5 \text{ m}$$

Required:

$$e_1 = ?$$

$$s_c = ?$$

14

Solution:

$$C_c = \frac{\Delta c}{\log\left(\frac{P_2}{P_1}\right)}$$

$$\frac{e_0 - e_1}{\log_{10}\left(\frac{P_2}{P_1}\right)}$$

$$0.31 = \frac{1.02 - e_1}{\log_{10}\left(\frac{170}{130}\right)}$$

$$e_1 = 0.983$$

Now

$$S_c = \frac{H}{1+Se} \times C_c \log_{10}\left(\frac{P_2}{P_1}\right)$$

$$= \frac{5}{1+0.2} \times 0.31 \log_{10}\left(\frac{170}{130}\right) \times 1000$$

$$= 2.47 \times 0.03611 \times 1000$$

$$= 0.08920 \times 1000$$

$$S_c = 89.39 \text{ mm}$$