

I.D :- 7278

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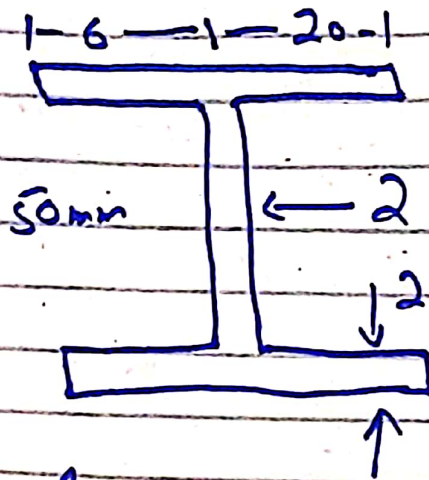
PAPER :- Mas II

SUBMITTED TO :-

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DATE :-

23/06/2020

Q. 1  
(a)

Required:-

location of shear center

Sol:-

As we know that

$$e = \frac{b^2 h^2 b^2}{4I}$$

$$I = 2 \left[ \frac{bh^3}{12} + Ay^2 \right] + \left[ \frac{by^3}{12} + Ay^2 \right]$$

$$I = 2 \left[ \frac{26(2)^3}{12} + (20 \times 2)(25)^2 \right] + \left[ \frac{2(50)^3}{12} + 0 \right]$$

$$I = 50034.66 + 20833$$

$$I = 70867.99 \text{ mm}^4$$

$$e = \frac{2(50)^2(25)^2}{4(70867.99)} = 11.02 \text{ mm}$$

So shear center  $e = 11.02 \text{ mm}$

Q No 1  
Part (b)

GIVEN DATA →

$$\Rightarrow H = 26 \text{ ft}$$

⇒ assume diameter

$$D = 22 \text{ ft}$$

$$\Rightarrow \text{tangential stress} = 600 \text{ lb/ft}^2$$

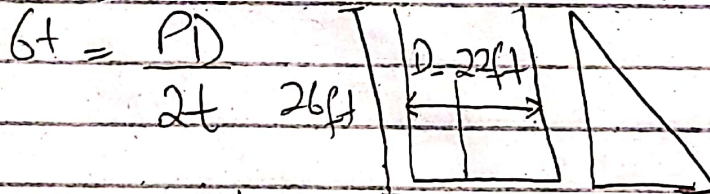
$$\Rightarrow \text{Specific weight of water tank} = \gamma_{\text{tank}} = 62.4 \text{ lb/ft}^3$$

we have to find thickness = ?

**SOLUTION:** →

The pressure develop by water  $P = \gamma h$

$$\sigma_t = \frac{PD}{2t}$$



$$\sigma_t = \frac{PD}{2t} = \frac{\gamma h D}{2t}$$

$$2t \times \sigma_t = \gamma h D$$

$$2t = \frac{\gamma h D}{\sigma_t}$$

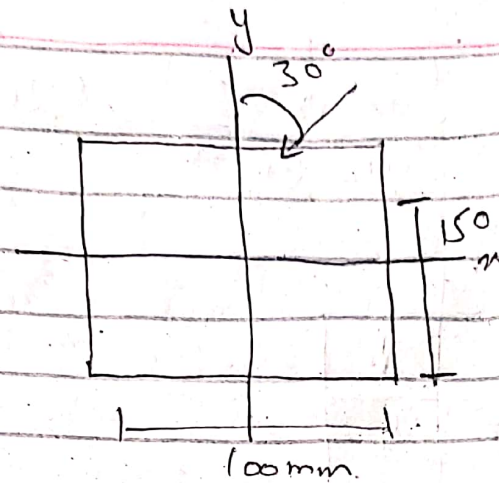
$$t = \frac{\gamma h D}{\sigma_t \times 2}$$

Pg ③

$$t = \frac{(62.4) \times (26 \times 12) \times (22 \times 12)}{(12)^3 \times 6000 \times 2}$$

$$t = 0.24''$$

Q2(a)



Moment of Inertia

$$I_z = \frac{bh^3}{12} = \frac{0.1 (0.15)^3}{12} = I_z = 2.8125 \times 10^{-5}$$

Now

$$I_y = 1.25 \times 10^{-5}$$

$$\delta = \frac{M_z y}{I_z} + \frac{M_y z}{I_y}$$

$$\delta = \frac{M \cos \theta}{I_z} + \frac{M \sin \theta}{I_y}$$

where

$$M \cos \theta = P \cos \theta = M_z$$

$$= 12 \cos 60^\circ = M_z$$

$$M_z = 1.8510$$

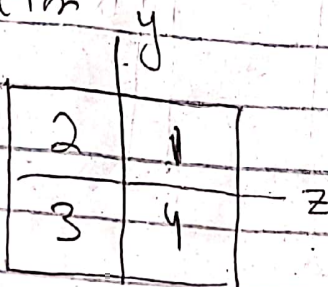
$$M \sin \theta = P \sin \theta = M_y$$

$$M_y = -11.8563$$

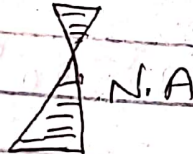
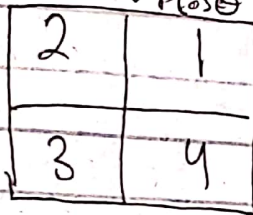
$$\delta = \left( \frac{M_z}{I_z} \right) + \left( \frac{M_y}{I_y} \right)$$

$$b = \left[ \frac{1.851}{2.812 \times 10^6} \right] + \left[ \frac{-11.8563}{1.25 \times 10^5} \right] = 825628 \text{ Nm}^2$$

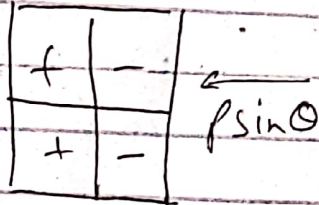
Sign convention



If we have compression as negative  $\epsilon$  tension as positive  $\epsilon$  the beam is a simply supported.



Quadrant 1, 2 - ve  
Quadrant 3, 4 +ve



Quadrant 1, 4 - ve  
Quadrant 2, 3 +ve

Case of unsymmetrical loading the neutral axis lies at an angle of  $\alpha$ . The principle axis  $\epsilon$  the algebraic sum of stress at N.A. is zero.

$$b = \frac{m \cos \theta}{I_z} + \frac{M \sin \theta}{I_y} \quad \text{--- (1)}$$

in the case N.A. passes through 2, 4, 2, 0

$$C = \frac{M \cos \theta}{I_z} + \frac{M \sin \theta}{I_y} \quad \text{---}$$

Let consider point "A" on N/A lies  
Quadrant 2, where

• Bending stress due to  $P \cos \theta$  is  
compressive

• Bending  $\epsilon$  stress due to  $P \sin \theta$  is  
Tensile

$$\text{eq ①} \Rightarrow 0 = \frac{-M \cos \theta y_A}{I_z} + \frac{m \sin \theta}{I_y} z_A$$

$$\Rightarrow 0 = \frac{m \cos \theta y_A}{I_z} + \frac{m \sin \theta}{I_y} z_A$$

$$\Rightarrow \frac{m \cos \theta y_A}{I_z} + \frac{m \sin \theta}{I_y} z_A$$

$$\frac{y_A}{z_A} = \frac{I_z \sin \theta}{I_y \cos \theta} = \tan \alpha \frac{I_z}{I_y} \tan \theta \quad \text{--- ②}$$

Now put value of  $I_z, I_y$  &  $\theta$  in eq ②

$$\tan \alpha = \frac{2.8125 \times 10^{-5}}{1.25 \times 10^{-5}} \quad (\tan 30^\circ)$$

$$\tan \alpha = -14.4129$$

$$\alpha = \tan^{-1} (-14.4129)$$

$$\alpha = 1.5^\circ$$

$$\alpha = 1^\circ 3' 05''$$

Q2(b)

Ans (b)  
GIVEN

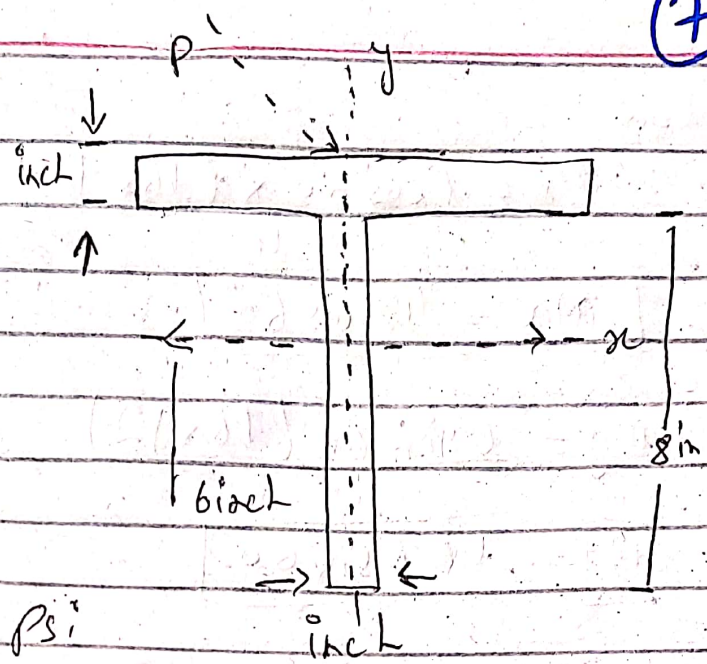
$C = 16 \text{ ft}$

$I_x = 11.26 \text{ in}^4$

$I_y = 18.7 \text{ in}^4$

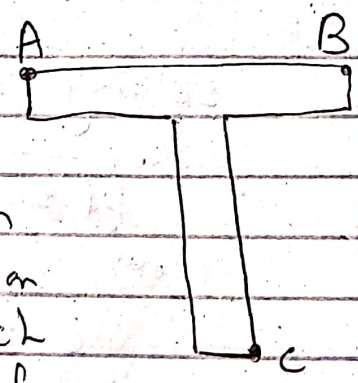
$\sigma_c = 12000 \text{ psi}$

$\sigma_t = 5000 \text{ psi}$



Sol:

By looking to the figure, we can judge that maximum compression would on A & maximum tension at B these will be tension as well as compression which will reduce the effects of each other so we will calculate stresses at A & C.

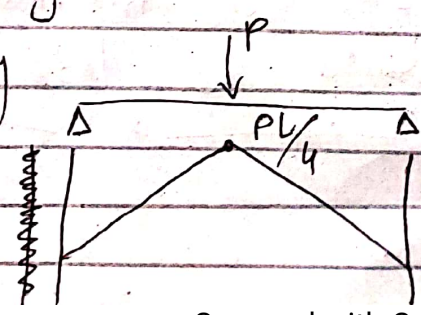


So

$\sigma_A = \frac{mxy}{I_x} + \frac{myx}{I_y}$  (comp)

$\sigma_C = \frac{mxy}{I_x} + \frac{myx}{I_y}$  (tension)

Now  $M_x$  &  $M_y$





So

$$M_x = \frac{P \cos 60 \times (16 \times 12)}{4}$$

$$M_x = 48 \cos 60^\circ$$

$$M_y = \frac{P \sin 60 (16 \times 12)}{4}$$

$$M_y = 48 P \sin 60$$

Now

$$\sigma_A = \frac{m_x y}{I_x} + \frac{m_y x}{I_y}$$

$$12000 = \frac{48 P \cos 60^\circ \times 3.07}{112.6} + \frac{48 P \sin 60^\circ \times 3.18}{18.7}$$

Solving the equation

$$P = 1638.6 \text{ lb}$$

Now

$$\sigma_C = \frac{m_x y}{I_x} + \frac{m_y x}{I_y}$$

$$5000 = \frac{48 \cos 60 (5.93)}{112.6} + \frac{48 \sin 60 \times 0.5}{18.7}$$

Solving equation

$$P = 2104.9 \text{ lb}$$

So the maximum load  $P$  applied should be 1638.6 lb.

Q No 3

GIVEN DATA —

length  $l = 10 \text{ ft}$   
 As both sides are hinged

So

$$l_e = l$$

$$E = 10.3 \times 10^6$$

factor of safety = 2

$$b = 0.75 \text{ inch}$$

$$h = 2 \text{ inch}$$

Req: —

Determine safe load = ?

Sol: —

As

$$P_{cr} = \frac{\pi^2 EI}{l_e^2}$$

As we know that  $I = A\gamma^2$

$$I = A\gamma^2$$

$$\gamma = \sqrt{\frac{I}{A}}$$

$$\gamma = \sqrt{\frac{hb^3}{12}} = \sqrt{\frac{b^2}{12}}$$

$$\gamma = \frac{b}{2\sqrt{3}} \Rightarrow \frac{0.75}{2\sqrt{3}}$$

$$\gamma = 0.216 \text{ inch}$$

$$P_{cr} = \frac{\pi^2 EA}{(l_e/\delta)^2}$$

$$\Rightarrow \frac{(3.14)^2 (10.3 \times 10^6) (1.5)}{(10/0.216)^2}$$

$$P_{cr} = 853.8343$$

$$\text{safe load} = \frac{\text{Crippling load}}{\text{factor of safety}}$$

$$\Rightarrow \frac{853.8343}{2}$$

$$\text{safe load} \Rightarrow 426.917$$

for fixed ended column  
 $l_e = \frac{l}{2} = \frac{10}{2}$

$$l_e = 5 \text{ ft}$$

$$P_{cr} = \frac{\pi^2 EA}{(l_e/\delta)^2}$$

$$= \frac{(3.14)^2 (10.3 \times 10^6) (1.5)}{(60/0.216)^2}$$

$$P_{cr} = 1974.207$$

Pg ⑪

$$\text{safe load} = P_{cr}$$

factor of ~~safety~~  
safety

$$= \frac{1974.207}{2}$$

$$= 987.103$$