

UMAR HADI

ID # 7974

Section # B

Subject # Structural Analysis-I

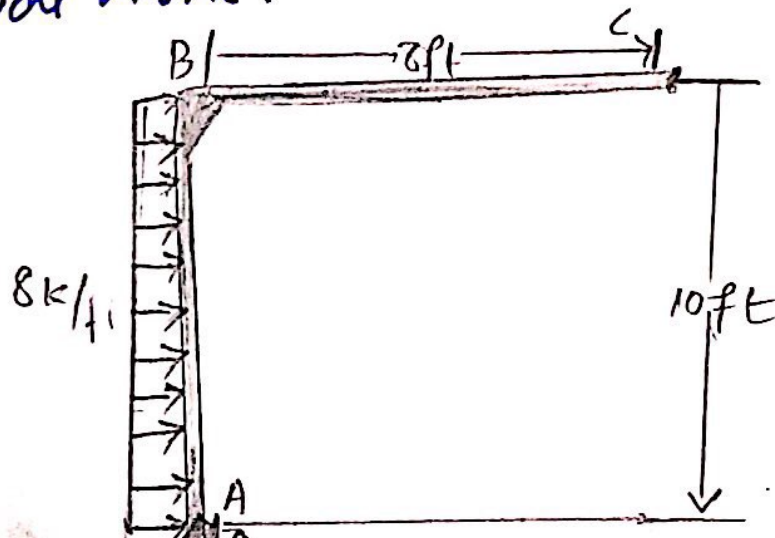
Submitted to Amjad Islam

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## QUESTION # 02

Determine the vertical displacement of free end point C on the frame shown in figure Take  $E = 29(10^3)$  ksi . . . . . Use Method of Virtual Work.



$$A_x = 40 \text{ kip} \quad A_y = 25 \text{ kip}$$

Solution :-

Given Data :-  $\Delta_B = ?$   
 $E = 29 \times 10^3 \text{ ksi}$   
 $I = 600 \text{ in}^4$

Finding Reaction:

$$\sum M_A = 0$$

$$-4(10)(5) + C_y(8) = 0$$

2)

$$\sum F_y = 25 \text{ kip}$$

$$\sum F_y = 0 \uparrow^+$$

$$25 + A_y = 0$$

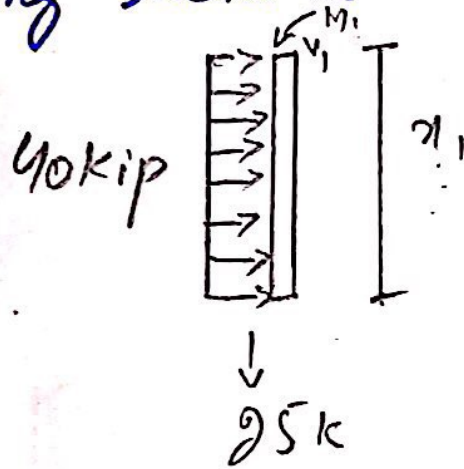
$$A_y = -25 \text{ kips}$$

$$\sum F_x = 0 \rightarrow^+$$

$$40 - A_x = 0$$

$$A_x = 40 \text{ k}$$

⇒ Taking sections -



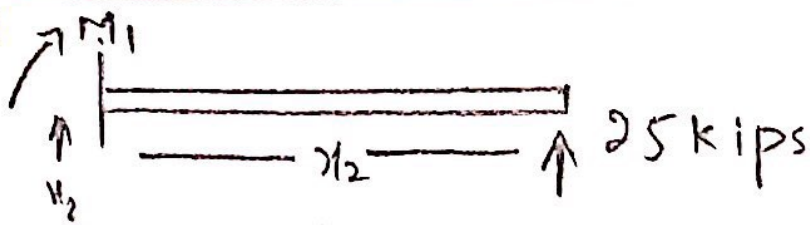
⇒ Real moment:

$$\sum M_1 = 0$$

$$-40(x_1) + 4x_1(x_1/2) + x_1 = 0$$

$$M_1 = 40x_1 - 2x_1$$

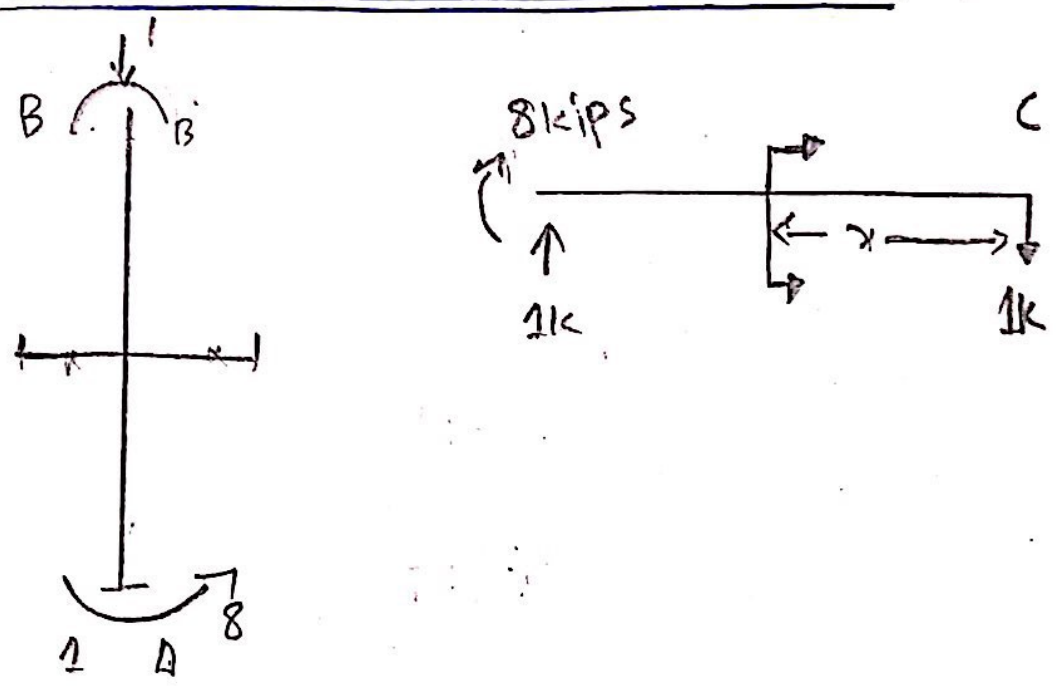
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$$-25(x_2) + M_2 = 0$$

$$M_2 = 25x_2 \text{ kips}$$

=> Now vertical Moments:



Members	BA	CB
Origin	B	C
limit	0-10	0-8
M	$2x^2$	0
m	8	11

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: By Vertical work Method:

$$1. \Delta_1 = \int_0^{10} \frac{(2x^2)(8) dx}{EI} + \int_0^8 \frac{(0)(x)}{EI}$$

$$= \frac{16x^3}{3} \Big|_0^{10} + 0$$

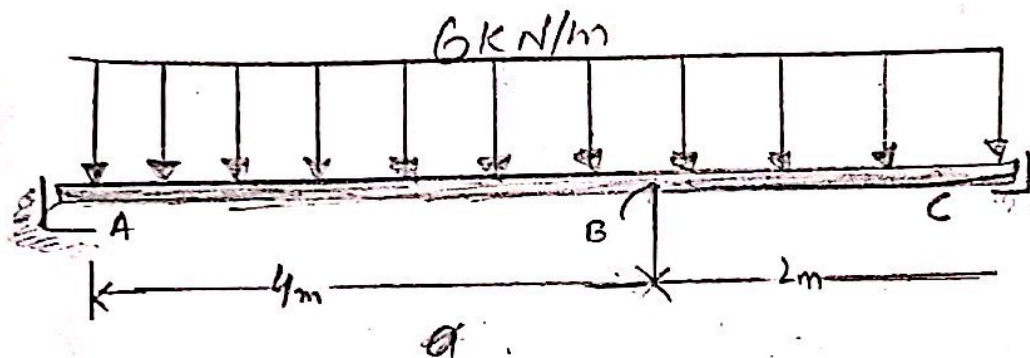
$$= \frac{16 \times 1000}{3} / EI$$

$$= \frac{5333.33}{EI} = \frac{5333.33}{29 \times 10^3 \times 600}$$

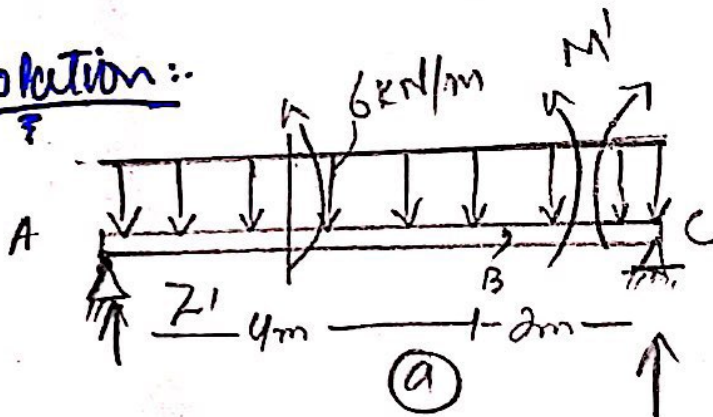
$$\Delta_1 = 3.06 \times 10^{-4} \text{ in}$$

## QUESTION # 03

Determine the slope and displacement at point B. Assume the support at A is a pin and C is a roller. Take  $E = 200 \text{ GPa}$ ,  $I = 60(10)^6 \text{ mm}^4$ . Use Castiglione's Theorem.



Solution:



$$18 \text{ kN} + 0.1667$$

$$18 - 0.1667$$

$$R_1 + R_2 = 0$$

$$\sum M_A = 0 \quad \text{G} +$$

$$4 + R_2 (6) = 0$$

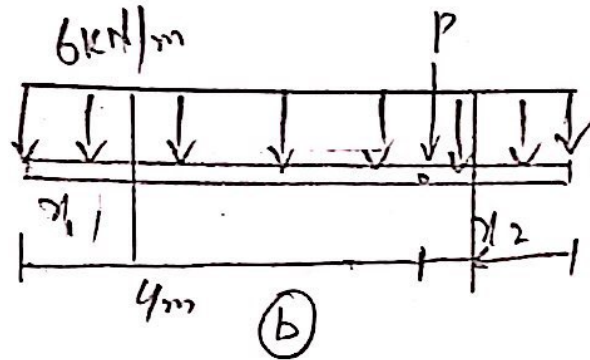
$$\Rightarrow -0.16667 \text{ put in } \textcircled{1}$$

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$$R_1 + (-0.1667) = 0$$

$$R_1 - (0.1667) = 0$$

$$R_1 = 0.16667 \text{ kN}$$



$$R_1 + R_2 = 1$$

$$\sum M_A = 0$$

$$-(1)(4) + R_2(6) = 0$$

$$R_1 = 0.667 \text{ kN}$$

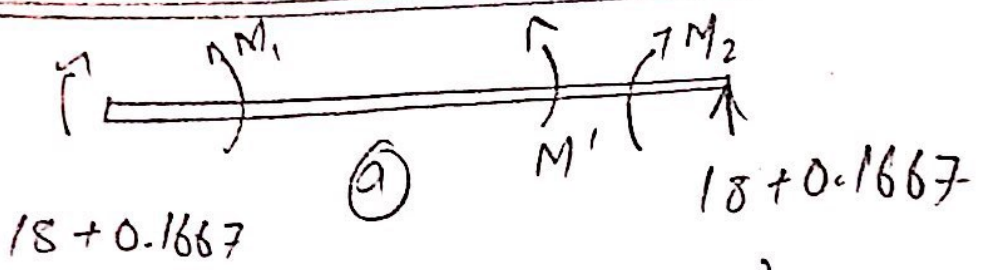
$$R_2 = 1 - 0.6667 \text{ kN}$$

$$R_2 = 0.333 \text{ kN}$$

$$M_1 = (18 + 0.1667 M') x_1 - \frac{1}{2} x_1^2$$

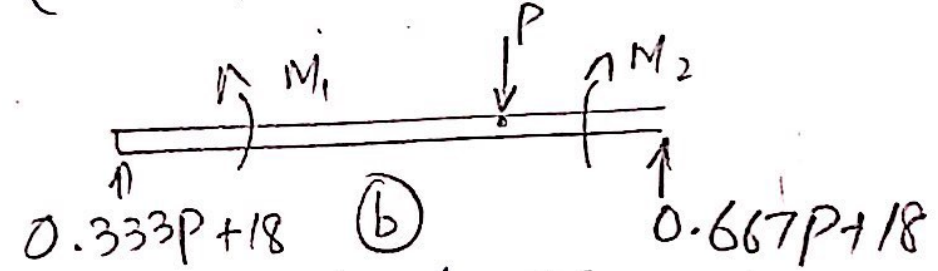
$$M_2 = (18 - 0.1667 M') x_2 - \frac{1}{2} x_2^2$$

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$$M_1 = (0.333P + 18)x_1 - \frac{1}{2}x_1^2$$

$$M_2 = (0.667P + 18)x_2 - \frac{1}{2}x_2^2$$



The displacement function shown in the figure 'a' above

$$\frac{\partial M_1}{\partial M'} = 0.1667x_1 \text{ and } \frac{\partial M_2}{\partial M'} = 0.1667x_2$$

Let  $M' = 0$ , then

$$M_1 = (18 + 0.1667(0))x_1 - \frac{1}{2}x_1^2$$

$$\rightarrow M_1 = (18x_1 - \frac{1}{2}x_1^2)$$

$$\rightarrow M_2 = (18x_2 - \frac{1}{2}x_2^2)$$

$$U_B = \int_0^L M \left( \frac{\partial M}{\partial M'} \right) \frac{dx}{EI}$$



4)

$$= \int_0^{4'} \frac{(18x_1 - 2x_1^2)(0.1667x_1)}{EI} dx_1 + \int_0^2 \frac{(18x_2 - 2x_2^2)(0.1667x_2)}{EI} dx_2$$

$$Q_B = \frac{42.65}{EI} + \frac{6.66}{EI}$$

$$Q_B = \frac{49.31}{EI}$$

$$Q_B = \frac{49.31}{(200 \times 10^6 \text{ kPa})(0.00006)}$$

$$Q_B = 0.441 \text{ rad}$$

→ For the displacement functions are shown in figure "b"

$$\frac{\partial M_1}{\partial M_P} = 0.333x_1, \text{ and } \frac{\partial M_2}{\partial P} = 0.667x_2$$

also set  $p=0$

$$\text{then } M_1 = (18x_1 - 2x_1^2) \text{ kN-m}$$

$$M_2 = (18x_2 - 2x_2^2) \text{ kN-m}$$

$$\text{thus } \Delta_B = \int_b^L M \left( \frac{\partial M}{\partial P} \right) \left( \frac{d\pi}{EI} \right)$$

$$\Delta_B = \int_0^4 \frac{(30x_1 - 2x_1^2)(0.333x_1) dx_1}{EI}$$

$$+ \int_0^2 \frac{(30x_2 - 2x_2^2)(0.6667x_2) dx_2}{EI}$$

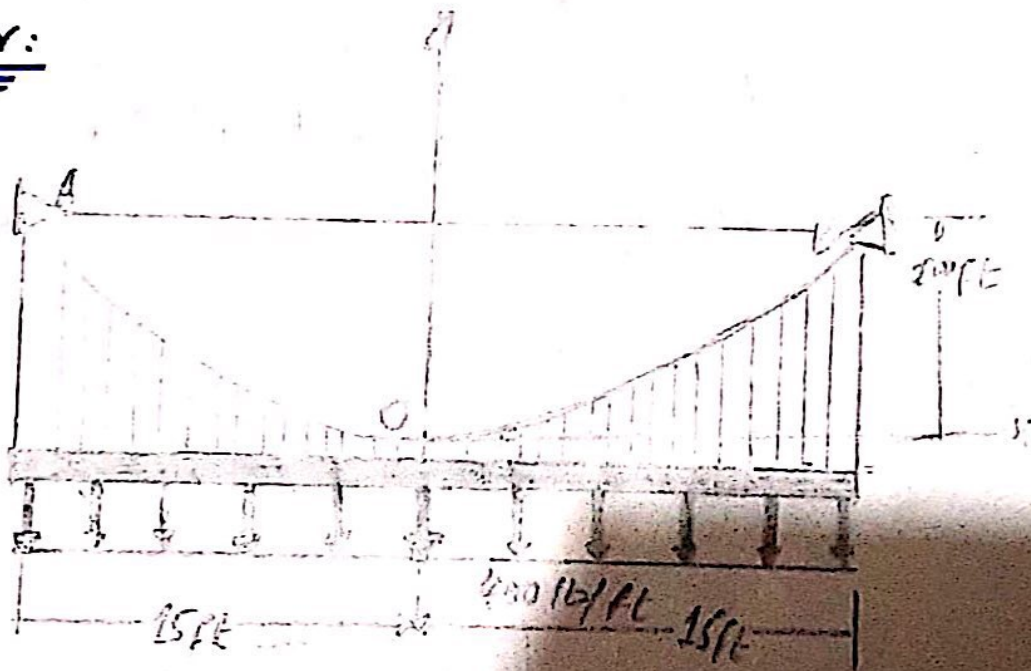
$$\Delta_B = \frac{\partial 18.5}{EI} \Rightarrow \frac{\partial 18.5}{(200 \times 10^6)(0.00006)}$$

$$\Delta_B = 0.018 \text{ m or } 18 \text{ mm}$$

QUESTION # 04.

The cable is subjected to the uniform loading. If the slope of the cable at point O is zero, determine the equation of the curve and the force in the cable at "O" and "B".

Answer:



⇒ As we know that

$$y = \frac{h}{L^2} x^2$$

$$y = \frac{10}{(15)^2} x^2$$

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$$y = 0.0444x^2$$

Now

$$\therefore T_0 = Fu = \frac{w_0 L^2}{2h}$$

$$= \frac{400(15)^2}{2(10)}$$

$$T_0 = 4500 \text{ lb}$$

$\therefore$  Divide by 1000

$$T_0 = 4.5 \text{ k}$$

Now

$$\therefore T_B = T_{\text{max}} = \sqrt{Fu^2 + (w_0 L)^2}$$

$$T_{\text{max}} = \sqrt{(4500)^2 + (400)^2 (15)^2}$$

$$T_{\text{max}} = \sqrt{20250000 +}$$

$$T_{\text{max}} = 7500 \text{ lb}$$

$\therefore$  divide by

1000

$$T_B = T_{\text{max}} = 7.5 \text{ k}$$

② Also

$$T_B = T_{max} = w_0 L \sqrt{1 + \left(\frac{L}{2h}\right)^2}$$

$$= 400(15) \sqrt{1 + \left(\frac{15}{2 \times 10}\right)^2}$$

$$= 6000 \sqrt{1 + \frac{225}{400}}$$

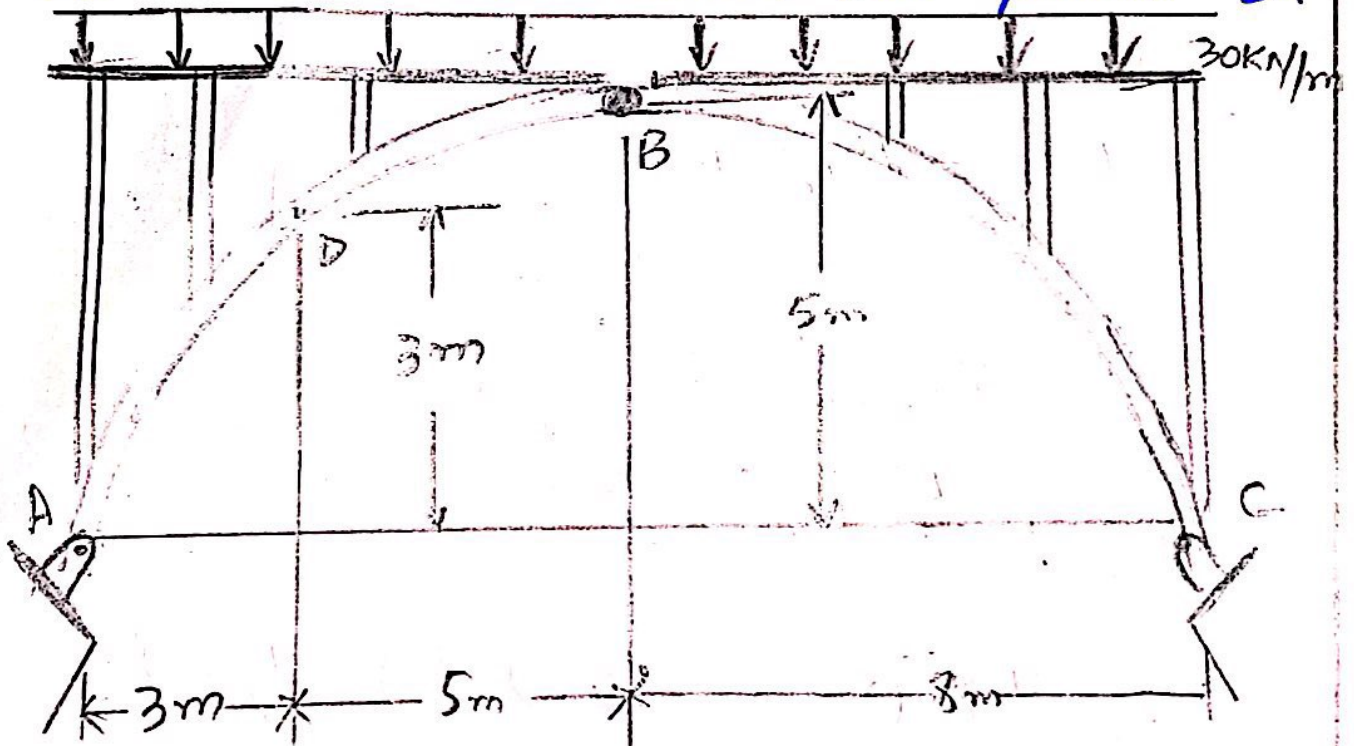
$$= 6000 (1.25)$$

$$= 7500 \text{ lb} \quad \therefore \text{Divide by } 1000$$

$$T_B = T_{max} = 7.5 \text{ k}$$

## QUESTION # 05

The three-hinged spandrel arch is subjected to the uniform load of  $30\text{ kN/m}$ . Determine the internal moment in the arch at point D.



Solution :-

Member AB.

$$\sum M_A = 0$$

$$\Rightarrow B_x(5) + B_y(8) - 240(4) = 0$$

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### Member BC :-

$$\sum M_C = 0$$

$$-B_x(5) + B_y(8) + 240(4) = 0$$

Now

$$B_x = 192 \text{ kN}, \quad \therefore B_y = 0$$

### Segment BD :-

$$\sum M_D = 0$$

$$= 192(2) - 150(2.5) - M_D = 0$$

$M_D = 9 \text{ kN}\cdot\text{m}$

