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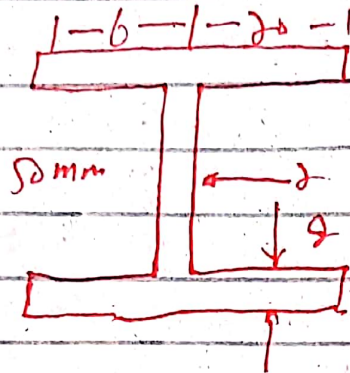
SUBMITTED TO :-

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DATED :-

23/06/2020

Q.No: 1 (a)



Required location of Shear Centre

Sol: As we know that

$$e = \frac{I_{fh}^2 b}{4I}$$

and

$$I = \left(2 \frac{bh^3}{12} + Ay^2 \right) + \left(\frac{by^3}{12} + Ay^2 \right)$$

$$I = 2 \left(\frac{2 \cdot 6 \cdot (2)^3}{12} + (2 \cdot 2) \cdot (25)^2 \right) + \left[\frac{2 \cdot (50)^3}{12} + 0 \right]$$

$$I = 50024 \cdot 66 + 20833$$

$$I = 70867 \cdot 99 \text{ mm}^4$$

$$e = \frac{2 \cdot (50)^2 \cdot (2)^2}{4 \cdot (70867 \cdot 99)} = 11.02 \text{ mm}$$

So Shear center $e = 11.02 \text{ mm}$

Q No 4 part (b)

Given data

$$\Rightarrow H = 26 \text{ ft}$$

\rightarrow assume diameter

$$D = 22 \text{ ft}$$

$$\rightarrow \text{tangential stress} = 600 \text{ lb/ft}^2$$

\rightarrow specific weight of water tank

$$\text{tank} = 62.4 \text{ lb/ft}^3$$

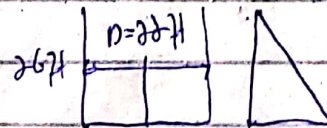
we have to find thickness :-

Solution:

the pressure develop by water

$$P = \gamma h$$

$$6t = \frac{PD}{2t}$$



$$6t = \frac{PD}{2t} = \frac{\gamma h D}{2t}$$

$$2t \times 6t = \gamma h D$$

$$\Delta t = \frac{\Delta h D}{6t}$$

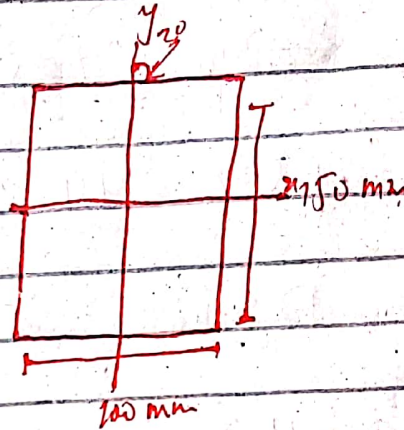
$$t = \frac{\Delta h D}{6t \times \Delta}$$

$$t = \frac{(62.4) \times (96 \times 12) \times (22 \times 12)}{(12^3)}$$

$$6000 \times 2$$

$$t = 0.24''$$

Q No. 2(a)



Moment of inertia

$$I_z = \frac{bh^3}{12} = \frac{0.1(0.25)^3}{12} = 2.8125 \times 10^{-5}$$

Now

$$I_y = \frac{bh^3}{12} = \frac{0.25(0.1)^3}{12}$$

$$I_y = 1.25 \times 10^{-5}$$

$$\sigma = \frac{M_z y}{I_z} + \frac{M_y z}{I_y}$$

$$\sigma = \frac{M \cos \theta}{I_z} + \frac{M \sin \theta}{I_y}$$

Where

$$\begin{aligned} M &= P \cos \theta = P \cos \theta = M_z \\ &= P \cos^2 \theta = M_z \end{aligned}$$

$$M_z = \cancel{1.8510} \cdot 1.2510$$

$$M \sin \theta = P \sin \theta = M_y$$

$$M_y = 12.510$$

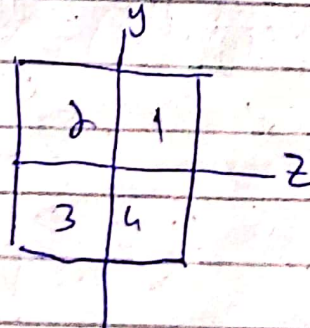
$$M_y = -11.8563$$

$$\sigma = \left(\frac{M_z}{I_z} \right) + \left(\frac{M_y}{I_y} \right)$$

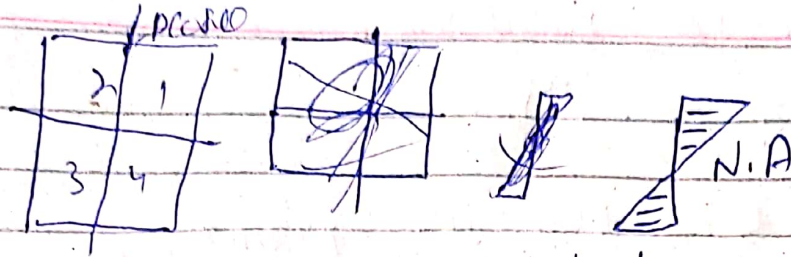
$$\sigma = \frac{1.851}{2.812 \times 10^5} + \left(\frac{-11.8563}{1.251 \times 10^5} \right) =$$

$$-82.678 \text{ N/m}^2$$

Sign Convention

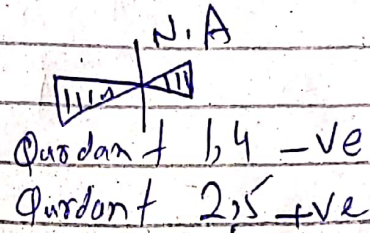
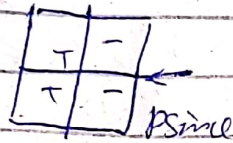


if we compression as negative and tension as positive and beam is simply supported



Quadrant 1, 2 -ve

Quadrant 3, 4 +ve



Case of lin. symmetrical loading
 the neutral axis line of an angle α
 the principal axis and the algebraic sum of
 stress at N.A is zero

$$\sigma = \frac{M \cos \alpha}{I_x} + \frac{M \sin \alpha}{I_y} \quad (1)$$

In the case N.A passes through
 $\alpha = 90^\circ$

$$\sigma = \frac{M \cos \alpha}{I_x} + \frac{M \sin \alpha}{I_y}$$

Let consider point 'A' on N.A lies quadrant 2,
 where Bending stress due to $P \cos \alpha$ is
 Compressive

Bending end stress due to $P \sin \alpha$ is Tensile

$$\text{eqn (1)} \Rightarrow \sigma = \frac{-M \cos \alpha y_A}{I_x} + \frac{M \sin \alpha z_A}{I_y}$$

$$\Rightarrow \frac{M \cos \gamma_A}{T_x} + \frac{M \sin \alpha}{T_y} \gamma_A$$

$$\Rightarrow \ominus 0 = \frac{M \cos \gamma_A}{T_x} + \frac{M \sin \alpha}{T_y} \gamma_A$$

$$\Rightarrow \frac{M \cos \gamma_A}{T_x} + \frac{M \sin \alpha}{T_y} \gamma_A$$

$$\frac{\gamma_A}{EA} = \frac{T_x}{T_y} \frac{\sin \alpha}{\cos \alpha} = \tan \alpha \frac{T_x}{T_y} \tan \alpha \Rightarrow (1)$$

NOW put value of T_x , T_y and α in eq (1)

$$\tan \alpha = \frac{2.8125 \times 10^{-5}}{1.25 \times 10^{-5}} (\tan 30^\circ)$$

$$\tan \alpha = 14.4129$$

$$\alpha = \tan^{-1} (14.4129)$$

$$\alpha = 1.5^\circ$$

$$\alpha = 1.305''$$

C.No: 2(b)

Given data

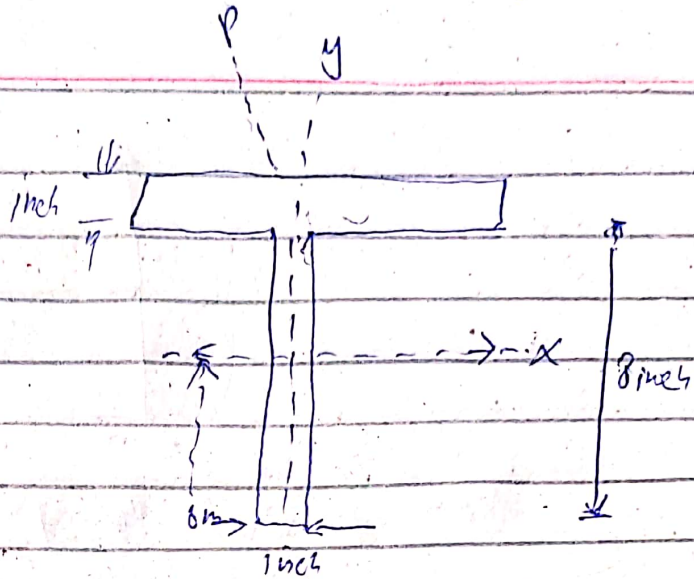
$$L = 16.77$$

$$I_x = 112.6 \text{ in}^4$$

$$I_y = 18.7 \text{ in}^4$$

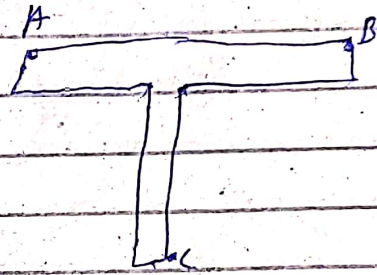
$$\sigma_c = 18000 \text{ psi}$$

$$\sigma_t = 5000 \text{ psi}$$



Sol

By looking to the fig we can judge that maximum compression would occur on A and maximum tension at C at B there will be tension as well as compression which will reduce the effect of each other so we will calculate stress at A and C



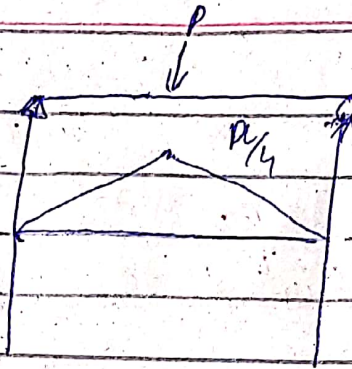
so

$$\sigma_A = \frac{Mx}{I_x} + \frac{My}{I_y} \quad (\text{Compression})$$

$$\sigma_C = \frac{Mx}{I_x} - \frac{My}{I_y} \quad (\text{Tension})$$

Now M_x and M_y

So



$$M_x = \frac{P \cos 60^\circ \times (16 \times 12)}{4}$$

$$M_x = 48 P \cos 60^\circ$$

$$M_y = \frac{P \sin 60^\circ (16 \times 12)}{4}$$

$$M_y = 48 P \sin 60^\circ$$

Now

$$\sigma_A = \frac{M_x y}{I_x} + \frac{M_y x}{I_y}$$

$$12000 = \frac{48 P \cos 60^\circ \times 3.07}{112.6} + \frac{48 P \sin 60^\circ \times 3}{18.7}$$

Solving the equation

$$P = 1638.6 \text{ lb}$$

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Now

$$TC = \frac{Mxy}{1x} + \frac{Myx}{1y}$$

$$5000 = \frac{48P(500)(5.93)}{112.6} + \frac{48(500)(0.5)}{18.7}$$

Solving the equation

$$P = 2104.9 \text{ Rs}$$

So the maximum load P applied should be 1638.6 kg

Q3

Given data

$$\text{length } l = 10\text{ft}$$

As both side are hinged
So

$$l_e = L$$

$$E = 10.3 \times 10^6$$

$$\text{Factor of Safety} = 2$$

$$b = 0.75 \text{ inch}$$

$$h = 2 \text{ inch}$$

Required

Determine Safe Load = ?

Sol:

As

$$P_{cr} = \frac{\pi^2 EI}{l_e^2}$$

As we know that $I = Ax^2$

$$I = Ax^2$$

$$x = \sqrt{I/A}$$

$$x = \frac{b \sqrt{hb^3}}{\sqrt{12}} = \sqrt{\frac{b^2}{12}}$$

$$x = \frac{b}{2\sqrt{3}} = \frac{0.75}{2\sqrt{3}}$$

$$\delta = 0.216 \text{ inch}$$

$$P_{cr} = \frac{\pi^2 EI}{(l_e/\delta)^2}$$

$$\Rightarrow \frac{(3.14)^2 (10.3 \times 10^6) (1.5)}{(10 / 0.216)^2}$$

$$P_{cr} = 853.8843$$

Safe load = Crippling load

Factor of Safety

$$\frac{853.8843}{2}$$

$$\text{Safe load } 426.917$$

for fixed ended column

$$l_e = l/2 = 10/2$$

$$l_e = 5H$$

$$PCV = \frac{HSA}{\left(\frac{1}{g}\right)^2}$$

$$= \frac{(3/4) \times (10.34 \times 10^6) \times (1.5)}{\left(\frac{60}{0.216}\right)^2}$$

$$PCV = 1974.207$$

$$\text{Safe Load} = \frac{PCV}{\text{factor of safety}}$$

$$= \frac{1974.207}{2}$$

Sol

$$= 987.103$$