

# Assignment No Last

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(1)

Ans " Stage discharge relationship  
for a concrete rectangular  
" Box culvert "

" Given data "

Width = ~~1.7m~~ 1.7m

Height = 0.7m

Length = 40m

Slope = 1:1000

Manning's  $n = 0.013$

Spare edged entrance ;  $K_e = 0.5$

Range = 0-3m

Sol:

$H/D \leq \text{~~1.7m~~ 1.7m}$

$H < 0.7m$

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Discharge is given by

$$Q = 2.492 \gamma_0 \left[ \frac{1.2 \gamma_0}{1.2 + 2 \gamma_0} \right]^{2/3} \text{ --- "A"}$$

$\gamma_0$ (m)	$Q$ ( $\text{m}^3/\text{s}$ )	$\gamma_c$ (m)
0.3	0.234	0.124
0.5	0.481	0.200
0.7	0.754	0.271

By putting values of " $\gamma_0$ " we will get the corresponding discharge

$$Q_1 = 2.492(0.3) \left[ \frac{1.2(0.3)}{1.2 + 2(0.3)} \right]^{2/3}$$
$$= 0.234 \text{ m}^3/\text{s}$$

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$$Q_2 = 2.29(0.5) \left[ \frac{1.2(0.5)}{1.2 + 2(0.5)} \right]^{2/3}$$
$$= 0.481 \text{ m}^3/\text{s}$$

$$Q_3 = 2.29(0.7) \left[ \frac{1.2(0.7)}{1.2 + 2(0.7)} \right]^{2/3}$$
$$= 0.754 \text{ m}^3/\text{s}$$

Critical depth? (4)

$$y_c = (q^2/g)^{1/3} \text{ --- "A"}$$

$$q = Q/B \text{ --- "B"}$$

By putting values in eq "B"

$$q_1 = Q_1/B \rightarrow = \frac{0.234}{1.7} = 0.137$$

$$q_2 = Q_2/B \rightarrow = \frac{0.481}{1.7} = 0.282$$

$$q_3 = Q_3/B \rightarrow = \frac{0.754}{1.7} = 0.443$$

Now putting values in eq 'A'

$$y_{c1} = (q_1^2/g)^{1/3} = \left( \frac{(0.137)^2}{9.81} \right)^{1/3} = 0.124 \text{ m}$$

$$\gamma C_2 = \left( \frac{q_2^2}{g} \right)^{1/3} = \left( \frac{(0.282)^2}{9.81} \right)^{1/3} = 0.200 \text{ m}$$

$$\gamma C_3 = \left( \frac{q_3^2}{g} \right)^{1/3} = \left( \frac{(0.443)^2}{9.81} \right)^{1/3} = 0.271 \text{ m}$$

At the inlet over short reach

$$H = \gamma_0 + \frac{V^2}{2g} + K_e \cdot \frac{V^2}{2g}$$

$$\text{So } V = 1.142 \text{ m/s}$$

$$H_1 = \gamma_{01} + \frac{V^2}{2g} + K_e \cdot \frac{V^2}{2g}$$

$$= 0.3 + \frac{(1.142)^2}{2 \times 9.81} + 0.5 \left( \frac{(1.142)^2}{2 \times 9.81} \right)$$

$$= 0.399 \text{ m}$$

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$$H_2 = 0.5 + \frac{(1.142)^2}{2 \times 9.81} + 0.5 \left( \frac{(1.142)^2}{2 \times 9.81} \right)$$
$$= 0.599 \text{ m}$$

$$H_3 = 0.7 + \frac{(1.142)^2}{2 \times 9.81} + 0.5 \left( \frac{(1.142)^2}{2 \times 9.81} \right)$$
$$= 0.799 \text{ m}$$

$y_0$ (m)	H (m)	Q ( $\text{m}^3 \text{s}^{-1}$ )
0.3	0.399	0.234
0.5	0.599 - $x_1$	0.481 - $x_1$
0.7	0.799 - $x_2$	0.754 - $x_2$
orifice 707 1.2 D	0.84	0.809

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"2" H/D  $\approx$  7, 1.4

"a" for orifice flows

$$Q = Cd (1.7 \times 0.7) \left[ 2g \left( H - \frac{D}{2} \right) \right]^{1/2}$$

$$Q = 0.6 (1.7 \times 0.7) \left[ 2 \times 9.81 \left( 0.84 - \frac{0.7}{2} \right) \right]^{1/2}$$

$$Q = 6.864 \text{ m}^3/\text{s}$$

The following results are obtained

H(m)	Q (m <sup>3</sup> s <sup>-1</sup> )	$\gamma_0$ (m)
0.84	6.864	70.7
		no orifice flow exists



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'b' For pipe flow the energy equation gives

$$H + S_0L = D + h_L$$

where

$$h_L = K_e \frac{V^2}{2g} + (Vn^2) \frac{L}{R^{4/3}} + \frac{V^2}{2g}$$

$$Q = 2.08 (H - 0.57)^{1/2}$$

During rising stages the barrel flows full from  $H = 1.08$  and during falling stages the flow becomes free surface flow when  $H = 0.777$  m

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The following table summarizes the results

H(m)	Q (m <sup>3</sup> /s)	Type of flow
Rising Stages		
0.399	0.234	open channel
0.599	0.481	open channel
0.799	0.754	<del>open channel</del> Pipe flow
0.844	0.809	Pipe flow
1.00	1.364	Pipe flow
2.00	2.487	Pipe flow
3.00	3.242	Pipe flow
Falling Stages		
2.00	2.487	Pipe flow
1.00	1.364	Pipe flow
0.844	0.809	Pipe flow
0.799	0.754	Pipe flow
0.599	0.481	open channel
0.399	0.234	open channel

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Scour:

Scour is an erosional process that can occur in rivers due to the interaction between any of structure located under water and the river flow

OR

Scour is an erosional process that can occur in rivers due to natural or man made events.

Natural erosional processes take place in rivers because they act as conduits for movement of water sediment. Manmade scour can be caused for instance by legal or illegal sediment extraction dam operation and the

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influence in general of any structure placed in to the river stream.

Different types of Scour:

Natural Scour:

Natural Scour occurs due to the natural variability of river stream flow and sediment regime, considering the influence from the catchment to the river ~~side~~ scale. Gradation of the river bed, lateral channel migration, bend and confluence Scour are part of natural Scour

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Local Scour:

Local Scour emerges due to a local concentration of turbulence generated by structures that obstruct and split the flow (e.g. bridge piers and abutments). Local Scour occurs around these structures because of the limited influence range they have on the river flow.

~~Constriction~~ Constriction Scour:

Constriction Scour occurs due to flow contraction when flow velocity and thus shear stress increase, for instance between

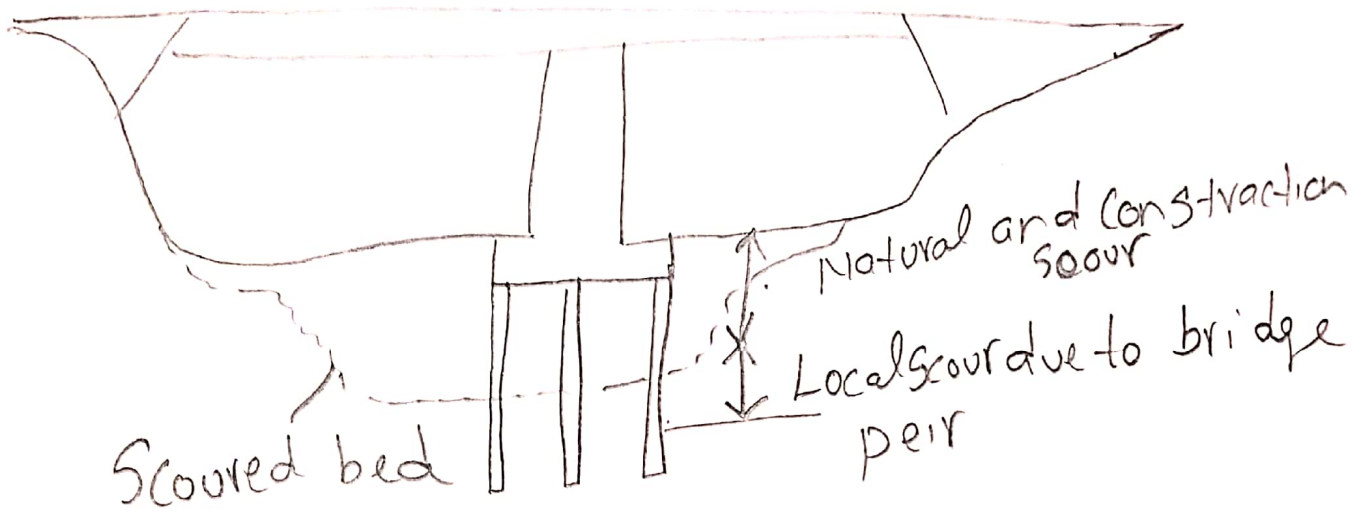
(13)

bridge abutments. Construction scour occurs around these. normally take place within the complete river stream width

Total Scour:

Total Scour is defined as the sum of effects of all the scour processes that take place at a given location

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# Mechanics of Scour on bridge

## Phase

Initial ~~Scour~~:

The SCOUR process starts showing erosional patterns on the lateral side of the cylinder pier

Progressing phase:

The erosional pattern progress from the lateral side to the front of pier. From the moment the two scour patterns coincide at the front of the pier, the deepest scour depth is achieved.



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Developing phase.

The Scour process develops and the ~~scour~~ scour rate slows down

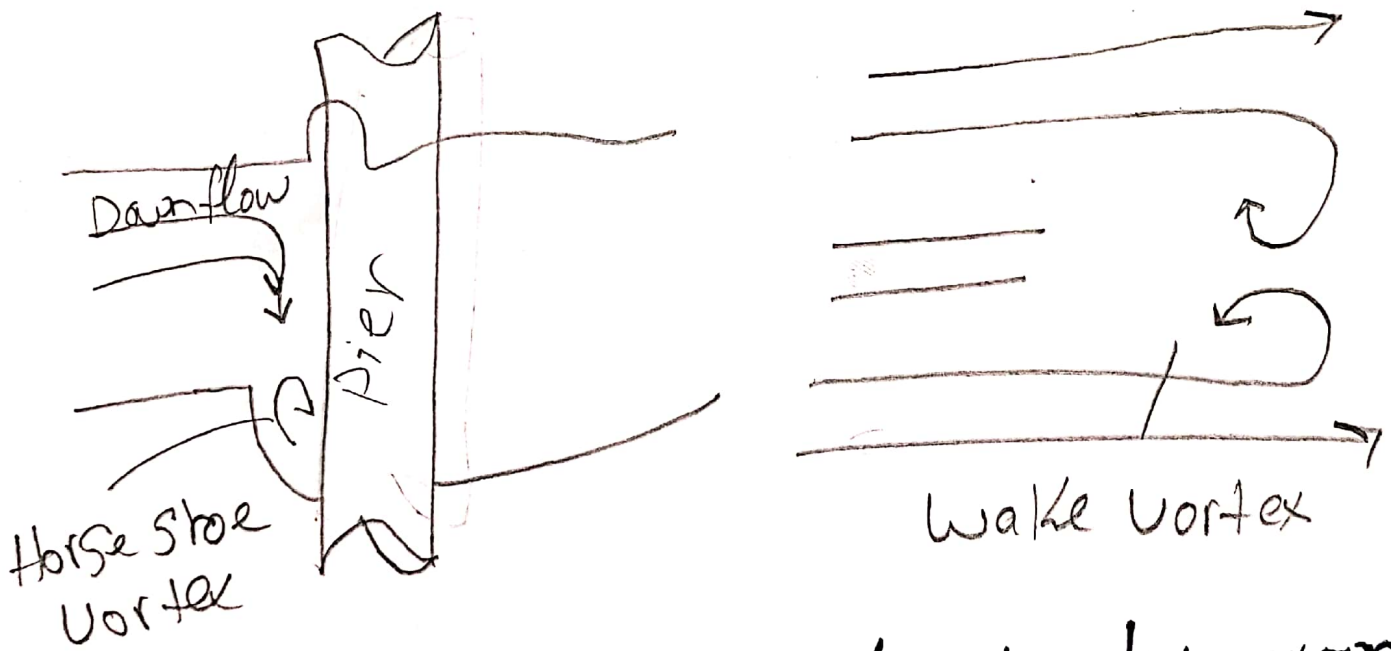
Equilibrium phase

Erosion inside the Scour hole is negligible

At the obstruction in form of pier or abutment the unidirectional flow changes into ~~the~~ three dimensional of the water pileup in front face of the obstruction and the flow accelerates around the nose. This phenomenon results

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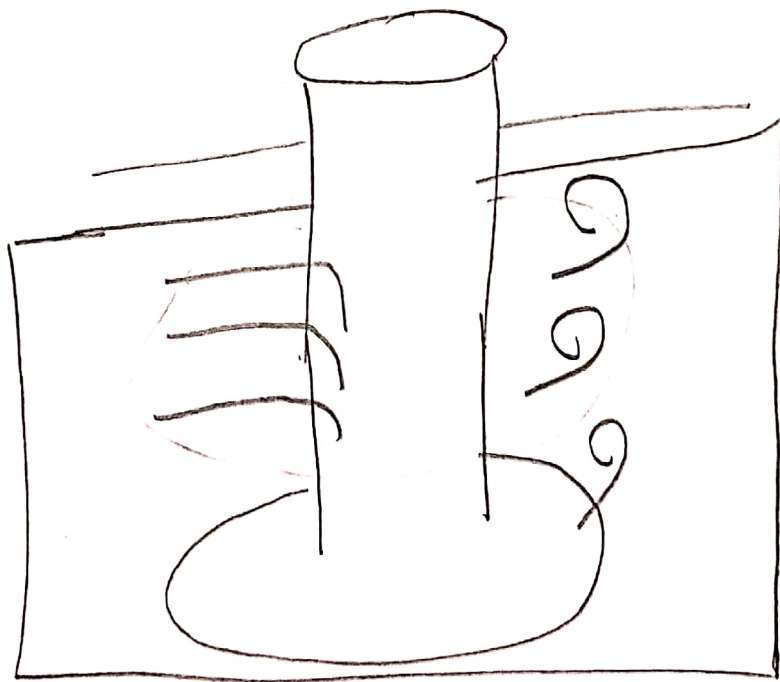
information of Vortex at the base of the pier known as horse shoe vortex and the vortex from in the vertical direction downstream of the pier known as wake vortex as shown in figure



presentation of Vortex around a circular path

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The pileup of water due to obstruction ~~is~~ because of decelerations of flow due to stagnation pressure of water causes a downward flow results in horse vortex. The vertical component of the downward flow causes erosion around the base of the pier.



Due to rolling of unstable layer at the surface of the pier wake vortex are generated at the separation and moves forward with flow downstream of the pier: It can be shown

In the practical case the river bed is generally composed of mixture of different size of material. Due to washing out of finer material and armor layer is formed of coarser materials which protect the underlying finer particles from further scour.

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▽ Pier

Downflow

▽

Horse Shoe Vortex

Bed Sand

Armor Layer