

# Structure Analysis

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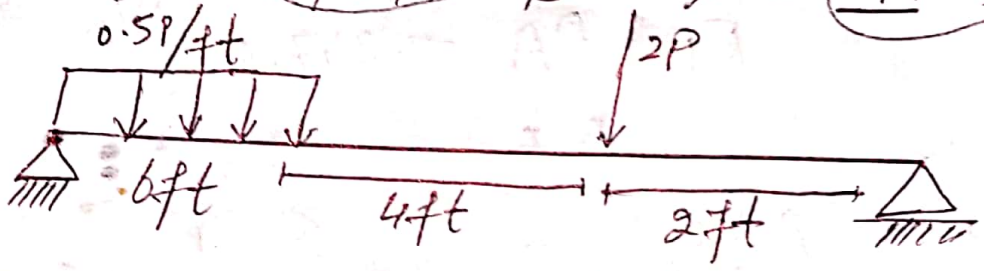
ID :- 7899

Sec :- A

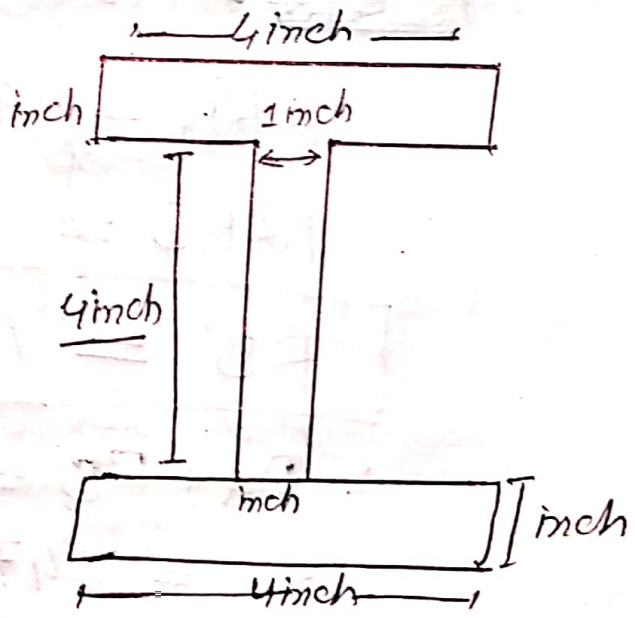
# Q.No.1.

Ans.:

Hints: P, where P that will be last two digits of my registration no that 7899 So last 99.

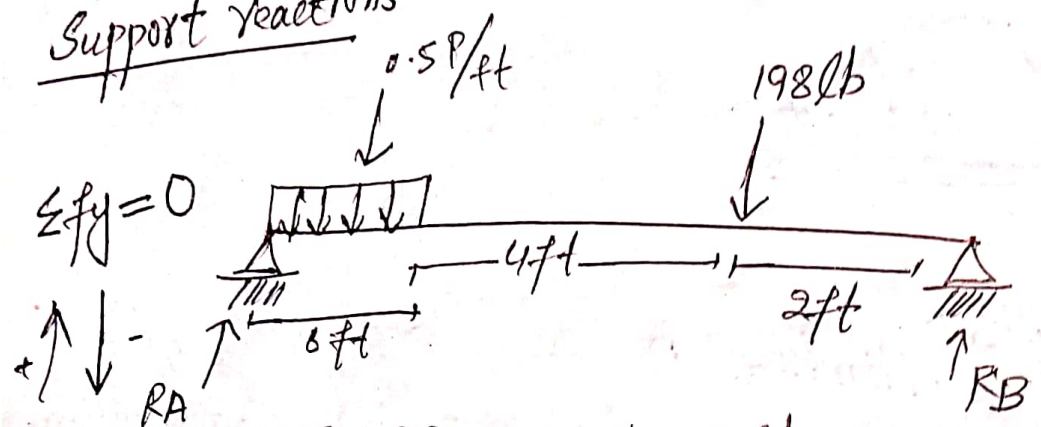


So  $2P = 2 \times 99 \Rightarrow 198 \text{ lb}$



Force body diagram

Support reactions



$\sum F_y = 0$

$0.5 \times 6 = 49.5 \text{ lb}$

$R_B + R_A = 247.5 \text{ lb}$  (total load  $\Rightarrow 198 + 49.5$ )

Now

$\sum M = 0$  (clockwise +, counter-clockwise -)

~~$R_B \times 12$~~   $- 198 \times 10 - 49.5 \times 3 = 0$

$12 R_B - 1980 - 148.5 = 0$

$12 R_B = 2128.5$

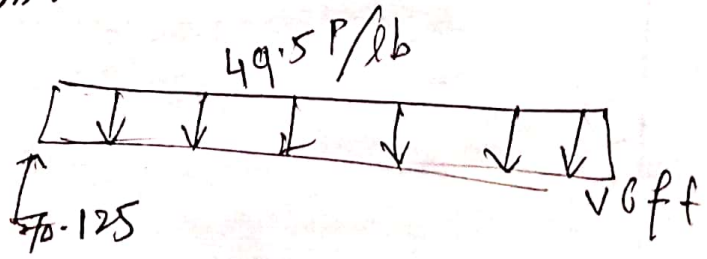
$R_B = 177.375$

$R_A + R_B = 247.5$

$R_A = 247.5 - R_B \Rightarrow 247.5 - 177.375$

$R_A = 70.125$

Now shear force at change point of beam.



So Shear force at 6ft from left support

$$\sum F_y = 0 \quad \uparrow^- \quad \downarrow^+$$

$$+ V_{6ft} - 70.125 + 49.5 \times 6 = 0$$

$$+ V_{6ft} + 226.875 = 0$$

$$V_{6ft} = -226.875$$

$$V_{6ft} = -226.875 \text{ lb}$$

Now shear force at 10ft

$$\sum F_y = \uparrow^- \quad \downarrow^+$$

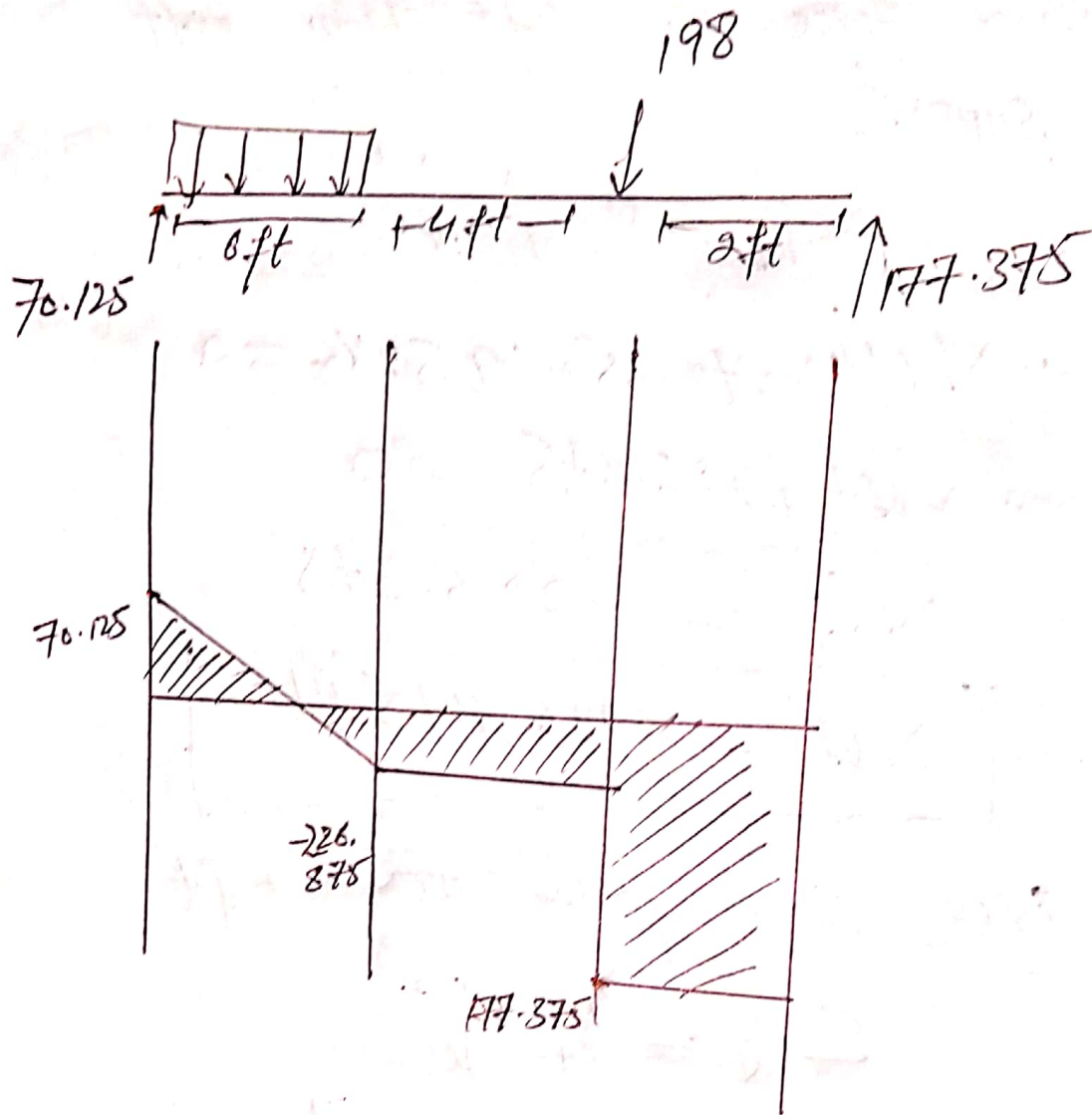
$$- 70.125 + 49.5 \times 10 + V_{10ft} = 0$$

$$V_{10ft} = -177.375$$

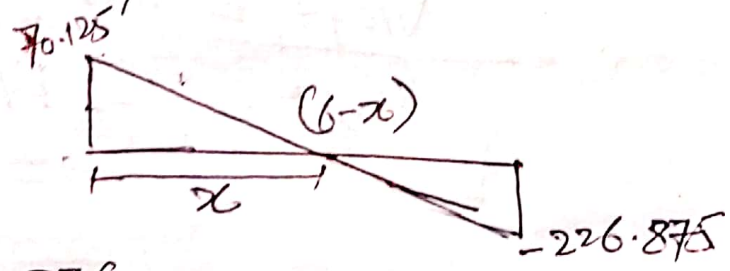
$$V_{10ft} = -177.375$$



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Now moment at change point  
Find zero shear point.



$$\frac{70.125}{x} = \frac{-226.875}{(6-x)}$$

$$\begin{aligned} (70.125)(6-x) &= -226.875x \\ 420.75 - 70.125x &= -226.875x \\ 420.75 &= -226.875x + 70.125x \\ &= x(-226.875 + 70.125) \end{aligned}$$

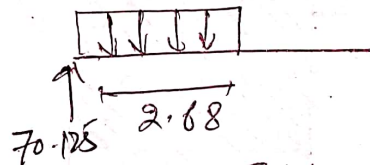
$$420.75 = 156.75x$$

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$$x = 2.68$$

As we know that moment is maximum where shear force is zero.

Take section at 2.68 from left support and find moment.



$$\sum M_{2.68} = 0$$

$$M_{2.68} - 70.125 \times 2.68 + \frac{49.5}{2} \left( \frac{2.68}{2} \right) = 0$$

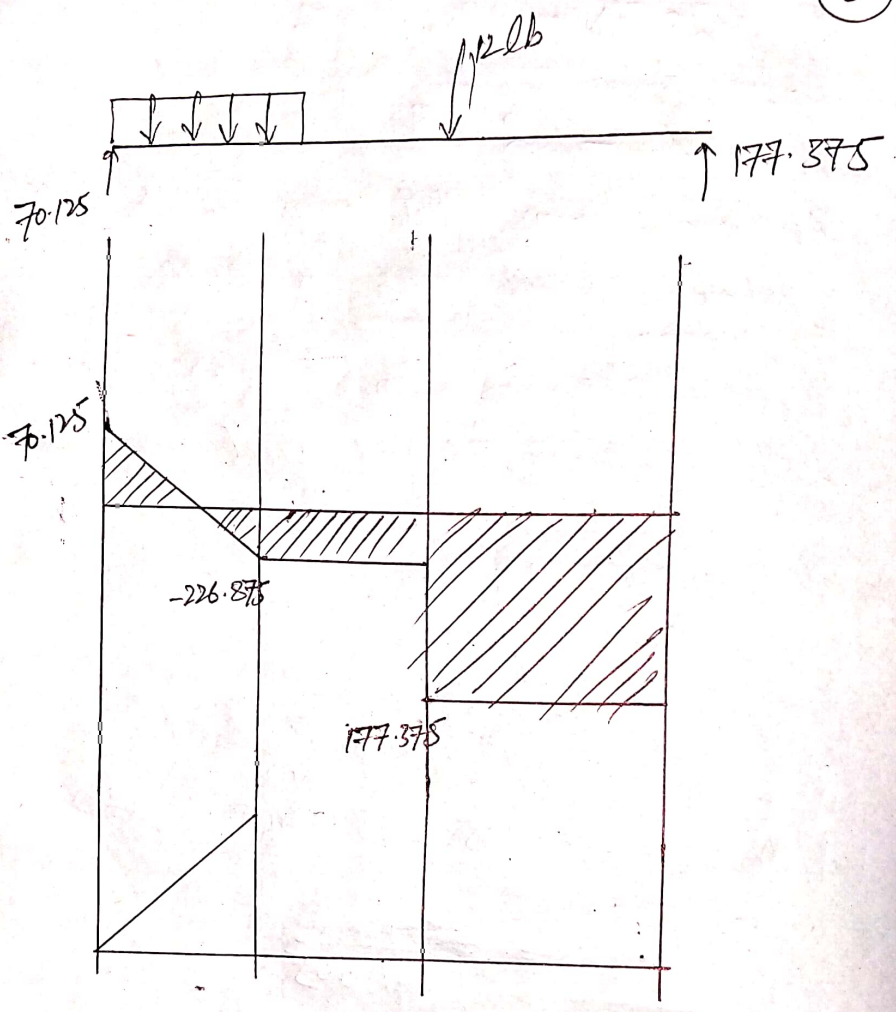
$$M_{2.68} = \frac{121.605}{1}$$

Now  $M_{6ft} \Rightarrow 70.125 \times 6 + 49.5 \times 6 \times 3 = 0$

$$M_{6ft} \Rightarrow 24.75$$



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Now

Shear stress

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As per Question  
The maximum shear stress

$$J = \frac{VQ}{It}$$

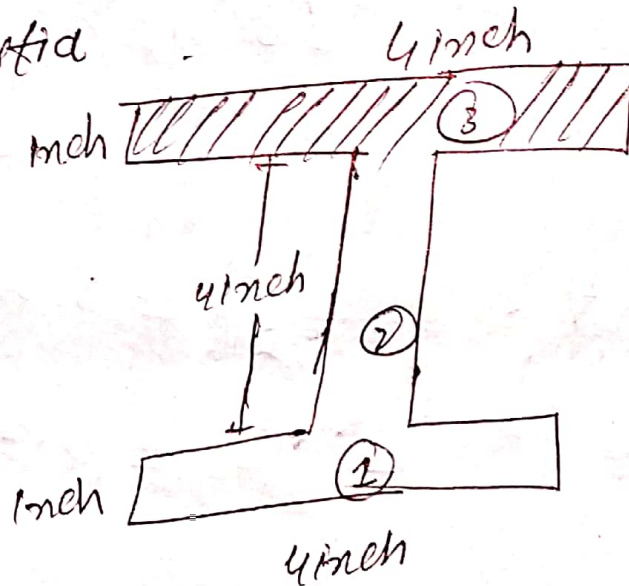
occure where the

maximum shear force lies in above  
diagram max shear force is \_\_\_\_\_

So to find the shear stress  
we have the following formula

$$\tau = \frac{VQ}{It}$$

we first find the moment  
of inertia



As we know that to find  
Centroid we have the following  
formula



$$\bar{y} = \frac{A_1 y_1 + A_2 y_2 + A_3 y_3}{A_1 + A_2 + A_3}$$

$$A_1 = 4 \times 1 = 4$$

$$A_2 = 4 \times 1 = 4$$

$$A_3 = 4 \times 1 = 4$$

$$\bar{y} = \frac{4 \times 1}{4+4+4}$$

$$\bar{y} =$$

Now moment of inertia

No	A (cm <sup>2</sup> )	I <sub>x</sub> (cm)	d = (ȳ - y <sub>1</sub> ) (ȳ - y <sub>2</sub> )(ȳ - y <sub>3</sub> )
①	4	$\frac{4 \times 1^3}{2} = 0.333$	
②	4	$\frac{1 \times 4^3}{12} = 5.333$	
③	4	$\frac{4 \times 1}{12} = 0.333$	

(Now "d")

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$$\textcircled{1} \quad d = (\bar{y} - y_1) = (3 - 0.5) = 2.5$$

$$\textcircled{2} \quad d = (\bar{y} - y_1) = (3 - 3) = 0$$

$$\textcircled{3} \quad d = (3 - 5.5) = -2.5$$

(Now  $d^2$ )

$$\textcircled{1} \quad 4 \times (2.5)^2 = 25$$

$$\textcircled{2} \quad 4 \times (0)^2 = 0$$

$$\textcircled{3} \quad 4 \times (-2.5)^2 = 25$$

Now  $I_x = I_x + Ad^2$

$$\textcircled{1} \quad 0.333 + 25 = 25.333$$

$$\textcircled{2} \quad 5.333 + 0 = 5.333$$

$$\textcircled{3} \quad 0.333 + 25 = 25.333$$

$$I = 55.999 \text{ in}^2$$

Now shear stress

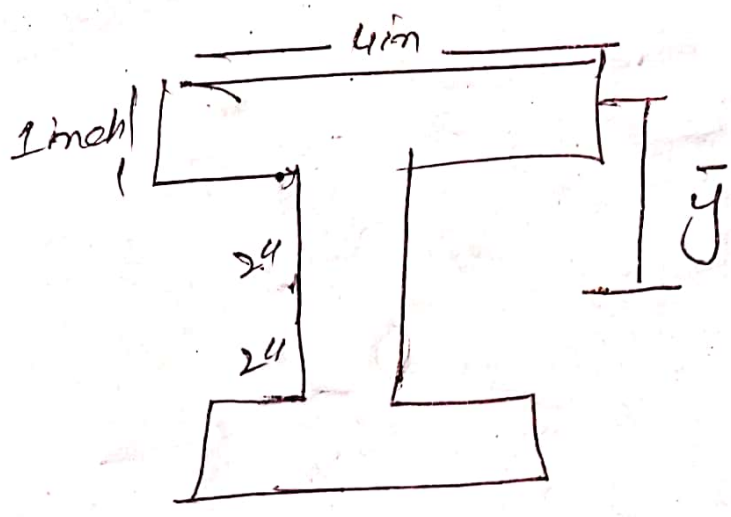
$$\tau = \frac{VQ}{Ib}$$

$$V_{\text{max}} = 177.375$$

$$Q = \tau A$$

$b$  = breadth of that fiber.

Shear stress at point C located  
Centre of uniformly distributed load  
and 1 inch below the top fiber.



$$\bar{y} = 2 + 0.5 = 2.5$$

$$A = 1 \times 4 = 4$$

$$Q = 4 \times 2.5 = 10$$

As we know that

$$\tau = \frac{VQ}{Ib}$$

$$\tau = \frac{(177.375 \times 10)}{(55.996)(4)}$$

$$\tau \Rightarrow 7.919 \text{ psi}$$



Now flexural stress Analysis (1)

$$\sigma = \frac{Mx}{I}$$

where  $M$  is maximum moment in BMA.

$$M = \frac{1.5}{2}$$

$$\sigma = \frac{(1.5)(2)}{55.996}$$

$$\sigma = 0.0535$$

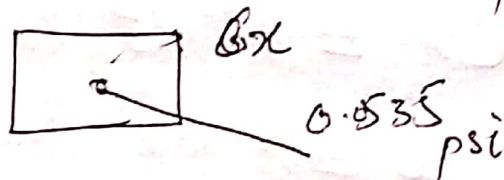
So shear stress at point "C" is

$$\tau = 7.919 \text{ psi}$$

Flexural stress at point C

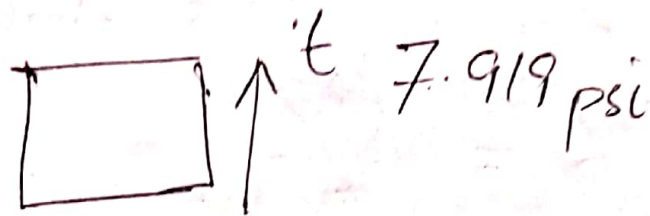
$$\sigma = 0.0535 \text{ psi}$$

Now consider "C" is a plane element



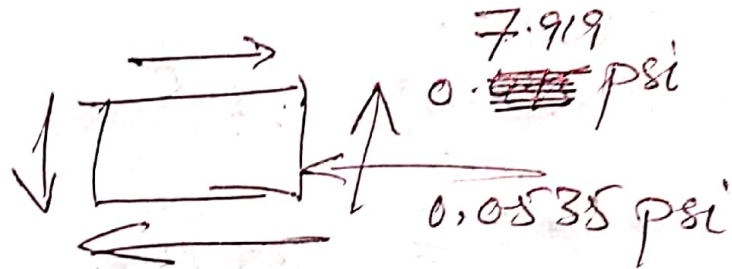
0.0535 is compressive because point C lies in compression zone of beam cross

Now



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Combine stress on 2D element



Now stress com find the stress state consider of point "C" at a degree of  $20^\circ$  clockwise orientation.

Solve Given stress state

$$\sigma_x = -0.0535$$

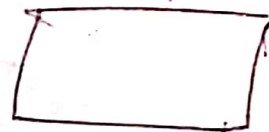
$$\sigma_y = 0$$

$$\tau_{xy} = 0.475$$

$$\sigma_{x'} = ?$$

$$\sigma_{y'} = ?$$

$$\Sigma \tau_{ij} = ?$$



As we derive the following formula  
 eq. equation for stress transformation.

As we derived the following formula equation for stress transformation

$$\sigma_{x'} = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta + \tau_{xy} \sin 2\theta$$

$$\sigma_{y'} = \frac{\sigma_x + \sigma_y}{2} - \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau_{xy} \sin 2\theta$$

$$\tau_{x'y'} = -\frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$$

So first

$$\sigma_{x'} = \frac{-0.0535 + 0}{2} + \frac{-0.0535 - 0}{2} \cos 2(-20) + (0.475) \sin 2(-20)$$

$$= -0.047 \text{ (compression)}$$



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$\sigma_y'$  Now

$$\sigma_y = \frac{-0.0535 + 0}{2} + \frac{(-0.0535) - 0}{2} (0.5(2(-20)))$$

$$- (0.475) \sin 2(-20)$$

$$= -0.00635 \text{ Compression}$$

Now  $\tau_{x'y'}$

$$\tau_{x'y'} = \frac{-0.0535}{2} - 0 \sin^2(-20) + \frac{0.475}{2} \cos 2(-20)$$

$$= 0.01706 \text{ PSI}$$

and the Principle Stress

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$$\sigma_{1,2} = \frac{-0.0535 + 0}{2} \pm \sqrt{\left(\frac{-0.0535 - 0}{2}\right)^2 + (0.11)^2}$$

$$= -0.0265 \pm \sqrt{0.00071 + 0.22}$$

$$= -0.0265 \pm 0.469$$

So  $\pm$  become

$$\sigma_y = \sigma_1 = -0.0265 + 0.469 = 0.4425$$

$$\sigma_x = \sigma_2 = -0.0265 - 0.469 = -0.4955$$

Now

max in plane shear stress

$$\tau_{xy} = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

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$$I_{xy} = \sqrt{\left(-\frac{0.0535}{2} - 0\right)^2 + (0.475)^2}$$

$$= \sqrt{0.00071 + 0.22}$$

$$I_{xy} = 0.469$$

To Draw Mohr's Circle for the  
given problem

First we find centre coordinates

$$(h, k) = \left(-\frac{0.0535}{2}, 0\right)$$

$$= (-0.026, 0)$$

Now Radius

$$r = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + I_{xy}}$$



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$$r = \sqrt{\left(\frac{-0.0535 + 0}{2}\right)^2 + (0.473)^2}$$

$$\tau_{xy} = 0.464$$

