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PAPER :-

HIGHWAY & Traffic ENGINEER

SUBMITTED TO :-

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QNO:1 Difference b/w flexible and rigid pavement

Ans: flexible pavement

① Grain to grain load transfer

② joints are not required

③ initial cost is low

④ less Durability

⑤ ~~good subgrade is requir~~

⑤ good subgrade is required

⑥ Temperature variation no effects the stress variation

⑦ life span is short ~ 15 year

⑧ Repair work is easy

Rigid pavement

① slab action take place

② joint are required

③ initial cost is high

④ Durability is high

⑤ good subgrade is not required

⑥ temperature variation effect the stress variation

⑦ Long life span ~ 30 years

⑧ Repair work is tough

QNO:2 What are the advantages of water bound over wet mix macadam?

Ans: ① water bound macadam is superior in quality b/c the material are carefully graded and the resulting mass is almost void less compacted mass.

- ② the interlocking of aggregate particles impacts adequate strength of materials selected for filling the voids. These insure non-entring of the plastic materials of the subgrade into the voids.
- ③ water bound macadam is less costly as compared to bituminous base course.

Q No: 3 - what are the difference b/w asphalt and bitumen

Ans:

<u>Asphalt</u>	<u>bitumen</u>
① Asphalt pavement are durable with a layer depth 25-40 mm	① bitumen pavement are less durable with layer of depth 10-20 mm
② life span 20 year	② life span 5-10 years
③ Asphalt surface is made smoother and more skid resistant ensuring the driver safety and minimal noise	③ the loose fragments on bitumen pavement make the driving experience noiser and can wear down consequently causing safety
④ installation is comparatively costlier	④ Cheap to install compared to asphalt

Q.No. 2 A crest vertical curve joining a +3 percent and a -4 percent grade is to be designed for 75 m/h. If the tangent intersect at station (345+60) at an elevation of 250 ft, determine the station and elevation of ~~PVI~~ BVC and EVC. Also calculate the elevations of intermediate point on the curve of the whole station ~~Ans~~

Solution For design speed of 75 m/h, $L = 512$ (from table)
 minimum length = $312 (3 - (-4)) = 2184$ ft
 Station of BVC = $(345+60) - \left(\frac{21+84}{2}\right) = 334+68$
 Station of EVC = $(334+68) + (21+84) = 356+52$
 Elevation of BVC = $250 - \left(0.05 \times \frac{21+84}{2}\right) = 217.74$ ft
 The remain of the ~~computation~~ computation is effectively done designing the tangent shown in table

Station	distance BVC (ft)	tangent Elevation (ft)	offset $r = \left(\frac{Ax^2}{2L}\right)$ ft	Curve elevation tangent elevation offset ft
BVC 334+68	0	217.74	0.01 0.01	217.74
BVC 335+00	32	217.74	0.02	217.18
BVC 336+00	132	221.20	0.28	220.92
BVC 337+00	232	225.21	0.86	223.34

Station	Distance BVC (ft)	Tangent Elevation (ft)	Offset $V = \frac{11x^2}{200L}$	Curve elevation tangent elevation
BVC 338+00	332	227.20	2.177	225.43
BVC 339+00	432	230.20	2.99	227.21
BVC 340+00	532	233.20	4.54	228.66
BVC 341+00	632	236.20	6.40	229.80
BVC 342+00	731	239.20	8.59	230.61
BVC 343+00	832	242.20	11.09	231.11
BVC 344+00	932	245.20	13.92	231.25
BVC 345+00	1032	248.20	17.07	231.13
BVC 346+00	1132	251.20	20.54	230.66
BVC 347+00	1232	254.20	24.30	229.88
BVC 348+00	1332	257.20	28.43	228.77
BVC 349+00	1432	260.20	32.86	227.34
BVC 380+00	1582	263.20	37.61	225.59
BVC 357+00	1682	266.20	42.68	223.05
BVC 352+00	1732	269.20	48.07	221.13
BVC 353+00	1832	272.20	53.79	218.41
BVC 354+00	1932	275.20	59.82	215.38
BVC 355+00	2032	278.20	66.17	212.05
BVC 356+00	2132	281.20	72.84	208.34
BVC 356+00	2184	282.76	76.41	206.32

Qn 10:3
Step No: 1

Ans: Draw a line joining the reliability level of 99% the overall standard deviation S_D of 0.49 and extend line to intersect the 1st TL line of point A

Step No: 2

Draw a line joining point A to the ESAL of 2×10^5 and extend this line to intersect the 1st TL line at point B

Step 3:

Draw a line joining point B and resilient modulus (M_R) of base course and extend this line to intersect the design serviceability loss chart at point C

Step 4:

Draw a horizontal line from point C to intersect the design serviceability loss (PSI) curve at point D. So here
~~PSI~~ $\Delta PSI = 4.5 - 2.5 = 2$

Step 5:

The structure number require to protect the base curve and to find this D_1 of the surface course is 2.6

Step 1:

Determine the appropriate structure layer Co-efficient for each construction material resultant value of

$$\text{Asphalt} = 150,000 \text{ lb/in}^2$$

therefore

$$a_1 = 0.44$$

$$D_1 = S_{M1} / a_1$$

$$= 2.6 / 0.44$$

$$= \text{~~5.9~~ } \cdot 5.9^2$$

Thickness should be taken to the nearest 0.5 inches so the thickness of the surface course is 6"

$$S_{M1} = D_1 \cdot a_1$$

$$6 \cdot 0.44$$

$$2.64$$

Now find S_{M2} and D_2 (base course)

find the value of a_2 from layer coefficient table and M_2 from drainage coefficient table

19 (7)

Thickness of base Course (D_2)

~~$D_2 = (SN_1 - SN_2) / a_2 m_2$~~

$D_2 = (SN_1 - SN_2) / a_2 m_2$

~~$D_2 = (3.8 - 2.64) / 0.14 \times 0.80$~~

~~$D_2 = 10.36$~~

$D_2 = (SN_1 - SN_2) / a_2 m_2$

$D_2 = 8$

$D_1 = (3.8 - 2.64) / 0.14 \times 0.80$

$D_2 = 10.36$

use 12"

So the thickness of base course is 12"

$SN_1 = 0.14 \times 0.80 \times 12 \times SN$

$SN_2 = 1.33 + 2.64$

$SN_2 = 3.98$

Finding SN_3 and D_3 Subbase course and also layer coefficient a_3 and drainage coefficient m_1 from their respective table

$D_3 = (SN_3 - SN_2) / a_3 m_1$

$D_3 = (4.4 - 3.98) / 0.10 \times 0.80$

$$D_3 = 5.24''$$

we will use 6" as a subbase

$$SN_3 = 2.64 + 1.3 \times 6'' \times 0.10 \times 0.80$$

$$SN_3 = 4.46 > 3.4, \text{ okay}$$

final design

Surface Course = 6"

Base Course = 12"

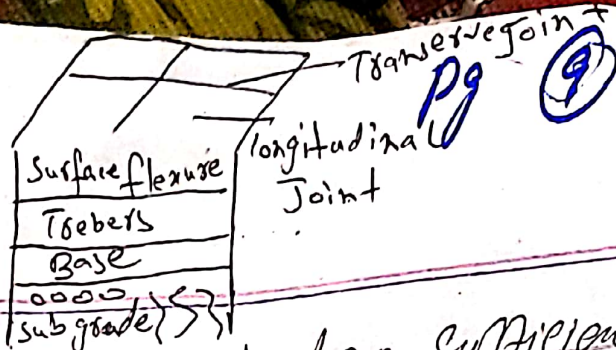
Sub base = 6"

Total pavement thickness = 24"

QNo: 4 different pavement distresses?

Ans: introduction: A highway pavement structure consisting super imposed layers of processed ~~vari~~ material the pavement structure should be able to provide a surface of acceptable riding quality Adequate skid resistance favorable light reflecting characteristic and low noise pollution

Rigid pavement



Rigid pavement have sufficient flexural strength to transmit the wheel load stresses to a wider area below. In rigid pavement load is distribute by slab action

Factor affecting pavement performance

- Traffic compact process wheel loading axle configuration moving load
- Structural model elastic model
- Material Characterization

* Environment factor temp moisture

Common rigid pavement distress

- 1) Spalling
- 2) faulting
- 3) Cracking
- 4) longitudinal crack
- 5) Slab cracking
- 6) Durability cracking
- 7) Polished aggregates
- 8) Dumping and water bleeding
- 9) Shrinkage cracking

Details

1) Spalling At the joint :: Cracking breaking or chipping of joints/corner edges usually occurs with in about 0.6m (2ft) of joint/cracking. It causes of infiltration of incompressible material and subsequent expression (can also cause blowups)

2) Faulting :: A difference in elevation across a joint or crack usually associated with undoloid JPCD. usually the approach slab is higher than the leave slab due to pumping

3) Longitudinal cracking

Longitudinal crack not associated with corner breaks or blowup that extend across the entire slab into two or four pieces.

↳ CORNER CRACKING ::

A slab that intersect the slab joint near the corner with in about 2m/ft) or so. It extend through the entire slab and caused by high corner stress

4) RUTTING :: Surface depression in a wheel path are particularly evident after a rain when they are filled with water

Cause:

- ① insufficient compaction
- ② Subgrade rutting
- ③ improper mix design

Repair

- ① Slight ruts (1/2 inch deep)
- ② loss of skid resistance when wet

Causes

excessive asphalt binder in the HMA
 excessive application of asphalt binder
 during BST application

iv polished aggregate ::
 Cause ::

Repeated traffic application
 this can be occur quicker in
 the aggregate is susceptible to
 abrasion.