

IQRA NATIONAL UNIVERSITY

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SECTION : A

MODULE : 8TH SEMESTER

#01/01

(No #01
part #A)

PA
R
1

(A) Define Drag with its Components
equation for friction Drag.
Coefficient both in laminar &
Turbulent boundary layers?

→ DRAUGHT A body which is wholly immersed in a homogenous fluid may be subjected to two kinds of force arising from relative motion B/w body & fluid that those forces are termed as drag & lift. If the forces are parallel to the motion

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Then it is termed as drag force.

There are Two Components

(i) PRESSURE DRAG (F_p)

It is equal to integration of Components in direction of motion of all pressure forces exerted on surface of body

$$F_p = C_p \left[\frac{\rho}{2} V^2 \right] A$$

where C_p depends on shape

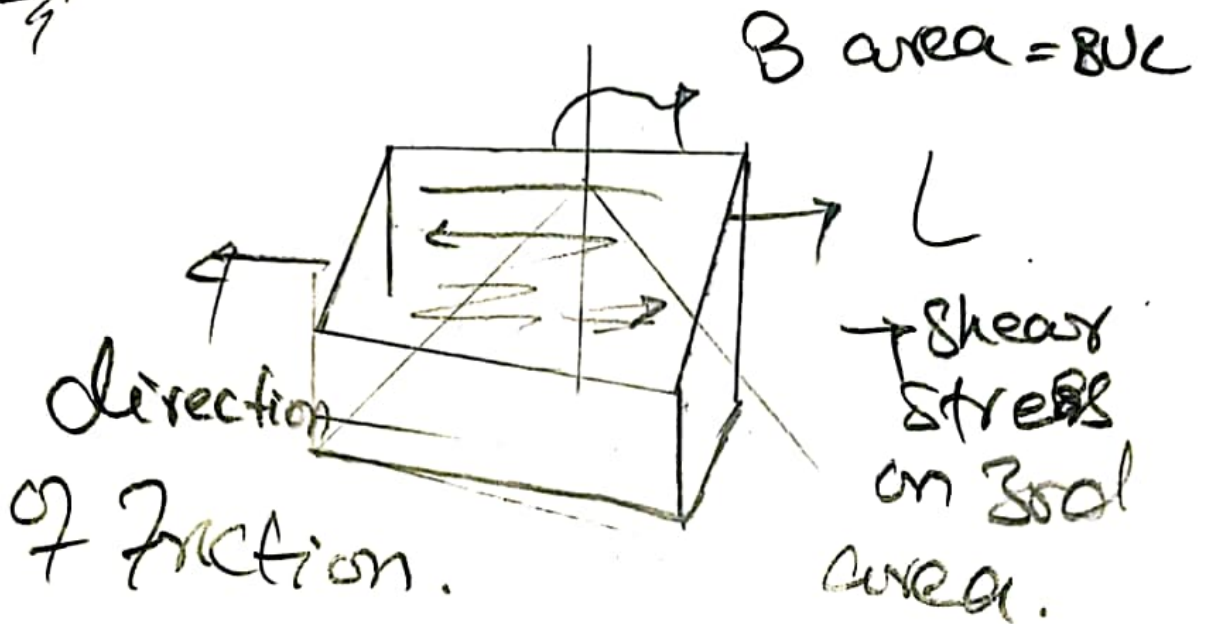
(ii) FRICITION DRAG (F_f)

It is equal to integration of Components of shear stress along surface of body in direction of motion.

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$$F_f = C_f \int \frac{V^2}{2} BC$$

Fig: 7



→ FRICTIONAL DRAG OF B. LAYER

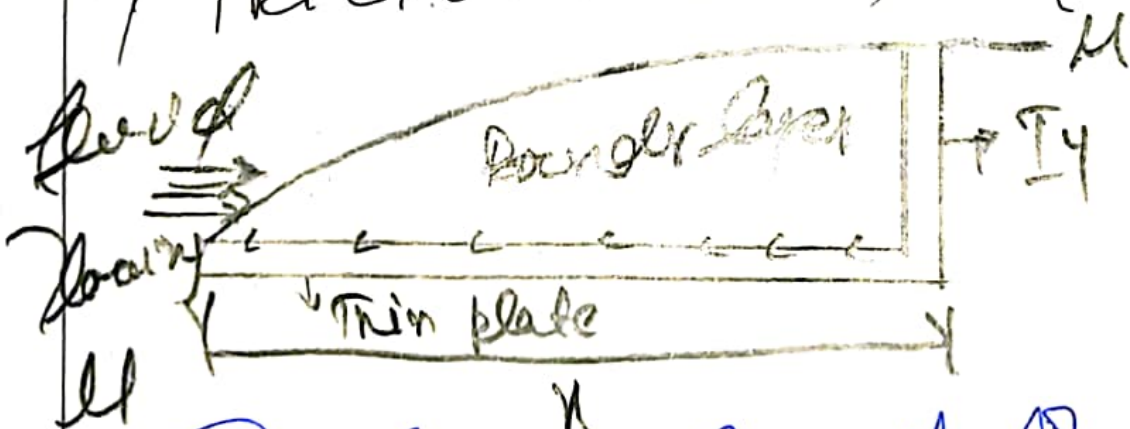


Fig shows growth of boundary layer along one side of smooth plate inside the fluid.

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~~Diagram~~
whose δ is thickness of boundary layer & is undisturbed velocity

$$\text{Thus } -F_x = \text{drag} =$$

(rate of momentum in x-direction).

(Leaving through BC + rate of momentum through AB) - rate of momentum entering through (DA).

$$\Rightarrow \Delta P = P_{out} - P_{in}$$

Thus according to momentum

$$\Sigma F = \frac{d(P)}{dt} = \frac{d m v}{dt}$$

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where

$$\frac{dm}{dt} = \int \rho \cdot \mathbf{T} \cdot \mathbf{n} \, ds$$

$$F = \int \rho \mathbf{Q} \, V$$

$$F = \int A \cdot V \cdot V$$

$$F = \int A V^2$$

$$DA \rightarrow \int U \text{ (OBS)}$$

$$BC \rightarrow \int_B \int_1^S u^2 \, dy$$

$$AB \rightarrow \int U \text{ (OBS)} - B \int_1^S u \, dy$$

putting value^o

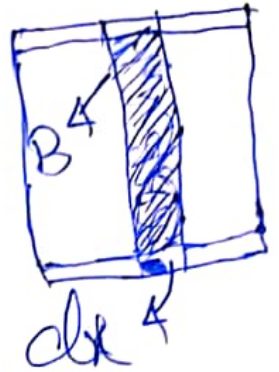
$$F_x = \int_B \int_1^S u (u - U) \, dy$$

$F_x = \int_B C u^2 f \alpha$ (when α is penetration of boundary layers.)
Now to find local wall shear \rightarrow

01/06

stress.

$$\tau_0 = \frac{dF_x}{B \cdot dx \text{-area}}$$



$$F_x = \int B U^2 \rho dx$$

$$\tau_0 = \rho U^2 \propto \frac{d\delta}{dx}$$

On general
equation of
shear stress.

LAMINAR BOUNDARY LAYER

$$\frac{u}{v} = F\left(\frac{y}{\delta}\right)$$

Assume

$$n = \frac{1}{2} \text{ or } \gamma = n\delta$$

Then $\delta \frac{u}{v} = f(\eta) \text{ or } u = v f(\eta)$

In case of laminar flow

11/07

$$\begin{aligned} \tau_0 &= \mu \left(\frac{du}{dy} \right) \\ &= \frac{\mu}{\rho} \left(\frac{d\mu}{dx} \right) = \frac{\mu \rho}{\rho} \left[\frac{d\mu}{dx} \right] \end{aligned}$$

* Solving the equation?

$$\Rightarrow \tau_0 = \frac{\mu \rho}{\rho} \rightarrow \text{A}$$

* The general equation

$$\text{CS } \tau_0 = \int u^2 dx \frac{ds}{dx}$$

* Equating both equations?

$$\Rightarrow \frac{\mu \rho}{\rho} = \int u^2 dx \frac{ds}{dx}$$

Or.

$$\Rightarrow \text{Sol } S = \frac{\mu \rho}{\int u^2 dx} dx$$

#01/08

Integrating The equation

$$\frac{S^2}{2} = \frac{uB}{f_{uv}} x + C$$

Now at $x=0$, $S=0$

Thus $C=0$

$$\frac{S^2}{2} = \frac{uB}{f_{uv}} x$$

$$S = \sqrt{\frac{2uB}{f_{uv}} x} \quad \text{or} \quad \sqrt{\frac{2B}{\alpha}} \cdot \sqrt{\frac{u}{f_{uv}} x}$$

King of King by "x"

$$S = \sqrt{\frac{2B}{\alpha}} \cdot \sqrt{\frac{u}{f_{uv}}} \cdot \sqrt{x} \cdot \sqrt{x}$$

#01/09

where $\alpha = 0.135$

$$\beta = 1.63$$

$$R_n = \frac{\rho u x}{\mu}$$

$$S = \frac{4.91}{\sqrt{DN}} \cdot x \text{ or } \frac{S}{\alpha} = \frac{4.91}{\sqrt{R_n}}$$

$$\text{Now } \tau_0 = \frac{\mu u B}{S}$$

Thus putting value

$$\tau_0 = 0.332 \frac{\mu u \sqrt{R_n}}{x}$$

where R_n is local Reynolds number now

$$F_g = R \int_0^x \frac{\tau_0 dx}{\text{stress}}$$

of 10

Putting value

$$F_f = 0.664 B \sqrt{\rho \mu U^3}$$

As general equation is

$$F_f = C_f \rho \frac{U^2}{2} BL \rightarrow \text{equating both}$$

$$C_f = 1.328 \sqrt{\frac{\mu}{\rho U}} = \frac{1.328}{\sqrt{Re}} \text{ equation.}$$

TURBULANT BOUNDARY LAYER

That velocity distribution in turbulent boundary layer shows a much steeper gradient near wall & (flatter though) out remaining layer. The shear stress is greater in turbulent than in laminar layer.

$$\tau_0 = f \frac{\rho U^2}{8} \quad \text{where } U \text{ denotes Average velocity}$$

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Now we have obtained an approximate relation b/w U & u by using Pipe Factor equation of

$$\frac{U}{u_{max}} = \frac{1}{1 + 3.3 \sqrt{f}}$$

using friction factor of 0.028 from chart which is middle critical value.

So $U = 1.235u$

Now we have

$$\tau_0 = f \rho \frac{u^2}{8} \text{ as we know}$$

$$f = \frac{0.316}{R^{0.25}}$$

$$\text{Thus } \tau_0 = \frac{0.316 R^{0.25}}{(\rho u)^{0.25}} \rho \frac{u^2}{8}$$

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where $v = \frac{U}{1.235}$ $\frac{1}{1.235}$

$$\tau_0 = \frac{0.316}{\left(\frac{\rho}{\mu} \left(\frac{U}{1.235}\right)^{\frac{1}{4}}\right)} \cdot \frac{f}{8} \left(\frac{U}{1.235}\right)^2$$

$$\Sigma D = 25$$

$$\text{Thus } \tau_0 = \frac{0.0023 f U^2}{\left(\frac{\rho \mu}{\rho}\right)^{\frac{1}{4}}}$$

As we know

$$\tau_0 = \int U^2 \alpha \frac{ds}{dx}$$

Evaluating both Σ into
graphing for boundary
condition of $x=0, s=0$

$$\text{Thus } s = \left(\frac{0.0023 f}{\alpha}\right)^{\frac{4}{5}} \left(\frac{U}{\mu x}\right)^{\frac{1}{5}} x$$

$$\text{For } \alpha = 0.05772$$

$$s/x = \frac{0.377}{\left(\frac{\rho}{\mu}\right)^{\frac{1}{5}}}$$

#01/13

$$\frac{\delta}{x} = \frac{0.377}{(R_x)^{1/5}}$$

putting values in equation

$$\tau_0 = 0.0587 \frac{\rho U^2}{2} \left(\frac{U}{\nu x}\right)^{1/5}$$

$$\text{Now } f_D = \int_0^L \tau_0 dx$$

$$f_D = 0.0735 \frac{\rho U^2 L}{2} \left(\frac{U}{\nu L}\right)^{1/5}$$

$$\text{As } f_D = C_D \int \frac{U^2}{2} BL$$

equating both

$$C_D = \frac{0.0735}{R^{1/5}} \quad R \text{ is less than } 10^7 \text{ for}$$

$$\text{For } R > 10^7 \quad 5000000 < R < 10^7$$

$$C_D = \frac{0.455}{(\log R)^{2.58}} \quad \text{Answer}$$

(Q #01)
Part-B

→ Equation for Critical Depth,
Critical velocity of rectangular
- Section of a channel?

As Specific Energy, $E = y + \frac{V^2}{2g}$

→ Depth of flow at that
point is critical depth y_c
& velocity at that point
is critical velocity V_c .

Thus: $E = y + \frac{1}{2g} \left(\frac{Q^2}{y^2} \right)$

For minimum Specific Energy

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$$\frac{dE}{dy} = 0$$

THUS:

$$\frac{dE}{dy} = 1 - \frac{2}{29} \left(\frac{v^2}{y^3} \right) = 0$$

$$= \frac{v^3}{9y^3} = 1 \Rightarrow v^2 = 9y^3$$

$$= \frac{v^2}{9} = y^3 \Rightarrow \left(\frac{v^2}{9} \right)^{\frac{1}{3}} = y$$

Now

$$v^2 = 9y^3$$

$$v = \sqrt{9y^3} \Rightarrow v^2 = 9y^3$$


or

$$v^2 = 9y^3$$

or

$$v = \sqrt{9y^3} \text{ Ans.}$$

#08/d

 #08

Find Depth of Rectangular Channel

For rate = $3.5 \text{ m}^3/\text{s}$ with bed

bed slope = 0.0008 &

$$n = 0.0219$$

width of Bed = 7724 mm

Required, ^{Rectangular channel} Critical Depth = ?
Critical Velocity = ?

Sub Critical or
Super Critical ?

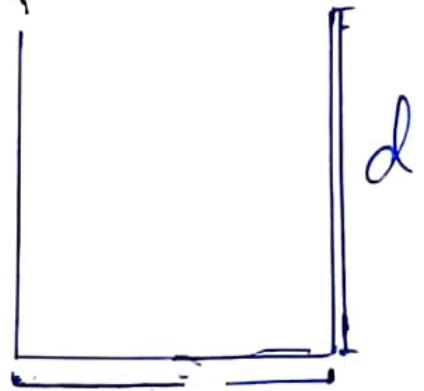
Solution of Manning Equation
 $Q = \left(\frac{1}{n} R^{2/3} S_0^{1/2} \right) A \rightarrow \text{①}$

#08/02

Solution

$$\text{Area} = 7.724 \times d.$$

$$= 7.724 d$$



$$\text{Perimeter} = d + 7.724 + d$$

$$= 7.724 + 2d$$

$$\text{Hydraulic Radius } R_h = \frac{\text{Area}}{\text{Perimeter}}$$

$$= \frac{7.724 d}{2d + 7.724}$$

put value in eqn (1)

$$Q = \left[\frac{1}{n} R_h^{2/3} \left(\frac{1}{2} \right) \right] A$$

$$3.5 = \left[\frac{1}{0.0249} \times \left(\frac{7.724 d}{2d + 7.724} \right)^{2/3} \times \left(\frac{0.0008}{7.724 d} \right)^{1/2} \right]$$

#2/3 :

$$\Rightarrow \frac{3.5 \times 0.0219}{(0.00008)^{\frac{1}{2}}} = \left(\frac{7.724}{2d+7.724} \right)^{\frac{2}{3}} \times 7.724d$$

$$\Rightarrow 2.709 = \sqrt[3]{\frac{7.724d}{2d+7.724}} \times 7.724d$$

$$\Rightarrow 2.709 = \frac{7.724d}{2d+7.724} \times 7.724d$$

$$\Rightarrow (2.709)(2d+7.724) = 59.66d$$

$$\Rightarrow (5.418d + 20.924) = 59.66d$$

$$\Rightarrow 20.924 = (59.66d - 5.418d)$$

$$\Rightarrow 20.924 = 54.242d$$

$$\Rightarrow d = \frac{20.924}{54.242}$$

$$\Rightarrow d = 0.387m \text{ Answer.}$$

#102/04

⇒ So the Depth of channel ⇒

$$\text{is } \boxed{0.387 \text{ m}}$$

Now

As $q_0 = \text{discharge}$
per unit width

$$q_0 = \frac{Q}{b}$$

$$= \frac{8.5}{7.724}$$

$$q_0 = 0.453$$

⇒ CRITICAL DEPTH (ycr)

using equation

$$y_{cr} = \left(\frac{q^2}{g} \right)^{1/3} \Rightarrow \left(\frac{(0.453)^2}{9.81} \right)^{1/3}$$
$$= \left(\frac{0.2052}{9.81} \right)^{1/3} = 0.156 \text{ m}$$

02/05

$$\underline{y_{cr} = 0.156 \text{ m}}$$

→ Critical velocity, V_{cr}
using eqn $g y_{cr} = V_{cr}^2$

$$\rightarrow V_{cr} = \sqrt{(9.81)(0.156)}$$

$$\rightarrow V_{cr} = 0.765 \text{ m/s} \quad V = \frac{Q}{A} = \frac{3.5}{7.7}$$

→ y & R_d are
same

$$\underline{V = 0.175 \text{ m}}$$

$$\rightarrow \underline{y = 0.387 \text{ m}} \quad \underline{y_{cr} = 0.156 \text{ m}} \quad \underline{V_{cr} = 0.765 \text{ m/s}}$$

As $y > y_{cr}$ & $V < V_{cr}$
∴ flow is Subcritical

03/01

DATA of

(Q #03)

Friction Drag on one side of a smooth plate 200mm wide & 800 mm length longitudinally.

oil specific Gravity = 0.89

The v. disturb velocity = 5 m/s

Kinematic Viscosity = $0.93 \times 10^{-4} \text{ m}^2/\text{s}$

Required Data friction drag

Solution of \rightarrow on one side of a smooth plate, $F_f = ?$

Check the flow

~~Re~~ $\nu = 0.93 \times 10^{-4} \text{ m}^2/\text{s}$

$$R = \frac{U \mu}{\nu} = \frac{(0.8)(5)}{0.93 \times 10^{-4}}$$

#03/02

$$R = 43010.75 < 500,000$$

Thus flow is laminar.

$$\frac{R_{low}}{cf} = \frac{1.328}{\sqrt{R}} = \frac{1.328}{\sqrt{43010.75}}$$

$$cf = 6.4 \times 10^{-3}$$

$$cf = 0.0064$$

$$\Rightarrow f_f = cf \rho \frac{V^2}{2} BL$$

$$= (0.0064) (\text{soil} \times \text{water}) \times \frac{(5)^2}{2} \times$$

$$(0.2) (0.8)$$

$$= (0.0064) (0.89 \times 1000) \times \frac{5^2}{2} \times$$

$$(0.2) (0.8)$$

$$\Rightarrow f_f = 11.892 \text{ N}$$

Answer.