

P: (2)

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Section : " B "

Subject : High way & Traffic Engr

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Q NO I :

Part (a) :

What is different b/w Flexible & rigid Pavement ?

Ans: Flexible Pavement : Rigid Pavement

1: Bitumen is used a binder in flexible Pavement → Cement is used a binder in rigid Pavement.

2: → Deformation in the sub grade is transferred to the upper layer. → Deformation in the sub-grade is not transferred to sub-sequent layer.

3: Load is transferred by grain to grain contact. → No such phenomenon of grain to grain load transfer exist.

4: Flexible Pavement have low initial construction cost but have high maintenance cost. → Rigid Pavement have low initial construction cost but have high maintenance cost.

(P:2)

- 5: It have low life span \Rightarrow Life span is more as usually 10 to 15 Year: compare to flexible usually cost.
- 6: Surfacing cannot be laid directly on the subgrade but a sub-base is needed \Rightarrow Surfacing can be directly ~~the~~ span laid on the subgrade.
- 7: Road can be used for traffic within 24 hr. \Rightarrow Road cannot be used until 14 days of curing

2: Part "B"

What are the advantages of water bound over wet mix macadam?

Ans: Following are the advantages of water bound over wet mix macadam:

- 1: Water bound macadam is slightly cheaper than the wet-mix macadam. The reason is (WBM) requires general labours while wet mix macadam requires mixer plant & power.

(P: 3)

2: Aggregate of WBM can be broken by hand while the (M.M.M) need a crusher for the disintegration of Aggregates.

3: The interlocking of aggregate particles impart adequate strength of a material for filling the voids. These ensure non-entry of the plastic material of the subgrade into voids.

1 (Part: c)

Ans: Bitumen is actually the liquid binder that hold asphalt together.

*1 Asphalt is used as a term to refer to the combination of bitumen & gravel specially for road construction.

A.T.O.

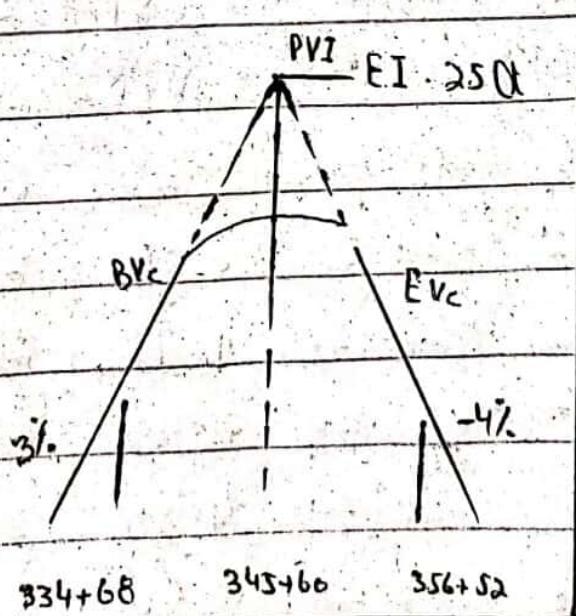
(P:4)

* Bitumen is known for being strongly adhesive & resistance to damage from water & oil spills, this make bitumen the ideal binder for asphalt because asphalt is commonly used as a surface for road, car and parks etc.

(P: 5)

QNO: 2

A crest vertical curve joining a 3% grade to a -4% grade is to be designed for 75 m/s. Also calculate the elevation of intermediate point on the curve.



P.T.O

(P:6)

Solution:

For a design speed of 75 mph,

$$K = 312 \quad [\text{From table}]$$

$$\Rightarrow \text{Minimum length} = 312 \times [3 - (-4)] = 2184 \text{ ft}$$

$$\Rightarrow \text{Station of BVC} = [345 + 60] - \left(\frac{21 + 84}{2}\right) = 334 + 68$$

$$\text{Station of PVI} = [334 + 68] + [21 + 84] = 356 + 52$$

$$\Rightarrow \text{Elevation of BVC} = 250 - \left[0.03 \times \frac{2184}{2}\right] = 217.24 \text{ ft}$$

The remainder of the computation

is accordingly done using the formula

shown in table.

Station	Distance BVC (ft)	Tangent Elevation (ft)	Offset $(y = \frac{ax^2}{200^2})$ (ft)	Curve elevation tangent offset (ft)
BVC 334+68	0	217.24	0.01	217.24
BVC 335+00	32	$217.24 + \frac{32}{100} \times 3$	0.02	218.18
BVC 336+00	132	221.20	0.28	220.92
BVC 337+00	232	224.20	0.86	223.34
BVC 338+00	332	227.20	4.77	225.43
BVC 339+00	432	230.20	2.99	227.21
BVC 340+00	532	233.20	4.54	228.66
BVC 341+00	632	236.20	6.40	229.86
BVC 342+00	732	239.20	8.59	230.61
BVC 343+00	832	242.20	11.09	231.11
BVC 344+00	932	245.20	13.92	231.25

(P:7)

Buc 345+00	1032	248.20	17.07	231.13
Buc 346+00	1132	251.20	20.54	230.66
Buc 347+00	1232	254.20	24.32	229.88
Buc 348+00	1332	257.20	28.83	228.77
Buc 349+00	1432	260.20	32.80	227.34
Buc 350+00	1532	263.20	37.61	225.59
Buc 351+00	1632	266.20	42.68	223.052
Buc 352+00	1732	269.20	48.07	221.13
Buc 353+00	1832	272.20	53.79	218.41
Buc 354+00	1932	275.20	59.82	215.38
Buc 355+00	2032	278.20	66.17	212.03
Buc 356+00	2132	281.20	72.84	208.36
Buc 356+50	2182	282.76	76.44	206.32

(P: 8)

QNO # 3:

step 1: Answer:

⇒ Draw a line joining the reliability level OD 99%

& the overall standard deviation so OD 0.49

extend line to intersect the First (TL) - Line at Point A.

step # 2:

⇒ Draw a line joining Point

A to the ESAL OD

2×10^6 & extend this line to intersect the First TL

Line at Point "B"

step # 3:

⇒ Draw a line joining Point 'B'

& resilient modulus (M_r)

OD base course & extend this

line to intersect the design

serviceability loss chart at Point 'C'

(P: 9)

Step # 4:

⇒ Draw a Horizontal line
From Point (C) to intersect
the design serviceability

⇒ loss (PSI) curve at Point 'D'
So here

$$\Delta PSI = 4.5 - 2.5 = \boxed{2}$$

step # 5:

⇒ The structure number
require to protect the
base course & to
Find the thickness D_1
of the surface course
is 2.6

step # 6:

⇒ Determine the appropriate
structure layer coefficient
For each construction
material Resilient value
of ASPHALT = 450,000 lb/in²

Here D_{ore} :

$$a_{11} = 0.44$$

(P: 10)

$$D_1 = SN_1 / a_1$$

$$= 2.6 / 0.44$$

$$D_1 = 5.9''$$

Thickness should be taken

to the nearest 0.5 inches

so that the thickness of

the surface course is 6''.

$$SN_1 = D_1 \times a_1$$

$$= 6 \times 0.44 = 2.64$$

$$SN_1 = 2.64$$

⇒

NOW Find SN_2 & D_2 (Base course)

Find the value of a_{12} from

layer coefficient table of M_2

from drainage coefficient

table.

⇒

Thickness of base course (D_2)

$$D_2 = [SN_2 - SN_1] / a_{12} m_2$$

$$D_2 = [3.8 - 2.64] \cdot 14 \times 0.80$$

$$D_2 = 10.36''$$

(P: 11)

use 12"

So the thickness of base
course is 12"

$$\Rightarrow SN_2 = 0.14 \times 0.80 \times 12 + SN_1$$

$$\Rightarrow SN_2 = 1.34 + 2.64$$

$$\boxed{SN_2 = 3.98}$$

\Rightarrow

Finding SN_3 & D_3 (sub-base course)

q_3 also = layer coefficient

m_2 & drainage coefficient

m_2 from their respective

table -

$$\Rightarrow D_3 = \frac{SN_3 - SN_2}{q_3 m_2}$$

$$D_3 = \frac{4.4 - 3.98}{0.10 \times 0.80}$$

$$\boxed{D_3 = 5.24''}$$

the mill used 6" as

a sub-base:

$$SN_3 = 2.64 + 1.34 + 6'' \times 0.10 \times 0.80$$

$$\boxed{SN_3 = 4.46}$$

74.4 ok.

Final design :

⇒ Surface course = 6"

⇒ Base course = 12"

⇒ Sub-base = 6"

Total Pavement Thickness = 24"



QNO: 4

In what are the different
Pavement distresses ?

Following are the different
Pavement distresses.

1. Alligator cracking:

→ Possible cause:

- * Over load
- * Inadequate structural design
- * Poor construction

→ Repair:

- * Crack sealing is ineffective...
- * Digout & replace area of poor sub grade.

(13)

2: Block cracking :

→ Problem :

Allows moisture infiltration.

→ Possible cases:

- * HMA shrinkage
- * Asphalt binder aging
- * Poor choice of asphalt binder in the mix design.

→ Repair :

→ Low severity cracks
($< \frac{1}{2}$ inch wide) cracks seal
to prevent entry of
moisture .

→ High severity cracks
($> \frac{1}{2}$ inch wide & crack
with raveled edges) Remove
and replace the cracks
pavement layer on overlay.

3: Potholes:

= Small bowl-shaped depression in the pavement surface that penetrate all the way through the HMA layer down to the base course.

⇒ Potholes are mostly likely to occur on road with the thin HMA surface.

→ Problem:

Roughness, moisture infiltration

⇒ cases Problem:

Generally Potholes are the end result of fatigue cracking. As fatigue crackings creates small cracks of pavement which can be dislodged as vehicles drive over them.

Repair:

Polishing techniques.

4: Rutting:

→ Surface depression in the wheel path, are particularly evident after a rain when they are filled with water.

→ Surface depression in the wheel path are particularly evident after a rain when they are filled with water.

Possible causes:

→ In sufficient compaction of HML A layer during construction.

→ Sub-grade rutting (e.g. a result of adequate pavement structure)

→ improper mix design.

Repair:

slight ruts ($< \frac{1}{3}$ inch deep)
 can generally be left
 untreated - Pavement with
 deeper ruts should be
 leveled and overlaid.

5: Bleeding:→ Problem:

Loss of skid
 resistance when wet.

→ Possible causes:

- Excessive asphalt binder in the HMA.
- Excessive application of asphalt binder during BST application.
- Low HMA air void content.

6: Polished Aggregate:

⇒ Possible causes:

Repeated traffic application this can occur quicker if the aggregate is susceptible to abrasion.

⇒ Repair:

Apply a skid resistance slurry seal. BST or non structural overlay.

7: Reveling:

Loose debris on the pavement which increase pavement roughness and loss of skid resistance.

⇒ Possible causes:

⇒ Asphalt binder aging.

⇒ Aggregate regradation. If

fine particles are missing from the

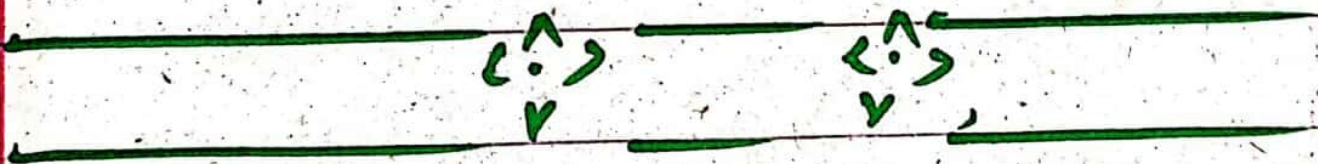
18

Aggregate matrix

⇒ Inadequate compaction
during construction.

⇒ Repair:

Fog seal / slurry seal
or Remove the
damaged Pavement
& overlay.



The
End!