

MUHAMMAD FURQAN

7802

NAME	MUHAMMAD FURQAN
ID :	7802
SECTION	" A "
SEMESTER	6 <sup>th</sup>
SUBJECT:	HIGHWAY and TRAFFIC ENGINEERING
INSTRUCTOR:	Dr. NADEEM ANWER
EXAM:	FINAL EXAM SPRING SEMESTER, 2020.

PART: (a)QUESTION: 01FLEXIBLE PAVEMENT

1. It typically distribute weight loads to lower layers of pavement section
2. It consists generally of bituminous material
3. Deformation in subgrade is transferred to upper layers.
4. It has low initial construction cost but high maintenance cost
5. Load is transferred by grain to grain contact.
6. Surfacing can not be laid directly on to subgrade therefore subbase is required.
7. Life span is low i.e usually 10-15 years.
8. Strength of road is highly dependent on strength of subgrade
9. Roads can be used for traffic within 24 hrs.

RIGID PAVEMENT

1. It typically distribute wheel load over a wide area of sub-grade
2. It generally consists of cement concrete and may be reinforced with steel
3. Deformation in subgrade is not transferred to subsequent layers.
4. It has low maintenance cost, but high initial construction cost.
5. No such load transfer by grain to grain contact exists.
6. Surfacing can be laid directly on subgrade
7. long life span as compared to flexible pavement i.e 30+ years.
8. strength of road is less dependent on strength of subgrade
9. Roads can't be used till 14 days of curing.

PART: (b)

The advantage of water bound over wet mix macadam is that water bound macadam is cheaper as less costly as compared to wet mix macadam which is costlier. The reason for high cost of wet mix macadam is that specification involve the use of mixing plant and power while water bound macadam is a labour oriented specification.

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PART: (C)ASPHALT

1. A dark brown to black cementitious material in which predominating constituents are bitumen which occur in nature or obtained in fractional distillation of petroleum (crude oil) along with certain Mineral Matter.
2. Asphalt refers to combination of bitumen and gravel specifically for road construction.
3. Surface made of Asphalt is smoother & more skid resistant, ensuring drivers' safety.
4. Reduced friction between tire and car, meaning better fuel economy and minimization of CO<sub>2</sub> emission.
5. Installation is relatively costlier.

BITUMEN

1. A class of black or dark coloured cementitious substance which might be solid, semi-solid or viscous, natural or manufactured, composed principally of high molecular weight hydrocarbons found in Asphalts.
2. Bitumen is actually liquid binder that holds asphalt together.
3. Loose fragments on bitumen pavement make driving experience noisier and can wear down tires, therefore cause safety issues.
4. Higher frictional resistance of bitumen pavement means less efficiency in energy utilization.
5. Cheap to install as compared to asphalt.

QUESTION: 02REQUIRED:

Station of BVC = ?

Station of EVC = ?

Elevation of BVC = ?

Elevation at intermediate points on curve at whole station.

SOLUTION:

For a design speed of 75 mi/h.

$$K = 312$$

We know

$$\text{Minimum length} = 312 [3 - (-4)] = 2184 \text{ ft}$$

$$\text{Station of BVC} = (345 + 60) - \left(\frac{21 + 84}{2}\right) = 334 + 68$$

$$\text{Station of EVC} = (334 + 68) + (21 + 84) = 356 + 52$$

$$\text{Elevation of BVC} = 250 - (0.03 \times \frac{2184}{2}) = 217.24 \text{ ft.}$$

Station	Distance from BVC, ft	Tangent Elevation (ft)	offset $y = \left(\frac{Ax^2}{200L}\right)$ (ft)	Curve Elevation (Tangent elevation - offset) ft
BVC 334 + 68	0	217.4	0.01	217.24
BVC 335 + 06	32	$217.4 + \frac{32^2}{100} \times 3 = 218.20$	0.02	218.18
BVC 336 + 00	132	221.20	0.28	220.92
BVC 337 + 00	232	224.20	0.86	223.34
BVC 338 + 00	332	227.20	1.77	225.43
BVC 339 + 00	432	230.20	2.99	227.21
BVC 340 + 00	532	233.20	4.54	228.66
BVC 341 + 00	632	236.20	6.40	229.80
BVC 342 + 00	732	239.20	8.59	230.61
BVC 343 + 00	832	242.20	11.09	231.11
BVC 344 + 00	932	245.20	13.92	231.28
BVC 345 + 00	1032	248.20	17.07	231.13
BVC 346 + 00	1132	251.20	20.54	230.66
BVC 347 + 00	1232	254.20	24.32	229.88
BVC 348 + 00	1332	257.20	28.43	228.77
BVC 349 + 00	1432	260.20	32.86	227.34
BVC 350 + 00	1532	263.20	37.61	225.59

BVC	351+00	1632	266.20	42.68	223.52
BVC	352+00	1732	269.20	48.07	221.13
BVC	353+00	1832	272.20	53.79	218.41
BVC	354+00	1932	275.20	59.82	215.38
BVC	355+00	2032	278.20	66.17	212.03
BVC	356+00	2132	281.20	72.84	208.36
BVC	356+52	2184	282.76	76.44	206.32
BVC	358+18/2				

QUESTION: 03Given Data:

Resilient Modulus of Asphalt concrete at 68°F  
450,000 lb/in<sup>2</sup>

CBR value of Base course 100. Mr. 31,000 lb/in<sup>2</sup>

CBR value of sub base course material 22  
Mr. 13,500 lb/in<sup>2</sup>

CBR value of subgrade material = 6  
Mr of subgrade 6 = 1500 lb/in<sup>2</sup> = 9000 lb/in<sup>2</sup>.

ANSWER:

Step:1 Draw a line joining the reliability level of 99% and overall standard deviation  $\sigma$  of 0.49. and extend line to intersect the first TL line at point A.

STEP: 02

Draw a line joining point A to ESAL of  $2 \times 10^6$  and extend this line to intersect first line TL at point B.

STEP: 03

Now, Draw a line joining point B and resilient Modulus (Mr) of base course and extend this line to intersect the design serviceability loss chart at point C.

STEP: 04

Now, Draw a horizontal line from point C to intersect the design serviceability loss (PSI) curve at point D, so here  
 $APSI = 45 - 2.5 = 2.$

The structure numbers require to protect the base course and to find thickness  $D_1$  of surface course is 2.6.

STEP: 06.

Determine appropriate structure layer co-efficient for each construction material. Resilient value of asphalt = 450,000 lb/in<sup>2</sup> therefore  $a_1$  0.44

$$D_1 = SN_1 / a_1$$

$$= 2.6 / 0.44 = 5.9''$$

Thickness should be taken to the nearest 0.5'' so the thickness of surface course is 6''.

$$SN_1 = D_1 \times a_1$$

$$= 6 \times 0.44 = 2.64$$

Now find  $SN_2$  and  $D_2$  (Base course)

find value of  $a_2$  from layer co-efficient table and  $m_2$  from drainage coefficient table

→ Thickness of Base course ( $D_2$ )

$$D_2 = (SN_2 - SN_1) / a_2 m_2$$

$$= (3.8 - 2.64) / 14 \times 0.80$$

$$D_2 = 10.36''$$

Now use 12''

So thickness of Base course is 12''

$$SN_2 = 0.14 \times 0.80 \times 12 \times SN_1$$

$$= 1.34 + 2.64$$

$$SN_2 = 3.98$$



→ Finding  $SN_3$  and  $D_3$  (sub base) and also layer co-efficient  $a_3$  and drainage co-efficient  $m_2$  from their respective table.

$$D_3 = (SN_3 - SN_2) / a_3 m_3$$

$$= (4.4 - 3.98) / 0.10 \times 0.80$$

$$D_3 = 5.24$$

We will use 6" as a sub base

$$SN_3 = 2.64 + 1.34 + 6" \times 0.10 \times 0.80$$

$$SN_3 = 4.46 > 4.4 \text{ okay.}$$

### FINAL DESIGN:-

⇒ Surface course = 6"

⇒ Base course = 12"

⇒ sub base = 6"

⇒ Total pavement thickness = 24"

## QUESTION : 04

PAVEMENT DISTRESS:: It can be defined as condition of pavement structure that reduces serviceability or may lead to reduction in service life.

It might occur due to unstable mixes or higher wheel loads than those considered in design.

TYPES:: The types include following:

1. ALLIGATOR CRACKING: Alligator cracks are interconnected cracks that are found on roads subjected to heavy traffic or severe climate. It resembles chicken wire or alligator skin, therefore called as alligator cracking.

CAUSES:: The possible causes include

1. poor construction
2. overloading
3. Inadequate structural design
4. severe climatic changes.

REPAIR:: Repair is possible in following ways.

1. Dig out and replace area of poor subgrade
2. Crack sealing is in effective

## 2. BLOCK CRACKING:

It's a pattern of cracks that divide the pavement into approximately rectangular pieces. This is called Block cracking.

It allows moisture infiltration.

CAUSES: It may be caused due to

1. poor choice of asphalt Binder
2. Asphalt Binder aging
3. HMA shrinkage

REPAIR: It can be repaired by

1. Low severity cracks ( $< \frac{1}{2}$  inch wide)
  - crack seal to prevent entry of moisture
2. High severity cracks ( $> \frac{1}{2}$  inch crack with raveled edges)
  - Remove and replace cracked pavement layer with overlay

8. BLEEDING: It occurs due to loss of skid resistance when wet.

CAUSES: The causes of Bleeding include

1. Low HMA air void content
2. Excessive Asphalt Binder in HMA
3. Excessive application of Asphalt Binders during BST application

#### 4. POLISHED AGGREGATE

It occurs when portion of aggregate extending above asphalt is either very small or there are no rough or angular aggregate particles to provide good skid resistance.

##### CAUSES:

- Repeated Traffic application
- This occur quicker if aggregate is susceptible to abrasions.

##### REPAIR:-

Apply a skid resistant slurry seal, BST or non structural overlay.

5. RUTTING: Rutting is the surface depression in wheel path, are particularly evident after a rain when filled with water.

##### CAUSES:-

1. Improper Mix design (e.g Excessive high asphalt content).
2. subgrade rutting (e.g inadequate pavement structure).
3. Insufficient compaction of HMA layer during construction.

REPAIR:

- Slight Ruts ( $< 1/3$  inch deep) can generally be left untreated
- Pavement with deeper ruts should be leveled and overlaid.

6. POTHOLES:

These are small, bowl shaped depressions in the pavement surface that penetrate all the way through HMA layer down to the base course.

Roughness and moisture infiltration occur.

CAUSES: It generally occurs due to fatigue cracking. When fatigue cracking becomes severe, interconnected cracks create small chunks of which can be dislodged as vehicle passes.

REPAIR:

patching technique is used for repair.

7 RAVELING: Raveling is basically loose debris on pavement which increase pavement roughness and result in loss of skid resistance.

CAUSES:

- ① Asphalt Binder aging,
- ② Inadequate compaction during construction
- ③ Aggregate segregation.

REPAIR: - Fog seal / slurry seal  
- Remove damaged pavement & overlay