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Subject = Mos 2

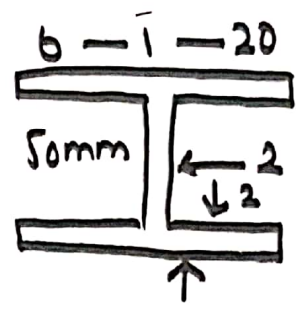
Submitted to = Sir Saqib Khan

Question No # 1 Part (A)

Required :- Location of Shear centre

Solo:- As we know

$$e = \frac{t f h^2 b^2}{4I}$$



and;

$$I = 2 \left(\frac{bh^3}{12} + Ay^2 \right) + \left[\frac{bh^3}{12} + Ay^2 \right]$$

$$= 2 \left[\frac{26(2)^3}{12} + (26 \times 2)(25)^2 \right]$$

$$+ \left[\frac{2(50)^3}{12} + 0 \right]$$

$$I = 50034.66 + 20833$$

$$I = 70867.99 \text{ mm}^4$$

$$e = \frac{2(50)^2(25)^2}{4(70867.99)} = 11.02 \text{ mm}$$

So Shear Centre
 $e = 11.02 \text{ mm}$

Question No 1
part (b)

(2)

Data:-

$$H = 26 \text{ ft}$$

\Rightarrow Assume diameter

$$D = 22 \text{ ft}$$

$$\Rightarrow \text{tangential stress} = 600 \text{ lb/ft}^3$$

\Rightarrow Specific weight of water

$$\text{tank} = 62.4 \text{ lb/ft}^3$$

we have to find the
thickness = ?

3

Solution:-

The pressure developed by

water $p = \gamma h$

$$\delta t = \frac{PD}{2t}$$

$$\delta t = \frac{PD}{2t} = \frac{\gamma h D}{2t}$$

$$2t \times \delta t = \gamma h D$$

$$2t = \frac{\gamma h D}{\delta t}$$

$$t = \frac{\gamma h D}{\delta t \times 2}$$

$$t = \frac{(62.4) \times (26 \times 12) \times (22 \times 12)}{(12)^3}$$

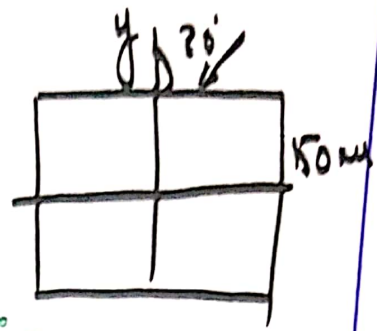
$$6000 \times 2$$

$$t = 0.24''$$

Question No # 2

Part (A)

(4)



Moment of inertia:-

$$I_z = \frac{bh^3}{12} = \frac{0.1(0.15)^3}{12} = I_z = 2.8125 \times 10^{-5}$$

Now

$$I_y = \frac{hb^3}{12} = \frac{0.15(0.1)^3}{12}$$

$$I_y = 1.25 \times 10^{-5}$$

$$I = \frac{M_z y}{I_z} + \frac{M_y z}{I_y}$$

$$I = \frac{M \cos \theta}{I_z} + \frac{M \sin \theta}{I_y}$$

where

$$M \cos \theta = P \cos \theta = M_z$$

$$M_z = 1.8 \cos 60^\circ = M_z = 1.8510$$

$$M \sin \theta = P \sin \theta = M_y$$

(5)

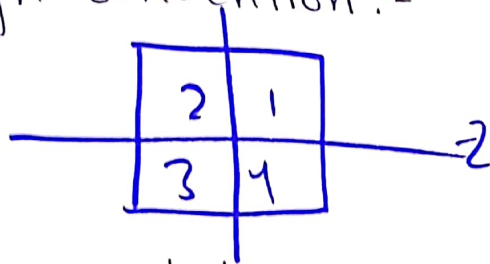
$$M_y = 12 \sin 60^\circ$$

$$M_y = -11.8563$$

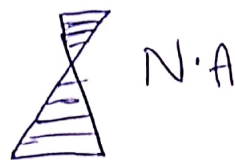
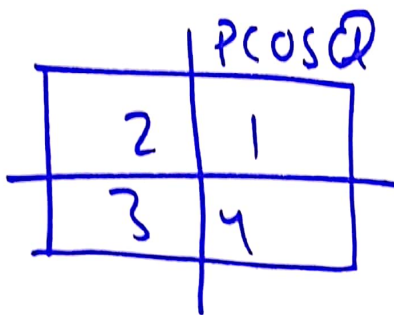
$$\sigma = \left(\frac{M_x}{I_x} \right) + \left(\frac{M_y}{I_y} \right)$$

$$\sigma = \frac{1.851}{2.812 \times 10^{-5}} + \frac{(-11.8563)}{(1.25 \times 10^{-5})} = 882628 \text{ N/m}^2$$

Sign Convention:-

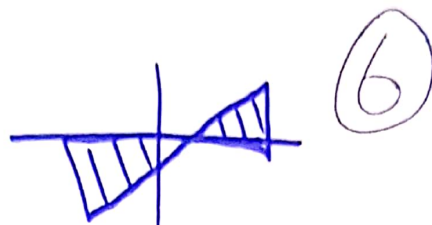
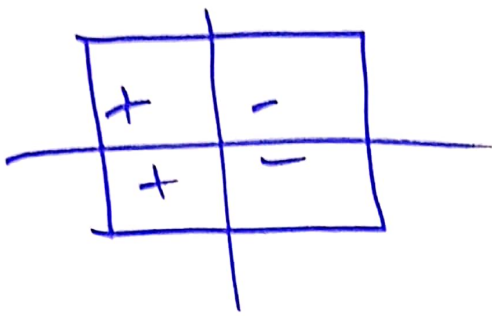


If we take compression as negative and tension as positive and the beam is a simply supported.



Quadrant 1, 2 \Rightarrow -ive

Quadrant 3, 4 \Rightarrow +ive



Quadr 1,4 -ive
Quadr 2,3 +ive

In case of unsymmetrical loading the neutral axis lies at an angle algebraic sum of stress at

N.A is zero

$$\sigma = \frac{M \cos \alpha}{I_z} y + \frac{M \sin \alpha}{I_y} z \quad \text{--- } \textcircled{1}$$

In this case, N.A passes through

2,4

$$\sigma = \frac{M \cos \alpha}{I_z} y + \frac{M \sin \alpha}{I_y} z$$

Let consider a point "A" on

N.A lies Quadrant ², where

- Bending stress due to $P \cos \alpha$ is compressive and

- Bending stress due to $P \sin \alpha$ is Tensile.

$$\text{eq (i)} \Rightarrow 0 = \frac{-M \cos \theta y_A}{I_z} + \frac{M \sin \theta z_A}{I_y} \quad \text{(i)}$$

$$\Rightarrow 0 = -\frac{M \cos \theta y_A}{I_z} + \frac{M \sin \theta z_A}{I_y}$$

$$\Rightarrow \frac{M \cos \theta y_A}{I_z} + \frac{M \sin \theta z_A}{I_y}$$

$$\frac{y_A}{z_A} = \frac{I_z \sin \theta}{I_y \cos \theta} \Rightarrow \tan \alpha = \frac{I_z}{I_y} \tan \theta \quad \text{(ii)}$$

Now put values of I_z , I_y and θ in eq (ii)

$$\tan \alpha = \frac{I_z}{I_y} \tan 30$$

$$\Rightarrow \tan \alpha = \frac{3.8125 \times 10^{-5}}{1.25 \times 10^{-5}} (\tan 30^\circ)$$

$$\tan \alpha = -14.4129$$

$$\alpha = \tan^{-1}(-14.4129)$$

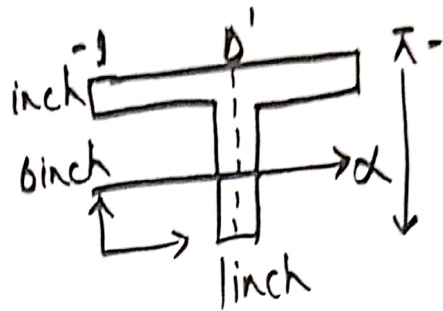
$$\alpha = 1.5^\circ$$

$$\alpha = 1^\circ 30' 5''$$

Question No # 2

Part (B)

Given:



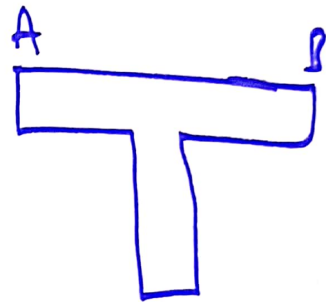
$$L = 16 \text{ ft}$$

$$I_x = 112.6 \text{ inch}^4$$

$$I_y = 18.7 \text{ inch}^4$$

$$\sigma_e = 12000 \text{ psi}$$

$$\sigma_t = 5000 \text{ psi}$$



Solution:

By looking the figure we can judge that maximum compression would occur on "a" and maximum tension c at B. There will tension as well a compression which will reduce that effect of each other so we will calculate stress at A & C.

So,

$$S_A = \frac{M_{xy}}{I_x} + \frac{M_{yx}}{I_y}$$

compression

(a)

$$S_C = \frac{M_{xy}}{I_x} + \frac{M_{yx}}{I_y}$$

(Tension)

Now M_x and M_y

So

$$M_x = P \cos 60^\circ \times (16 \times 12)$$

$$M_x = \frac{48 P \cos 60^\circ}{4}$$

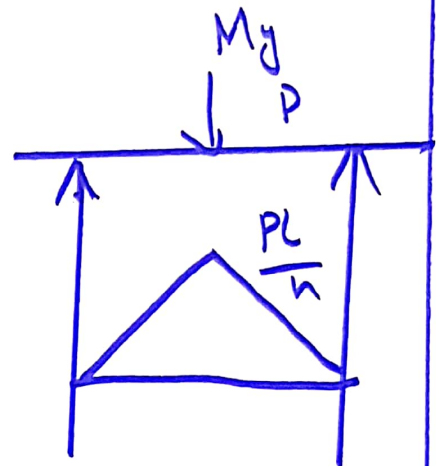
$$M_y = \frac{P \sin 60^\circ (16 \times 12)}{4}$$

$$M_y = 48 P \sin 60^\circ$$

Now

$$S_A = \frac{M_{xy}}{I_x} + \frac{M_{yx}}{I_y}$$

$$\Rightarrow 1200 = \frac{48 P \cos 60^\circ \times 3.07}{112.6} + \frac{48 P \sin 60^\circ \times 30}{18.7}$$



Solving the equation

(10)

$$P = 1638.6 \text{ lb}$$

Now

$$S_c = \frac{M_{xy}}{I_x} + \frac{M_{yx}}{I_y}$$

$$5000 = 48P \cos 60 \times (593) + \frac{48P \sin 60 \times 0.5}{18.7}$$

Solving the equation.

$$P = 2104.9 \text{ lb}$$

For Case 1

$$P_{cr} = \frac{n\pi^2 ET}{l^2}$$

So the maximum load of P applied should 1638.6 lb

Question No #3

11

Given Data:-

$$\text{Length} = 10 \text{ ft}$$

$$E = 10.3 \times 10^6$$

$$b = 0.75$$

$$h = 2$$

$$\text{Factor of safety} = 2$$

Required:-

(a) Safe load at hinged = ?

(b) Safe load at fixed = ?

Solution:-

(a) For hinged columns

$$L_e = L$$

$$I = \frac{I_x}{12} = \frac{(0.75)(2)^3}{12} = 0.5 \text{ in}^4$$

$$P_{cr} = \frac{n^2 EI \pi^2}{L_e^2} = \frac{(1)^2 (10.3 \times 10^6) (0.5) \pi^2}{(10 \times 12)^2}$$

$$P_{cr} = \frac{50776940}{14400} = 3526.176 \text{ lb}$$

$$P_{\text{safe load}} = \frac{P_{cr}}{\text{factor of safety}} = \frac{3526.176}{2} = 1763.088 \text{ lb}$$

(b) fixed column:-

$$L_e = L/2 \quad (\text{for fixed ended})$$

$$L_e = \frac{10}{2} = 5 \text{ ft}$$

$$I = I_y = 2 \times \frac{(0.75)^3}{12} = 0.07 \text{ in}^4$$

$$P_{cr} = \frac{n^2 EI \pi^2}{L_e^2} = \frac{(1)^2 (10.3 \times 10^6) (0.07) (3.14)^2}{(5 \times 12)^2}$$

$$P_{cr} = \frac{7108771.6}{(60)^2}$$

$$P_{cr} = 1974.658 \text{ lb}$$

$$P_{\text{safe load}} = \frac{1974.658}{2}$$

$$= 987.3293 \text{ lb}$$