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SEC - A

Subject - Fluid Mechanics II

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## Qno 1 (A)

Ans:-

### Forced on Immersed Bodies:-

A body which is wholly immersed in a homogeneous fluid may be subjected in two kind of forces arising from relative motion b/w body and fluid. These forces are termed as drag and lift. depending on forces either parallel or right angle to motion.

Drag Force on submerged body can have 2 components.

#### 1) Pressure Drag (FP):-

It is equal to the integration of ~~component~~ component also direction of motion of all pressure forces exerted on surface of body.

$$F_p = C_p \cdot \int \frac{\rho v^2}{2} \cdot A \quad \text{where } C_p \text{ depend on shape}$$

#### 2) Friction Drag:-

It is equal to integration of component of all shear stress along the surface in direction of motion.

$$F_f = C_f \cdot \int \frac{\rho v^2}{2} (BL)$$

#### • Friction Drag of Boundary layer:-

$$\bar{C}_D = \mu \frac{u \frac{d(u)}{dy}}{\rho \frac{d(u)}{dy}}$$

$$\bar{C}_D = \frac{\mu u \cdot B}{\rho S}$$

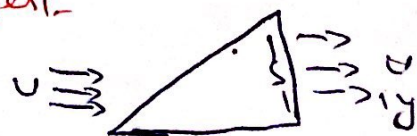
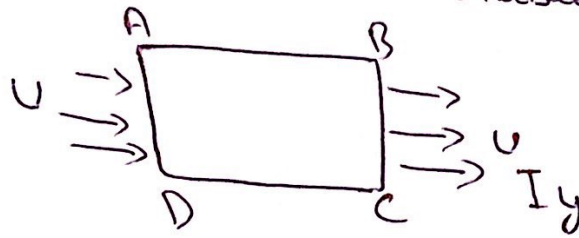


Figure shows growth of boundary layer along one side of smooth plate in steady flow of incompressible fluids. Consider value where  $S$  is the thickness of boundary layer &  $U$  is undisturbed velocity.



As we have  $\sum F_x = 0$

where

$$F_x = \frac{\Delta P}{\Delta t} = \frac{\Delta m v}{\Delta t}$$

$$F_x = \frac{\Delta \rho \cdot \text{vol} \cdot v}{\Delta t} = \Delta \rho \Delta v$$

$$F_x = \Delta \rho \Delta v$$

$F_x =$  rate of change  $BC + AB - AD$

$$AD = \int U (USB)$$

$$BC = \int B (U^2 dy)$$

$$AB = \int U (USB) - B \int_0^S U dy$$

$$F_x = \int B \int_0^S U (U - U) dy - 0$$

Integrate b.s

$$F_x = \int B U^2 S dx$$

where  $a$  is a function of boundary layer

Now to find shear stress

$$\tau = \frac{F_x}{A} = \frac{dF_x}{B dx} = \frac{dF_x}{B dx}$$

$$\bar{Z}_0 = \int \rho U^2 \alpha \frac{ds}{\rho \alpha u} = \int U^2 \alpha \frac{ds}{du}$$

$$\bar{Z}_0 = \int U^2 \alpha \frac{ds}{du} \quad - 1$$

Laminar Boundary layer.

$$\frac{u}{U} = f\left(\frac{y}{\delta}\right) \quad - 1$$

$$\frac{y}{\delta} = n \Rightarrow y = \delta n$$

$$dy = \delta dn \quad - 2$$

$$\frac{u}{U} = f(n)$$

$$du = U df(n) \quad - 3$$

For laminar flow

$$\bar{Z}_0 = \mu \frac{du}{dy} \quad - 4$$

$$\bar{Z}_0 = \mu \frac{U df(n)}{\delta dn}$$

$$\bar{Z}_0 = \frac{\mu U^2}{\delta}$$

As we have

$$\bar{Z}_0 = \int U^2 \alpha \frac{ds}{du}$$

Compare both

④

$$\int_0^{\delta} \rho u^2 \alpha \, ds = \frac{\mu U B}{s}$$

$$s \delta s = \frac{\mu B dx}{\rho U \alpha}$$

$$\frac{s^2}{2} = \frac{\mu B}{\rho U \alpha} x + c$$

$$s = \sqrt{\frac{2B}{\alpha}} \cdot \sqrt{\frac{\mu x}{\rho U}}$$

$$B = 1.68, \alpha = 1.35$$

$$s = \frac{4.91 x}{\sqrt{R_x}}$$

where  $(R_x)$  is local Reynold number

As we have

$$\tau_0 = \frac{\mu U B}{s}$$

Put (6) in eq (5)

$$\text{Now } Z_0 = 0.332 \frac{\mu u}{\rho} \sqrt{R_x}$$

$$F_f = B \int Z_0 dx$$

$$\text{where } Z_0 = 0.332 \frac{\mu u}{\rho} \sqrt{R_x}$$

$$R_x = \frac{\rho u^2}{\mu}$$

then putting value we have

$$F_f = 0.664 \sqrt{\rho \mu u^3}$$

As we have

$$F_f = C_f \int \frac{\rho v^2}{2} B dx$$

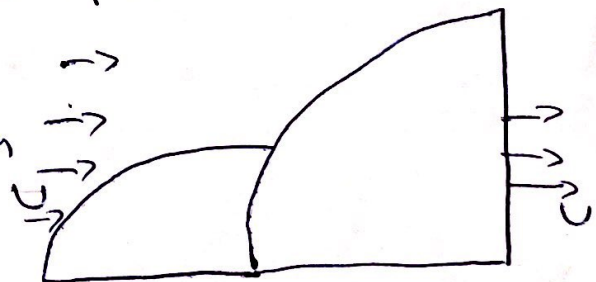
Equating b/s

$$C_f = 1.328 \sqrt{\frac{\mu}{\rho u}} = \boxed{\frac{1.328}{\sqrt{R_x}}}$$

### Turbulent Boundary layer:-

This Fig shows the velocity distribution of boundary layer which is steeper near walls and flatter through one remains of layer

through one remains of layer



The shear stress is greater in turbulent than in laminar

$$\text{thus } Z_0 = \int F \frac{v^2}{\rho}$$

where  $v$  is the average velocity

to obtain relation b/w

$$\frac{V}{U_{max}} = \frac{1}{1 + 1.33\sqrt{F}} \quad \therefore F = 0.028$$

$$\frac{V}{U_{max}} = \frac{1}{1 + 1.33\sqrt{0.028}}$$

$$U = 1.235 \text{ V}$$

$$v = \frac{U}{1.235}$$

$$F = \frac{0.316}{(R_u)^{1/4}}$$

$$R_u = \left(\frac{Dv}{\nu}\right)$$

$$Z_0 = F \left(\frac{\nu}{g}\right)^{1/2}$$

$$Z_0 = \frac{0.316}{\left(\frac{Dv}{1.235}\right)^{1/4}}$$

$$Z_0 = \frac{0.023 \nu^{1/2}}{\left(\frac{25}{v}\right)^{1/4}} \quad \text{--- (1)}$$

As we have general equation

$$Z_0 = \int U^2 \alpha \frac{ds}{dx} \quad \text{--- (2)}$$

equating (1) and (2)

$$u = 0, \quad s = 0$$

$$s = \left(\frac{0.0287}{\alpha}\right)^{4/5} \left(\frac{\nu}{Ux}\right)^{1/5} \cdot x$$

$$d = 0.0972$$

$$s = \frac{0.377}{(R_u)^{2/5}} \cdot x \quad \text{--- (3)}$$

$$Z_0 = 0.0587 \int \frac{U^2}{2} \left(\frac{\nu}{Ux}\right)^{1/5}$$

Now

$$Ff = B \int_0^L z \, dz$$

$$Ff = 0.0735 \int \frac{U^2}{2} \left( \frac{z}{L} \right)^{1/5} \cdot B \, dz$$

$$Ff = \cancel{CF} \cdot \int \frac{U^2}{2} \cdot B \, dz$$

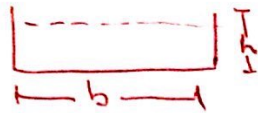
$$CF = \frac{0.0735}{(R)}$$

For  $R > 10$

$$CF = \frac{0.455}{(\log R)^{2.52}}$$



Q. No 1 (B)



A rectangular channel having width 'b' and height (h)

Area

Specific energy equation

$$E = h + \frac{v^2}{2g} \quad (\text{Kinetic energy}) \quad \text{--- (1)}$$

As we know that

$$Q = Av$$

or

$$v = \frac{Q}{A}$$

Put  $v = \frac{Q}{A}$

we get

$$E = h + \frac{Q^2}{A^2 \cdot 2g}$$

as  $A = b \times h$

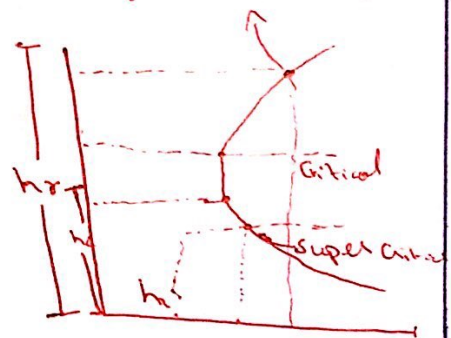
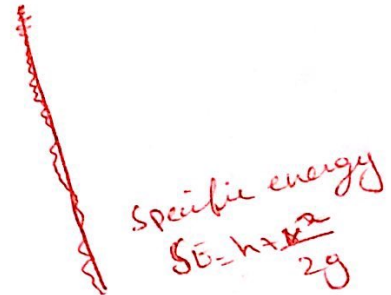
$$E = h + \frac{Q^2}{b^2 h^2 \cdot 2g}$$

as  $q = \frac{Q}{b}$

$$E = h + \left(\frac{Q}{b}\right)^2 \frac{1}{h^2 \cdot 2g}$$

So

$$E = h + \frac{Q^2}{2g \cdot b^2 \cdot h^2} \quad \text{--- (2)}$$



$h_1$  and  $h_2$  = Alternate height

$h_c$  = critical Depth



Derive equation (2) w.r.t "h"

$$\frac{dE}{dh} = \frac{d}{dh} \left( \frac{hc + q^2 hc^3}{2g} \right)$$

$$\frac{dE}{dh} = \frac{1 + (-2)q^2 hc^{-2}}{2g}$$

$$0 = 1 - \frac{2q^2 hc^{-3}}{2g}$$

$$0 = 1 - \frac{q^2}{hc^3 \cdot g}$$

$$0 = \frac{q^2}{hc^3 \cdot g} = 1$$

$$hc^3 \cdot g = q^2$$

$$hc^3 = \frac{q^2}{g}$$

or

$$hc = \left( \frac{q^2}{g} \right)^{1/3} \text{ Critical Depth}$$

Now For critical velocity "vc"

As

$$hc^3 = \frac{q^2}{g} \quad \text{--- (3)}$$

by C.M

$$hc^3 g = q^2$$

Now

$$\frac{q \cdot b}{A} = v$$

$$\frac{q \cdot b}{b \cdot hc} = v$$

$$\text{as } q = \frac{Av}{b}$$

$$q \cdot b = Av$$

or

$$v = \frac{q \cdot b}{A} \text{ as } A = b \cdot h$$

So

$$v = \frac{q}{hc}$$

$$q = vhc$$

From equation (3)

$$q^2 = hc^3 g$$

Put  $q = vhc$

or  $v_c^2 \cdot h^2 = hc^3 \cdot g$

$$v_c^2 = \frac{hc^3 \cdot g}{hc^2}$$

or  $v_c^2 = g \cdot hc$

$$v_c = \sqrt{g \cdot hc}$$

Critical velocity

Therefore

$$\text{Critical Depth } (h_c) = \left( \frac{q^2}{g} \right)^{\frac{1}{3}}$$

$$\text{Critical velocity } "v_c" = \sqrt{g \cdot hc}$$

Q2.

Given

Depth of Rectangular channel ( $d$ ) = ?

Flow rate ( $Q$ ) =  $3.5 \text{ m}^3/\text{sec}$

Slope of Bed ( $S_0$ ) =  $0.0008$

$$h = 0.0249$$

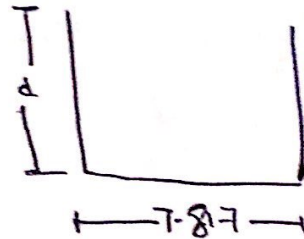
width of bed =  $7.817 = 7.817$

Critical depth = ?

Flow sub critical or super critical = ?

Solution:-

$$\begin{aligned} \text{Area} &= 7.817 \times d \\ &= 7.817d \end{aligned}$$



$$\begin{aligned} \text{Perimeter} &= d + 7.817 + d \\ &= 7.817 + 2d \end{aligned}$$

Hydraulic Radius ( $R_h$ ) =  $A/P$

$$= \frac{7.817d}{7.817 + 2d}$$

By using Manning Equation

$$Q = \frac{1}{n} A R_h^{2/3} S_0^{1/2}$$

Putting values

$$3.5 = \frac{1}{0.0219} \times 7.817d + \left( \frac{7.817d}{2d+7.817} \right)^{2/3} + (0.0008)^{1/2}$$

$$d = 0.558 \text{ m}$$

$$\text{Area} = 7.817(0.558)$$

$$= 4.36 \text{ m}^2$$

$$\text{Perimeter} = 7.817 + 2(0.558)$$

$$= 8.933 \text{ m}$$

$$\text{Hydraulic Radius (R}_h) = \frac{4.36}{8.933}$$

$$= 0.488 \text{ m}$$

Finding critical depth

$$y_{cr} = \left( \frac{q^2}{g} \right)^{1/3}$$

$$= \frac{3.5}{7.817}$$

$$= 0.447 \text{ m}^2/\text{sec}$$

$$y_{cr} = \left( \frac{(0.447)^2}{9.81} \right)^{1/3}$$

$$= 0.273$$

As  $y > y_{cr}$

$$0.558 > 0.273$$

So flow is sub-critical.  
Now critical velocity

$$V_c = \sqrt{g h_c}$$

$$V_c = \sqrt{9.81 \times 0.448}$$

$$V_c = 2.096 \text{ m/sec} \text{ critical velocity}$$

Qno (3)

Given

Friction Drag  ~~$F_D$~~  ( $F_D$ ) = ?

width (B) = 200mm = 0.2m

Length (L) = 800mm = 0.8m

Specific Gravity (S) = 0.89

undisturbed velocity (U) = 5m/sec

Kinematic viscosity ( $\nu$ ) =  $0.93 \times 10^{-4}$  m<sup>2</sup>/sec

Solution

Checking whether flow is laminar or not

By Reynold Number

$$R = \frac{DV}{\nu}$$

For smooth flat plate

$$D = L, \nu = 0$$

$$\text{So } R = \frac{LU}{\nu}$$

$$= \frac{0.8 \times 5}{0.93 \times 10^{-4}} = 43010$$

43010 < 500,000  $\rightarrow$  laminar

By using formula

$$F_f = C_f \cdot \rho \cdot \frac{U^2}{2} \cdot BL$$

where

$$C_f = \frac{1.328}{\sqrt{R}} = \frac{1.328}{\sqrt{43010}} = 0.0064$$

$$S = \frac{\rho_{\text{soil}}}{\rho_{\text{water}}} \rightarrow 0.89 = \frac{\rho_{\text{soil}}}{1000}$$

$$\rho_{\text{soil}} = 0.89 \times 1000$$

$$\rho_{\text{soil}} = 890 \text{ kg/m}^3$$

$$F_f = C_f \cdot \rho \cdot \frac{U^2}{2} \cdot BL$$

$$= 0.0064 \times 890 \times \frac{(5)^2}{2} \times 0.2 \times 0.8$$

$$F_f = 11.39 \text{ N}$$