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Section "A"

Subject Hydraulic Engineering

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Sir

Assignment#01Question#01Venture flume

The term flume is usually applied to devices in which the flow is accelerated due to a streamlined lateral contraction in channel sides and the combination of the lateral contraction, together with a streamlined hump in the inert channel bed.

- ↳ A venture flume is a critical flow open flume with a constricted flow which causes a drop in the hydraulic grade line, creating a critical depth.
- ↳ To ensure the occurrence of critical depth at the throat, the flumes are usually designed in such away that it form a hydraulic jump on the down stream side of the structure and these are called Standing wave flumes.

↳ Venturè flumes are used in open channels for the ~~measurement~~ measurement of very large flow rates, usually given in millions of cubic units

For measurement of discharge with venturè flumes, two measurements are required, one ~~one~~ upstream and other ~~on~~ at the throat. If the flow passes in a subcritical state through the flumes.

Question #02

Given data:

width of the channel = $b = 3\text{m}$

Discharge = $Q = 12\text{ m}^3/\text{sec}$

Required,

- critical depth = $y_c = ?$
- minimum specific energy = $E_{\min} = ?$
- The alternate depths when $E = 4\text{m}$.

Solution:

a) For critical depth,

As we know

$$q_v = \frac{Q}{b} = \frac{12}{3} = 4 \text{ m}^2/\text{sec}$$

we also know that for rectangular channel,

$$y_c = \left(\frac{q_v^2}{g} \right)^{1/3} = \left(\frac{4^2}{9.81} \right)^{1/3}$$

$$\Rightarrow \boxed{y_c = 1.177 \text{ m}}$$

b) For E_{min}

we know that

$$Q = A \cdot V \quad \text{--- (i)}$$

$$\text{also } Q = q_v \cdot b \quad \text{--- (ii)}$$

$$\therefore q_v = \frac{Q}{b}$$

So $A \cdot V = q_v \cdot b$

$$\cancel{b} \cdot y \cdot V = q_v \cdot \cancel{b}$$

$$y \cdot V = q_v$$

$$\cancel{q_v} \Rightarrow V = \frac{q_v}{y}$$

$$\Rightarrow V = \frac{Q}{y_c}$$

$$V = \frac{4}{1.177}$$

$$\Rightarrow \boxed{V = 3.398 \text{ m/sec}}$$

~~we~~
we know that

$$\therefore y = y_c, V = V_c$$

$$\begin{aligned} E_{\min} &= y_c + \frac{(V_c)^2}{2g} \\ &= 1.177 + \frac{(3.398)^2}{2 \times 9.81} \end{aligned}$$

$$\Rightarrow \boxed{E_{\min} = 1.766 \text{ m}}$$

c)

As

$$E = y + \frac{V^2}{2g}$$

$$\Rightarrow E = y + \frac{Q^2/A^2}{2g}$$

$$\begin{aligned} Q &= AV \\ \Rightarrow V &= \frac{Q}{A} \end{aligned}$$

$$E = y + \frac{Q^2}{2g \cdot A^2}$$

$$E = y + \frac{Q^2}{2g \cdot B^2 \cdot y^2}$$

$$\neq A = B \cdot y$$

$$E = y + \frac{q^2}{2g \cdot y^2}$$

$$\therefore q = \frac{Q}{b}$$

⇒ ~~E =~~

$$4 = y + \frac{4^2}{2 \times 9.81 \times y^2}$$

$$4 = y + \frac{0.8155}{y^2}$$

$$\Rightarrow y = 4 - \frac{0.8155}{y^2}$$

By iteration,

when $y = 4$

$$\Rightarrow y = 3.948 \text{ m} \approx 3.95 \text{ m}$$

⇒ From the Super critical Solution, the 2nd term associated with K.E dominates
So, by re arranging

$$y = 4 - \frac{0.8155}{y^2}$$

$$\Rightarrow \text{~~4 - y~~ .}$$

$$4 - y = \frac{0.8155}{y^2}$$

$$\Rightarrow y^2 = \frac{0.8155}{4 - y}$$

$$\sqrt{y^2} = \sqrt{\frac{0.8155}{4 - y}}$$

$$y = \sqrt{\frac{0.8155}{4 - y}}$$

From iteration, at $y = 4$

$$\Rightarrow y = 0.4814 \text{ m}$$

Hence the alternate depths are

$$y = 3.95 \text{ m} \quad \text{E} \quad y = 0.481 \text{ m}$$

Assignment #02

Q #01:

Given data:

$$y = d = 10 \text{ cm} = 0.1 \text{ m}$$

$$\therefore 1 \text{ m} = 100 \text{ cm}$$

$$v = 6 \text{ m/sec}$$

Required:

$$E = ?$$

Solution:

By using Froude number

$$Fr = \frac{V}{\sqrt{gy}}$$

$$= \frac{6}{\sqrt{9.81 \times 0.1}}$$

$$Fr = 6.058$$

As $Fr > 1$

So the flow is supercritical

$$As \ E = y + \frac{v^2}{2g} = 0.1 + \frac{6^2}{2 \times 9.81}$$

$$\Rightarrow \boxed{E = 1.935 \text{ m}}$$

which yields $\boxed{y = 1.935 \text{ m}}$

Q#2Given data

$$v = 2 \text{ m/sec}$$

$$y = 3 \text{ m}$$

Change in bottom deviation = $60 \text{ cm} = 0.6 \text{ m}$

Gradual downward step = $15 \text{ cm} = 0.15 \text{ m}$

Solution,

As

$$E = y + \frac{v^2}{2g}$$

$$\Rightarrow E_1 = y_1 + \frac{v_1^2}{2g} = 3 + \frac{2^2}{2 \times 9.81} = 3.20 \text{ m}$$

Now For downward Step

$$E_2 = E_1 - \Delta Z$$

$$E_2 = 3.20 - 0.60$$

$$\Rightarrow E_2 = 2.60 \text{ m}$$

So

$$\begin{aligned} E_2 &= y_2 + \frac{v_2^2}{2g} \\ &= y_2 + \frac{Q^2/A^3}{2g} \\ &= y_2 + \frac{Q^2}{2g \cdot B^2 \cdot y^3} \end{aligned}$$

$$\Rightarrow E = y_2 + \frac{q^2}{2g \times y_2^3}$$

$$q = \frac{Q}{b}$$

$$2.60 = y_2 + \frac{6^3}{2 \times 9.81 \times 3^3}$$

$$q = \frac{AV}{b}$$

$$q = \frac{v \cdot y \cdot v}{b}$$

$$\Rightarrow q = v \cdot y$$

$$q = 2.3 = 6$$

$$\Rightarrow \boxed{y_2 = 2.34 \text{ m}}$$

$$\Delta y = y_2 - y_1 = 2.34 - 3$$

$$\Delta y = 0.76 \text{ m}$$

So change in upset = $0.76 - 0.6 = 0.16 \text{ m}$

For downward step

$$E_2 = E_1 - \Delta Z = 3.2 - (0.15)$$

$$\boxed{E_2 = 3.35 \text{ m}}$$

So
$$E_2 = y_2 + \frac{q^2}{2g \times y_2^3}$$

$$\Rightarrow y_2 = 3.17 \text{ m}$$

So
$$\Delta y = \cancel{3.17} y_2 - y_1 = 3.17 - 3 = 0.17 \text{ m}$$

Also the water in down step

$$= 0.15 - 0.17$$

$$= -0.02$$

Hence water rises to 0.02 m

Now the maximum upstep possible before affecting upstream water lesser is for

$$y_2 = y_c$$

Hence,

$$y_c = \left(\frac{q^2}{g} \right)^{1/3} = \left(\frac{6^2}{9.81} \right)^{1/3}$$

$$\Rightarrow \boxed{y_c = 1.54 \text{ m}}$$

Assignment#03Question:Given data:

water depth on upstream side (y_1) = 3.6m

water depth at downstream side (y_2) = 0.9m

width of sluice gate (b) = 3.9m

Solution:

As we know that

$$E_1 = E_2$$

$$y_1 + \frac{v_1^2}{2g} = y_2 + \frac{v_2^2}{2g} \quad \text{--- (1)}$$

Also by discharge formula

$$Q = A_1 v_1 = A_2 v_2$$

$$\cancel{b_1} \cdot y_1 \cdot v_1 = \cancel{b_2} \cdot y_2 \cdot v_2 \quad \because b = b_1 = b_2$$

$$y_1 \cdot v_1 = y_2 \cdot v_2$$

$$v_2 = \frac{y_1}{y_2} \times v_1 = \frac{3.6}{0.9} \times v_1$$

$$\Rightarrow v_2 = 4 \cdot v_1 \quad \text{--- (2)}$$

$$\textcircled{1} \Rightarrow y_1 + \frac{v_1^2}{2g} = y_2 + \frac{v_2^2}{2g}$$

$$\Rightarrow 3.6 + \frac{v_1^2}{2g} = 0.9 + \frac{(4v_1)^2}{2g}$$

$$\frac{v_1^2}{2g} - \frac{16v_1^2}{2g} = 0.9 - 3.6$$

$$\Rightarrow + \frac{15v_1^2}{2g} = +2.7$$

$$v_1^2 = \frac{2.7 \times 2 \times 9.81}{15} = 3.532$$

$$\Rightarrow \boxed{v_1 = 1.879 \text{ m/sec}}$$

$$\textcircled{11} \Rightarrow v_2 = 4v_1 = 4 \times 1.879 = 7.516 \text{ m/sec}$$

also

$$Q_1 = A_1 v_1$$

$$= b y_1 \cdot v_1 = 3.9 \times 3.6 \times 1.879$$

$$\Rightarrow \boxed{Q_1 = 26.38 \text{ m}^3/\text{sec}}$$

$$Q = Q_1 = Q_2 = 26.38 \text{ m}^3/\text{sec}$$

So by Froude number, for upstream side

$$Fr_1 = \frac{V_1}{\sqrt{g y_1}} = \frac{1.879}{\sqrt{9.81 \times 3.6}}$$

$$Fr_1 = 0.316 < 1 \rightarrow \text{flow is subcritical}$$

For downstream side

$$Fr_2 = \frac{V_2}{\sqrt{g y_2}} = \frac{7.516}{\sqrt{9.81 \times 0.9}}$$

$$Fr_2 = 2.529 > 1$$

So the flow is supercritical