

<i>Name</i>	<i>Kaleem ullah khan</i>
<i>ID</i>	<i>7681</i>
<i>Sec</i>	<i>c</i>
<i>Dep</i>	<i>civil engineering</i>
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<i>Submitted to</i>	<i>Engr Abdul Waheed</i>

Q No 1

(1)

(a) Drag :

A body which is wholly immersed in a homogenous fluid may be subjected to two kind of forces arising from relative motion b/w body and fluid these forces are termed as drag and lift. if the forces parallel to the motion then it is termed as drag force.

⇒ There are two components.

(i) Pressure Drag (FP) :

It is equal to integrated of components in direction of motion of all pressure forces extended on surface of body.

~~$$FP = CP \frac{\rho V^2}{2} A$$~~

$$FP = CP \int \frac{V^2}{2} A$$

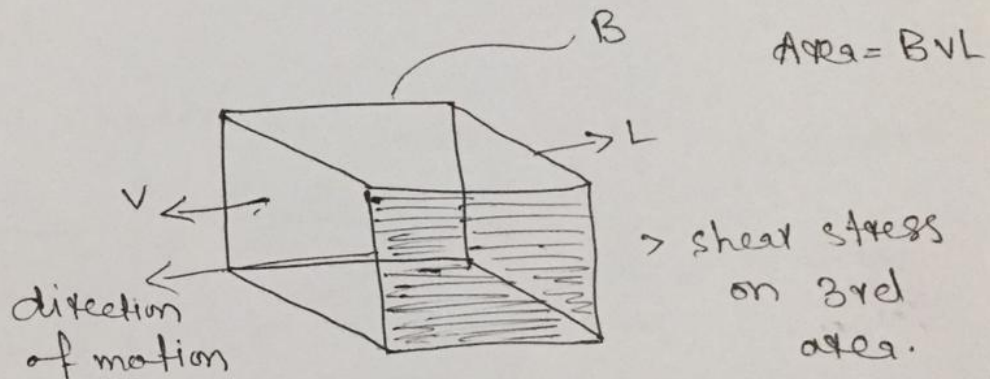
where CP depends on shape.

② Friction Drag (F<sub>f</sub>)

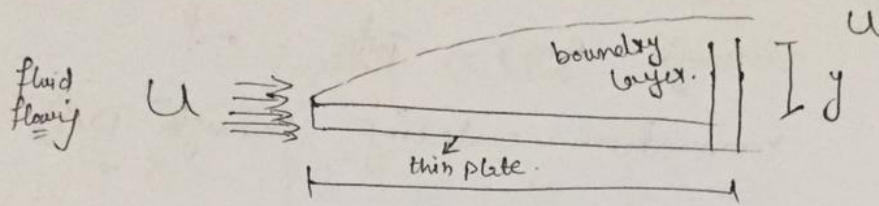
it is equal to integration of components of shear stress along surface of body in direction of motion.

$$F_f = C_f \underbrace{\int \frac{V^2}{2}}_{\text{shear stress}} BL$$

Fig:

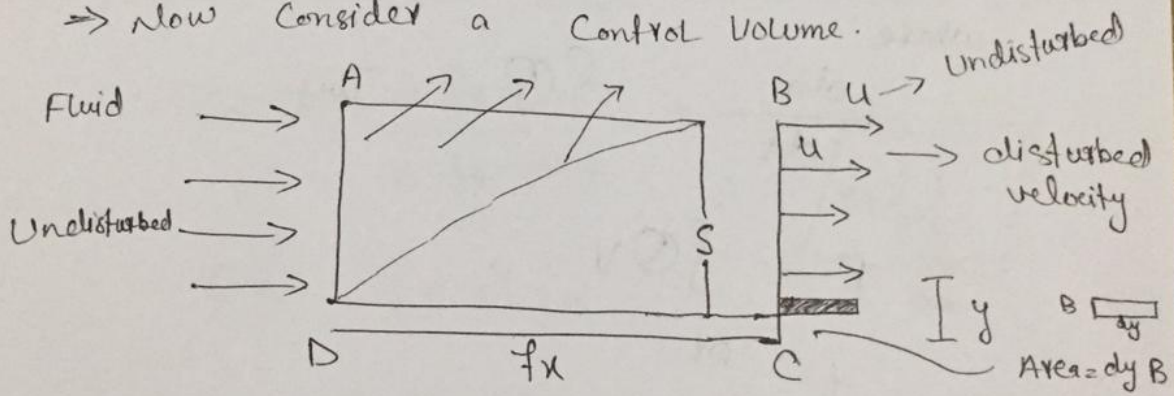


③ ⇒ Friction Drag of Boundary Layer



⇒ Fig shows growth of boundary layer along one side of smooth plate inside the fluid.

⇒ Now consider a control volume.



⇒ where  $S$  is thickness of boundary layer and  $U$  is Undisturbed velocity

Thus  $-F_x = \text{drag} = (\text{rate in momentum in } x\text{-direction})$



(4)

$\Rightarrow$  (Leaving through BC + rate of momentum through AB) - rate of momentum entering through DA)

$$\Delta P = P_{out} - P_{in}$$

Thus according to momentum

$$\Sigma F = \frac{d}{dt}(P) = \frac{dm \cdot v}{dt}$$

where

$$\frac{dm}{dt} = \rho Q \quad \text{Thus}$$

$$F = \rho Q v$$

or

$$F = \int A \cdot v \cdot v$$

$$F = \int A v^2$$

$$DA \rightarrow \int U (UBS)$$

$$BC \rightarrow \int_B \int u^2 \cdot dy$$

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$$AB \rightarrow \int U (UBS - B \int_0^s u \cdot dy)$$

Putting value

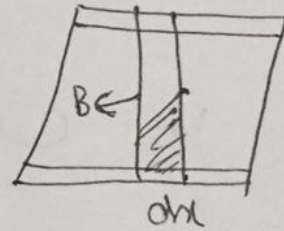
$$F_x = \int_B \int_0^s u (u - u) dy$$

$$F_x = \int_B u^2 f \alpha \quad \text{where } \alpha \text{ is function of boundary layer.}$$

Now to find local wall shear stress

$$\tau_0 = \frac{dF_x}{B \cdot dx - \text{area}}$$

$$F_x = \int B u^2 f \alpha$$



$$\tau_0 = \int u^2 \alpha \frac{ds}{dx} \quad \text{in general}$$

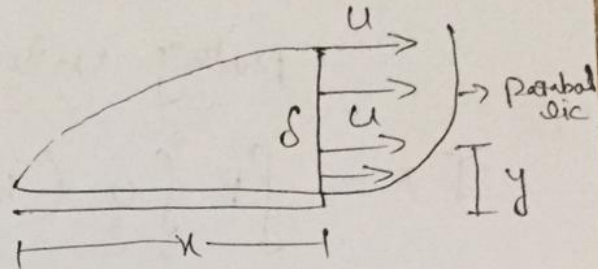
equation of shear stress.

(6)

⇒ Laminar boundary layer :-

$$\frac{u}{V} = F\left(\frac{y}{\delta}\right)$$

Assume



$$\eta = \frac{y}{\delta} \quad \text{or} \quad y = \eta \delta$$

Thus

$$\frac{u}{V} = f(\eta) \quad \text{or} \quad u = Vf(\eta)$$

In case of laminar flow

$$\tau_0 = \mu \left( \frac{du}{dy} \right)$$

$$= \frac{\mu V}{\delta} \left( \frac{df}{d\eta} \right) = \frac{\mu V}{\delta} \left[ \frac{df}{d\eta}(\eta) \right]$$

Solving the Eq.



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$$Z_0 = \frac{UUB}{S} \rightarrow (1)$$

As general Equation is  $Z_0 = \int u^2 \alpha \frac{ds}{dx}$

Equating both equation.

$$\frac{UUB}{S} = \int u^2 \alpha \frac{ds}{dx}$$

or

$$S ds = \frac{UUB}{\int u \alpha} dx$$

Integrating the equation.

$$\frac{S^2}{2} = \frac{UUB}{\int u \alpha} x + C$$

Now at  $x=0$ ,  $S=0$  thus  $C=0$

$$\frac{S^2}{2} = \frac{UUB}{\int u \alpha} x$$



or

(8)

$$\delta = \frac{\sqrt{2 \mu B}}{\rho U \alpha} x \quad \text{or} \quad \sqrt{\frac{2B}{\alpha}} \cdot \sqrt{\frac{\mu x}{\rho U}}$$

Multiplying and dividing by "x"

$$\delta = \sqrt{\frac{2B}{\alpha}} \cdot \sqrt{\frac{\mu x}{\rho U}} \cdot \frac{x}{\sqrt{x} \cdot \sqrt{x}}$$

where

$$\alpha = 0.135$$

$$B = 1.63$$

$$R_x = \frac{\rho U x}{\mu}$$

$$\delta = \frac{4.91}{\sqrt{R_x}} \cdot x \quad \text{or} \quad \frac{\delta}{x} = \frac{4.91}{\sqrt{R_x}}$$

Now

$$Z_0 = \frac{\mu U B}{\delta}$$

Thus putting value

$$Z_0 = 0.332 \frac{\mu U}{x} \sqrt{R_x}$$

where  $R_x$  is local Reynold number.

(9)

Now

$$F_g = B \int_0^2 \frac{\tau_0 dx}{\text{stress}}$$

Putting values:

$$F_g = 0.664 B \sqrt{\rho \mu L V^3}$$

As general equation is

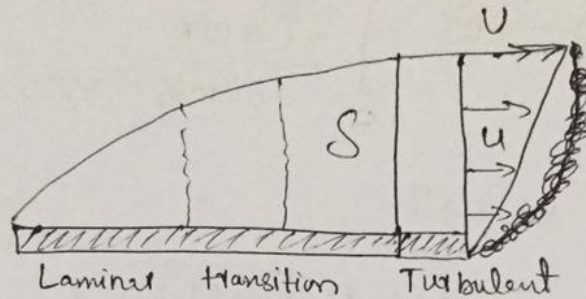
$$F_f = C_f \rho \frac{V^2}{2} BL \rightarrow \text{Equating both equations}$$

$$C_f = 1.328 \sqrt{\frac{\mu}{\rho L V}} = \frac{1.328}{\sqrt{R}}$$

————— x —————

(10)

## TURBULANT BOUNDARY LAYER:



resistance  
is less  
so curve  
become  
straight

Fig show that velocity distribution in turbulent boundary layer shows a much steeper gradient near wall and flatter through out remaining layer.

⇒ To shear stress is greater in turbulent than in laminar layer.

As we have.

$$\tau_0 = f \frac{\rho v^2}{8}$$

where  $v$  denotes  
average velocity of  
pipe.



(11)

→ now we have obtained an approximate relation b/w  $v$  and  $U$  by using pipe factor Equation of

$$\frac{v}{U_{\max}} = \frac{1}{1 + 1.33\sqrt{f}}$$

Using friction factor of 0.028 from chart which is middle critical value.

So

$$U = 1.235v$$

Now we have

$$\tau_0 = f \rho \frac{v^2}{8}$$

As we know

$$f = \frac{0.316}{R^{0.25}}$$

Thus

$$\tau_0 = \frac{0.316}{\left(\frac{Dv}{\nu}\right)^{1/4}} \rho \frac{v^2}{8}$$



(12)

where

$$v = \frac{U}{1.235} \quad \text{thus}$$

$$\tau_0 = \frac{0.316}{\left(\frac{\rho}{\nu} \left(\frac{U}{1.235}\right)\right)^{1/4}} \cdot \frac{\rho}{8} \left(\frac{U}{1.235}\right)^2$$

$$\leq D = 2\delta$$

Thus

$$\tau_0 = \frac{0.0238 U^2}{\left(\frac{\rho U}{\nu}\right)^{1/4}}$$

As we have

$$\tau_0 = \rho U^2 \alpha \frac{ds}{dx}$$

Equating both and integrating for boundary condition of  $x=0$ ,  $s=0$

(13)

Thus

$$S = \left( \frac{0.0287}{\alpha} \right)^{4/5} \left( \frac{U}{Ux} \right)^{1/5} x.$$

$$\text{For } \alpha = 0.0972$$

$$\frac{S}{x} = \frac{0.377}{(Rx)^{1/5}}$$

Putting values in equation.

$$\tau_0 = 0.0587 \rho \frac{U^2}{2} \left( \frac{U}{Ux} \right)^{1/5}$$

Now

$$F_f = B \int_0^L \tau_0 dx$$

$$F_f = 0.0735 \rho \frac{U^2}{2} \left( \frac{U}{UL} \right) BL$$

$$\text{As } F_f = C_f \rho \frac{U^2}{2} BL.$$

Equating Both:

$$C_f = \frac{0.0735}{R^{1/5}}$$

R is less than  $10^7$  for  $500,000 < R < 10^7$

(14)

For  $R > 10^7$

$$Cf = \frac{0.455}{(\log R)^{2.58}}$$

As

X

~~R~~

①

Q No 1  
Part (B)

As specific Energy

$$E = y + \frac{v^2}{2g}$$

The flow  $Q$  per unit width  $b$  can be expressed as

$$q = \frac{Q}{b}$$

Now average velocity will be

$$V = \frac{Q}{A} = \frac{qb}{by} = \frac{q}{y}$$

Thus

$$E = y + \frac{v^2}{2g} \Rightarrow y + \frac{1}{2g} \left( \frac{q^2}{y^2} \right)$$

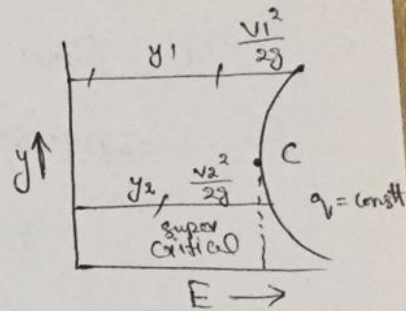
$$(E - y) = \frac{1}{2g} \left( \frac{q^2}{y^2} \right) \quad \text{or} \quad (E - y)y^2 = \frac{q^2}{2g}$$



(2)

This plot of  $E$  vs  $y$  will be parabolic. For particular  $q$ , there will be two kind of possible values of  $y$ , for a given  $E$ .

The Equation is cubic with three roots, with third root being negative point  $C$  represents dividing point b/w two regime of flow



Thus for given  $q$ ,  $\&$  value of  $E$  is minimum  $\&$  Flow at that point is critical flow. Depth of flow at that point is critical depth  $y_c$   $\&$  velocity at that point is critical velocity  $v_c$ .

Thus

(3)

$$E = y + \frac{1}{2g} \left( \frac{v^2}{y^2} \right)$$

For minimum specific energy  $\frac{dE}{dy} = 0$

Thus

$$\frac{dE}{dy} = 1 - \frac{2}{2g} \left( \frac{v^2}{y^3} \right) = 0$$

$$\Rightarrow \frac{v^2}{gy^3} = 1 \Rightarrow v^2 = gy^3$$

$$\frac{v^2}{g} = y^3 \Rightarrow \left( \frac{v^2}{g} \right)^{1/3} = y^{cr}$$

Now

$$v^2 = gy^3$$

$$v = Vy \Rightarrow v^2 y^2 = gy^3$$

or

$$v^2 = gy^{cr}$$

or

$$v^{cr} = \sqrt{gy^{cr}}$$

①

Q2 Given  
Water Flows at rate  $Q = 3.5 \text{ m}^3/\text{s}$   
Bed Slope,  $S_0 = 0.0008$   
 $n = 0.0219$

width of bed is student ID = 7681

Required:

Depth of rectangular channel = ?

Critical Depth  $y_c = ?$

Critical velocity  $V_c = ?$

Flow is critical or subcritical = ?

Solution:

$$Q = \left( \frac{1}{n} R n^{2/3} S_0^{1/2} \right) A \rightarrow \text{①}$$

$$\text{Area} = 7.681 \times d$$

$$\text{Parameter} = d + 7.681 + d$$



(2)

$$\text{Parameter} = d + 7.681 + d$$

$$\text{hydraulic Radius} = R_n = \frac{\text{Area}}{\text{Parameter}}$$

$$= \frac{7.681 \times d}{2d + 7.681}$$

we know that

$$Q = \left( \frac{1}{n} R_n^{2/3} S_0^{1/2} \right) A$$

put the value

$$3.5 = \left( \frac{1}{0.0219} \times \left( \frac{7.681 \times d}{2d + 7.681} \right)^{2/3} \times (0.0008)^{1/2} \right) \times 7.681 d$$

$$\frac{3.5 \times 0.0219}{\sqrt{0.0008}} = \left( \frac{7.681 \times d}{2d + 7.681} \right)^{2/3} \times 7.681 d$$

$$(2.59)^{3/2} = \frac{7.681 d}{2d + 7.681} \times 7.681 d$$



(3)

$$(4.461)(2d + 7.681) = 58.99d^2$$

$$8.92d + 34.21 = 58.99d^2$$

$$58.99d^2 - 8.92d - 34.21 = 0$$

$$d = 0.840\text{m}$$

So the depth of channel is 0.840m

As  $q = \frac{Q}{b}$

$$q = \frac{3.5}{7.681} = 0.455$$

For critical depth

$$y_c = \left( \frac{q^2}{g} \right)^{1/3}$$

$$y_c = \left( \frac{(0.455)^2}{(9.81)} \right)^{1/3}$$

(4)

$$y_{cr} = 0.276 \text{ m}$$

Now critical velocity

$$V_{cr} = \sqrt{y_{cr} \times g}$$

$$V_{cr} = \sqrt{(9.81)(0.276)}$$

$$V_{cr} = 1.645 \text{ m/s}$$

$$V = \frac{Q}{A}$$

$$V = \frac{3.5}{7.681 \times 0.846}$$

$$V = 0.542 \text{ m/s}$$

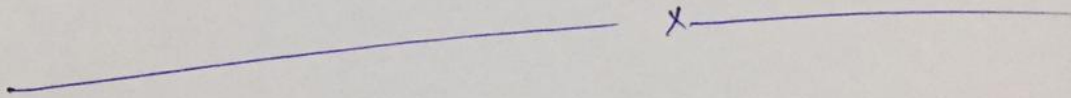
(5)

$$y = 0.840 \text{ m}, \quad y_{cr} = 0.276 \text{ m}$$

$$V = 0.542 \text{ m/s}, \quad V_{cr} = 1.645 \text{ m/s}$$

As  $y > y_{cr}$  and  $V < V_{cr}$

So the flow is subcritical





(1)

Q3 Given DATA:

width of smooth plate  $B = 200 \text{ mm}$

Length of smooth plate  $L = 800 \text{ mm}$   
 $= 0.8 \text{ m}$

Oil with specific gravity;  $S = 0.89$

Undisturbed velocity,  $u = 5 \text{ m/sec}$

Kinematic viscosity  $\nu = 0.93 \times 10^{-4} \text{ m}^2/\text{s}$

Required DATA:

Friction drag on one side of a  
smooth plate,  $F_f = ?$

SOL:

check the flow.

As  $\nu = 0.93 \times 10^{-4} \text{ m}^2/\text{s}$

$$R = \frac{LU}{\nu} = \frac{(0.8)(5)}{0.93 \times 10^{-4}}$$

$$R = 43010.75 < 500,000$$

Thus flow is laminar.

(2)

Now

$$C_f = \frac{1.328}{\sqrt{R}} \Rightarrow \frac{1.328}{\sqrt{43010.75}}$$

$$C_f = 6.403 \times 10^{-3}$$

$$C_f = 0.0064$$

$$\Rightarrow F_f = C_f f \frac{V^2}{2} BL$$

$$= (0.0064) (\text{Soil} \times \gamma_{\text{water}}) \times \frac{5^2}{2} \times 0.2 \times 0.8$$

$$= (0.0064) (0.89 \times 1000) \times \frac{5^2}{2} \times (0.2) (0.8)$$

$$\boxed{F_f = 11.392 \text{ N}}$$

Ans