

(Question # 01)

①

Advanced Mechanics of Materials  
Student ID # 11345

Solution = The moment at Section A is

$$M = 11000 \times 11 = 121000 \text{ Nm}$$

& the torque on the shaft is

$$T = 11000 \times 0.11 = 1210 \text{ Nm}$$

The normal stress due to  $M$  at A is

$$\sigma = \frac{64Md}{\pi d^4} = \frac{32M}{\pi d^3} \quad \left. \begin{array}{l} \text{maximum shear stress} \\ \text{due to } T \text{ at A is} \\ T = \frac{32Td}{\pi d^4} = \frac{16T}{\pi d^3} \end{array} \right\}$$

The Shear Stress due to shear force  $F$  is zero at A.

$$\sigma_{1,3} = \frac{1}{2} \sigma \pm \frac{1}{2} (\sigma^2 + 4\tau^2)^{1/2}$$

(i) Maximum Shear Stress Theory

$$\tau_{max} = \frac{1}{2} (\sigma_1 - \sigma_3)$$

$$= \frac{1}{2} (\sigma^2 + 4\tau^2)^{1/2}$$

$$= \frac{1}{2} \frac{32}{\pi d^3} (M^2 + T^2)^{1/2}$$

$$= \frac{16}{\pi d^3} (121000^2 + 1210^2)^{1/2} = \frac{391072}{d^3} \text{ Pa}$$

with a factor of safety  $N=11$  the value of  $\tau_{max}$  becomes

$$N\tau_{max} = \frac{45264}{d^3} \text{ Pa}$$

(2)

This should not exceed the maximum shear stress value at yielding in uniaxial tension test. Thus

$$\frac{1}{d^3} (45264) = \frac{\sigma_y}{2} = \frac{207}{2} \times 10^6$$

$$d^3 = 4512 \times 10^{-6} \text{ m}^3$$

$$d = 41.25 \times 10^{-2} = 41.25 \text{ cm}$$

(ii) Octahedral Shear Stress Theory

$$\tau_{oct} = \frac{1}{3} \left[ (\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2 \right]^{1/2}$$

with  $\sigma_2 = 0$

$$\tau_{oct} = \frac{1}{3} \left[ 2\sigma_1^2 + 2\sigma_3^2 - 2\sigma_1\sigma_3 \right]^{1/2}$$

$$\tau_{oct} = \frac{\sqrt{2}}{3} (\sigma^2 + 3v^2)^{1/2}$$

$$= \frac{\sqrt{2}}{3\pi d^3} \left[ (32\pi)^2 + 3(16\pi)^2 \right]^{1/2}$$

$$= \frac{16\sqrt{2}}{3\pi d^2} (4\pi^2 + 3\pi^2)^{1/2}$$

$$= \frac{16\sqrt{2}}{3\pi d^3} \left[ 4(121000)^2 + 3(1210)^2 \right]^{1/2}$$

$$= \frac{\sqrt{2}}{3\pi d^3} \times 393418 = \frac{\sqrt{2}}{3} \sigma_y$$

(8)

Equating this to octahedral shear stress at yield stress of an uniaxial tension bar & using a factor of safety = 1)

$$\frac{\sqrt{11}}{3A d^3} \times 11 \times 343418 = \frac{\sqrt{17}}{3} \sigma_y$$

$$\text{or } 11 \times 343418 = \pi d^3 \sigma_y = \pi d^3 \times 207 \times 10^6$$

$$d^3 = 3.77 \times 10^{-3}$$

$$d = 37.7 \text{ cm}$$

# Advanced Mechanics of materials

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(4)

(Question # 02)

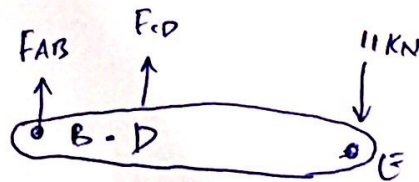
Solution, Apply a free body analysis to the Bar BDE to find the forces exerted by links AB & DC

→ Evaluate the deformation of links AB & DC or the displacements of B & D.

→ work out the geometry to find the deflection at E given the deflection at B & D.

(Free Body:)

Bar BDE



$$\sum M_B = 0$$

$$0 = -(11 \text{ kN} \times 0.6) + F_{DC} \times 0.2 \text{ m}$$

$$F_{DC} = + 33 \text{ Tension}$$

$$\sum M_D = 0$$

$$0 = -(11 \text{ kN} \times 0.4) - F_{AB} \times 0.2 \text{ m}$$

$$F_{AB} = - 22 \text{ Compression}$$

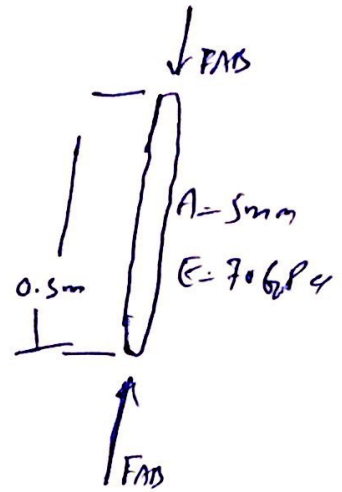
Displacement of B:

$$s_B = \frac{PL}{AE}$$

$$= \frac{(-22 \times 10^3 \text{ N}) (0.3 \text{ m})}{(5 \times 10^{-6} \text{ m}^2) (70 \times 10^9 \text{ Pa})}$$

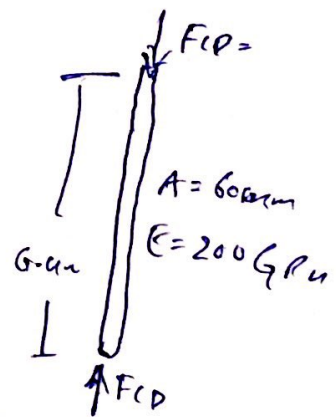
$$= -0.0188 \times 10^{-6} \text{ m}$$

$$\delta_{15} = \boxed{0.188 \text{ mm}}$$



Displacement of D: 
$$\frac{(33 \times 10^3 \text{ N}) (0.4 \text{ m})}{(60 \times 10^{-6} \text{ m}^2) (200 \times 10^9)}$$

$$\delta_D = \boxed{1.1 \times 10^{-6} \text{ m}}$$

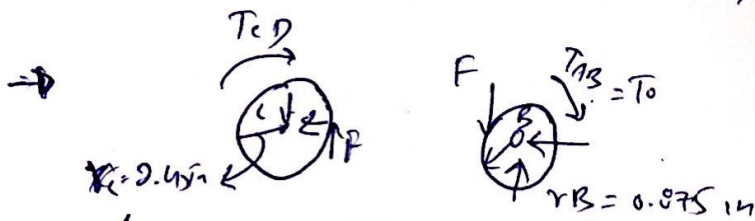


Question # 3

6

Solution:

- Apply a static equilibrium analysis on the two shafts to find a relationship b/w  $T_{CD}$  &  $T_0$
- Apply a kinematic analysis to relate the angular rotations of the gears.
- Find the maximum allowable torque on each shaft - choose the smallest.
- Find the corresponding angle  $\theta$  of twist for each shaft & the net angular rotation of end 4.



$$\sum M_B = 0 = F(0.875 \text{ m}) - T_0$$

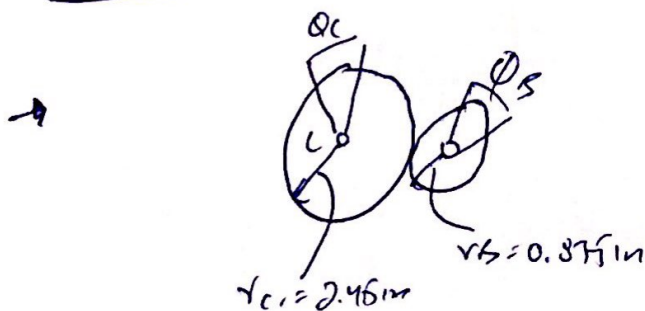
$$\sum M_C = 0 = F(2.45 \text{ m}) - T_{CD}$$

$$T_{CD} = 2.8 T_0$$

$$G = 41 \times 10^9 \text{ Pa}$$

$$L = 10 \text{ m}$$

$$\text{Dimensions} = 11 + 10 = 21$$



7

$$r_B \phi_B = r_C \phi_C$$

$$\phi_B = \frac{r_C}{r_B} \phi_C = \frac{245}{0.875} \phi_C$$

$$\boxed{\phi_B = 2.8 \phi_C}$$

$$\tau_{max} = \frac{T_{ASC}}{J_{AS}}$$

$$8000 \text{ psi} = \frac{T_0 (0.375 \text{ in})}{\frac{\pi}{2} (0.875 \text{ in})^4}$$

$$T_0 = 663 \text{ lb}\cdot\text{in}$$

$$T_{max} = \frac{T_C D_C}{J_C D}$$

$$8000 \text{ psi} = \frac{2.8 T_0 (0.5 \text{ in})}{\frac{\pi}{2} (0.5 \text{ in})^4}$$

$$T_0 = 561 \text{ lb}\cdot\text{in}$$

$$\boxed{T_0 = 561 \text{ lb}\cdot\text{in}}$$

$$\phi_{A/B} = \frac{T_{ABL}}{J_{ABG}} = \frac{(581 \text{ lb}\cdot\text{in})(24 \text{ in})}{\frac{\pi}{2} (0.575 \text{ in})^4 (11.2 \times 10^6 \text{ psi})}$$

$$= 0.387 \text{ rad} = 2.22^\circ$$

$$\phi_{C/D} = \frac{T_{CDL}}{J_{CDG}} = \frac{2.8(581 \text{ lb}\cdot\text{in})(24 \text{ in})}{\frac{\pi}{2} (0.5 \text{ in})^4 (11.2 \times 10^6 \text{ psi})}$$

$$= 0.514 \text{ rad} = 2.95^\circ$$

$$\phi_B = 2.8 \phi_C = 2.8 (2.95^\circ) = 8.26$$

$$\phi_A = \phi_B + \phi_{A/B} = 8.26 + 2.22$$

$$\boxed{\phi_A = 10.48^\circ}$$



Question = 04

(9)

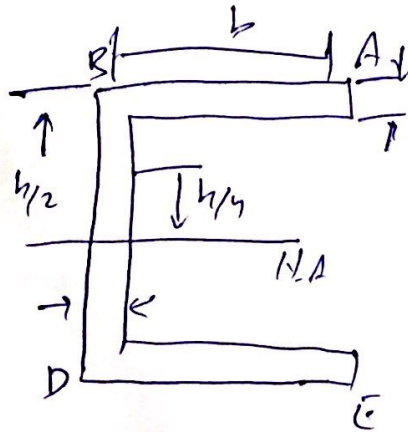
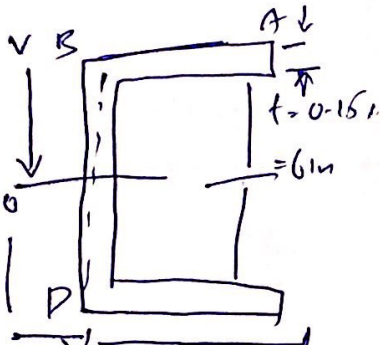
Given Data

$$b = 4 \text{ inch}$$

$$h = 6 \text{ inch}$$

$$t = 0.15 \text{ in}$$

$$V = 11 + 3 = 14 \text{ kips}$$



Solution:  $b = 4 \text{ in}$   
 $t = 0.15 \text{ in}$

Shear Stress distribution for

$$V = 14 \text{ kips}$$

$$\tau = \frac{VQ}{It} = \frac{VQ}{It}$$

Shearing stresses in the flanges

$$\tau = \frac{VQ}{It} = \frac{V}{It} (St) \frac{h}{2} = \frac{Vh}{2I} S$$

$$\tau_B = \frac{Vhb}{2 \left( \frac{1}{12} th^2 \right) (6b+h)} = \frac{6Vb}{th(6b+h)}$$

$$= \frac{14(2.5 \text{ kips})(4 \text{ in})}{(0.15 \text{ in})(6 \text{ in})(6 \times 4 + 6 \text{ in})} = \frac{140}{129.6}$$

$$\tau_B = \frac{1.080 \text{ ksi}}{1.080 \text{ ksi}}$$

(10)

Shearing stress in the web

$$\tau_{max} = \frac{VQ}{It} = \frac{V(k_b h^2)(4b+h)}{\frac{1}{12} t h^3 (6b+h)t}$$

$$= \frac{3V(4b+h)}{2t(6b+h)}$$

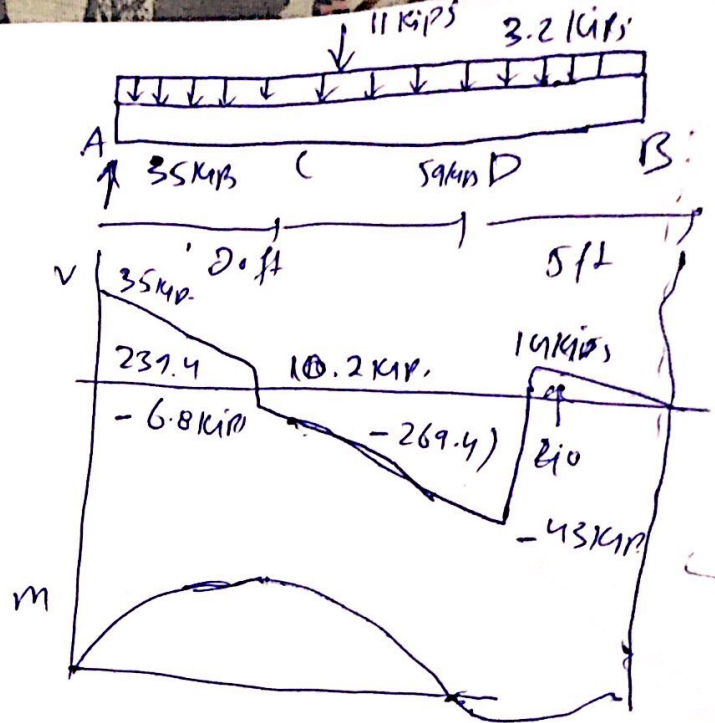
$$= \frac{3(14)(4 \times 4 + 6)}{2(0.15)(6 \times 6 + 6)}$$

$$= \frac{4.082}{388.8}$$

$$\tau_{max} = 10.37 \text{ ksi}$$

(11)

(Question # 05)



Solution:

- Determine reaction at A & D

$$\sum M_A = 0 \Rightarrow R_D = 59.1 \text{ kips}$$

$$\sum M_D = 0 \Rightarrow R_A = 35 \text{ kips}$$

- Determine maximum shear & bending moment from shear & bending moment.

$$|M|_{\max} = 293 \text{ kip}\cdot\text{ft} \text{ with } V = 10.2 \text{ kips}$$

$$|V|_{\max} = 43 \text{ kips}$$

- calculate required Section modulus & select appropriate beam section

$$S_{\min} = \frac{|M|_{\max}}{\sigma_{\text{all}}} = \frac{293 \text{ kip}\cdot\text{ft}}{24 \text{ ksi}} = 119.7 \text{ in}^3$$

Select W21 x 62 beam section

- Find maximum shearing stress

Assuming uniform shearing stress in web,

$$\tau_{max} = \frac{V_{max}}{A_{web}} = \frac{43 \text{ kips}}{8.40 \text{ in}^2} = 5.12 \text{ ksi} \text{ to } 14.5 \text{ ksi}$$

- Find maximum normal stress

$$\sigma_a = \frac{M_{max}}{S} = \frac{2893 \text{ kip}\cdot\text{in}}{127 \text{ in}^3} = 22.6 \text{ ksi}$$

$$\sigma_b = \sigma_a \frac{y_b}{c} = (22.6 \text{ ksi}) \frac{9.80}{10.5} = 21.3 \text{ ksi}$$

$$\tau_b = \frac{V}{A_{web}} = \frac{V}{A_{web}} = \frac{12.2 \text{ kips}}{8.40 \text{ in}^2} = 1.45 \text{ ksi}$$

$$\sigma_{max} = \frac{21.3 \text{ ksi}}{2} + \sqrt{\left(\frac{21.3 \text{ ksi}}{2}\right)^2 + (1.45 \text{ ksi})^2}$$

$$F = 21.4 \text{ ksi} \text{ to } 24 \text{ ksi}$$